



Enhancing our communities



Melville Court Development, Oro-Medonte

FUNCTIONAL SERVICING REPORT

Doncor Developments Inc.

Document Control

File:

420427

Date:



January
28, 2022

Prepared by:

Tatham Engineering Limited
41 King Street, Unit 4
Barrie, Ontario L4N 6B5
T 705-733-9037
tathameng.com

Prepared for:

Doncor Developments Inc.
1004 Line 4 North
Oro-Medonte, Ontario L0L 2L0

| Authored by: | Reviewed by: |
|--|--|
|  |  |
| <p>Lauren Buss, B.Eng., P.Eng. Intermediate Engineer</p> | <p>Matt Brace, P.Eng. Senior Project Manager</p> |

| Disclaimer | Copyright |
|---|---|
| <p>The information contained in this document is solely for the use of the Client identified on the cover sheet for the purpose for which it has been prepared and Tatham Engineering Limited undertakes no duty to or accepts any responsibility to any third party who may rely upon this document.</p> | <p>This document may not be used for any purpose other than that provided in the contract between the Owner/Client and the Engineer nor may any section or element of this document be removed, reproduced, electronically stored or transmitted in any form without the express written consent of Tatham Engineering Limited.</p> |

| Issue | Date | Description |
|-------|------------------|------------------|
| 1 | January 28, 2022 | First Submission |

Document Contents

| | | |
|----------|---|-----------|
| 1 | Introduction | 3 |
| 1.1 | Site Description & Surrounding Land Use..... | 3 |
| 1.2 | Proposed Development..... | 4 |
| 1.3 | Objectives | 4 |
| 1.4 | Guidelines & Background Information | 4 |
| 2 | Water Supply & Distribution | 6 |
| 3 | Sanitary Sewage System..... | 7 |
| 3.1 | Existing Sanitary Sewage System | 7 |
| 3.2 | Proposed Sanitary Sewage System | 7 |
| 4 | Preliminary Stormwater Management Plan | 8 |
| 4.1 | Design Criteria | 8 |
| 4.2 | Background Information..... | 9 |
| 4.3 | Existing Conditions..... | 9 |
| 4.4 | Proposed Conditions | 12 |
| 5 | Siltation & Erosion Control Plan | 20 |
| 6 | Grading & Landscaping | 21 |
| 7 | Utilities..... | 22 |
| 8 | Summary..... | 23 |
| 8.1 | Water Supply & Distribution..... | 23 |
| 8.2 | Sanitary Sewer Collection | 23 |
| 8.3 | Stormwater Management Plan..... | 23 |
| 8.4 | Siltation & Erosion Control | 23 |
| 8.5 | Grading & Landscaping | 23 |
| 8.6 | Utilities | 23 |



Tables

Table 1: Existing Operating Characteristics Summary - Outlet 1 11
Table 2: Existing Peak Flow Summary - Outlet 2 12
Table 3: Proposed Conditions Operating Characteristics Summary - Outlet 1..... 13
Table 4: Proposed Conditions Peak Flow Summary - Outlet 2 14
Table 5: Proposed Conditions Operating Characteristics Summary - Outlet #1 with Retrofit ... 15
Table 6: Proposed Conditions Peak Flow Summary - Outlet #2 with Retrofit..... 15
Table 7: Phosphorus Loading Summary..... 19

Figures

Figure 1: Site Location Plan 4

Appendices

- Appendix A: Draft Plan
- Appendix B: Sanitary Sewage System Calculations
- Appendix C: Stormwater Management Calculations
- Appendix D: Phosphorus Budget Calculations
- Appendix E: Utilities Correspondence
- Appendix F: Drawings



1 Introduction

Tatham Engineering Limited (Tatham) has been retained by Doncor Developments Inc. to prepare a Functional Servicing Report (FSR) in support of concurrent Official Plan Amendment (OPA), Zoning By-law Amendment (ZBA), and Draft Plan of Subdivision (draft plan) applications for a proposed residential development on Melville Court(subject site) located in the community of Guthrie, in the Township of Oro-Medonte (Township).

This report presents a servicing strategy, covering:

- water supply;
- sanitary sewage disposal system;
- drainage and stormwater management;
- grading and landscaping; and
- utility servicing.

1.1 SITE DESCRIPTION & SURROUNDING LAND USE

The site consists of approximately 6.6 ha of undeveloped land and is bound by farmland to the north and west, Melville Court and existing residential properties to the south, and Line 5 North to the east. The site is legally described as the east half of Lot 20, Concession 5 (Former Township of Oro). The site is located within the Lake Simcoe Region Conservation Authority (LSRCA) watershed. The east portion of the site is located in the LSRCA regulated area due to a tributary to Shelswell's Creek southeast of the property.

The location of the property is illustrated on the Site Location Plan (Figure 1), overleaf.



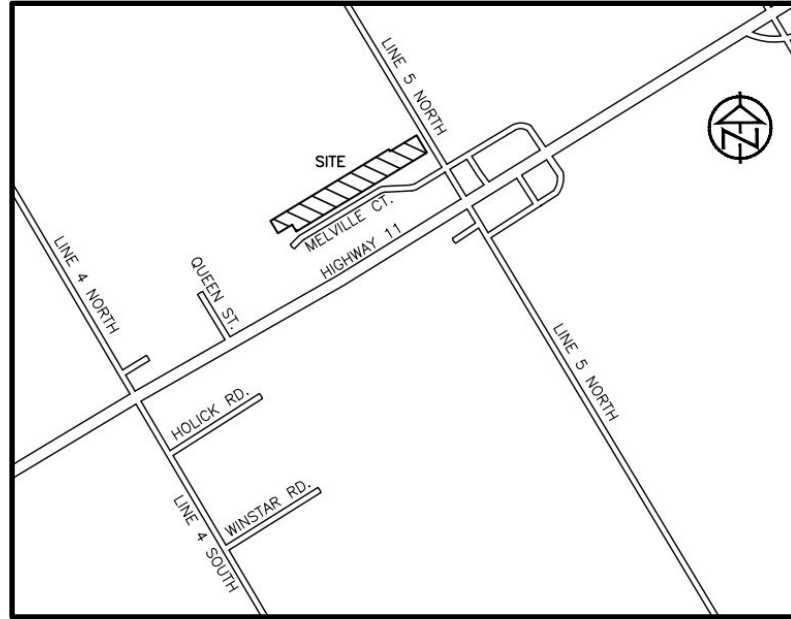


Figure 1: Site Location Plan

1.2 PROPOSED DEVELOPMENT

The proposed development consists of 16 single-family residential lots. 14 of the lots front onto Melville Court, while two lots front onto Line 5 North. The proposed draft plan, prepared by Innovative Planning Solutions is provided in Appendix A.

It is noted that there are no existing municipal water or sanitary services within the vicinity of the site. As such, each lot will be serviced with individual wells and sewage disposal systems. There is an existing stormwater management (SWM) facility southeast of the site which will be utilized to provide the majority of SWM controls required in support of the proposed development.

1.3 OBJECTIVES

The primary objective of this report is to assess the feasibility of the proposed development with respect to servicing and SWM and to ensure satisfactory information on these items is presented in support of the proposed planning applications. This will involve an evaluation of existing and future site conditions.

1.4 GUIDELINES & BACKGROUND INFORMATION

This report has been prepared recognizing applicable municipal, provincial and agency guidelines, including:



- The Ministry of the Environment, Conservation, and Parks (MECP, formerly known as Ministry of Environment), *Stormwater Management Practices Planning and Design Manual* (2003);
- The Ministry of the Environment, Conservation, and Parks (MECP, formerly known as Ministry of Environment), *Lake Simcoe Protection Plan* (LSPP) (2009);
- Lake Simcoe Region Conservation Authority (LSRCA), *Technical Guidelines for SWM Submissions* (2016);
- Lake Simcoe Region Conservation Authority (LSRCA), *Phosphorus Offsetting Policy* (2019);
- Government of Ontario, *Ontario Regulation 332/12: Building Code* (2020);
- Township of Oro-Medonte, *Development Engineering, Policies, Process, and Design Standards* (2019); and
- Ministry of Transportation Ontario (MTO) *IDF Curve Look-up Tool*, accessed August 12, 2020

The following studies have also been reviewed in support of our preparation of this FSR:

- Gemmell Engineering *Melville Meadows Functional Servicing Report*, (2010);
- Tatham Engineering Limited, formerly C.C. Tatham and Associates Limited *Oro-Medonte Comprehensive Stormwater Management Master Plan – Township of Oro Medonte Municipal Class Environmental Assessment*, (2015);
- Tatham Engineering Limited, formerly C.C. Tatham and Associates Limited *Guthrie Stormwater and Drainage Improvement Project Design Brief*, (2016);
- Peto MacCallum Limited, *Geotechnical/Hydrogeological Investigation Proposed Expansion of Melville Court Subdivision Oro-Medonte, Ontario*, (2021); and
- Peto MacCallum Limited, *Ground Water Level Monitoring Proposed Expansion of Melville Court Subdivision*, (2021).

It should be noted that Tatham was previously retained by the Township in 2015 to assess the drainage and flooding issues experienced in the area, as well as the existing SWM facility located on the south side of Melville Court. The SWM assessment provided in this FSR relies on background information presented in that report (*Guthrie Stormwater and Drainage Improvement Project Design Brief* (2016)).



2 Water Supply & Distribution

Under existing conditions there is no municipal water infrastructure within Meville Court, Line 5 North or within the vicinity of the subject lands. As such, private wells will be required for each lot to supply water to meet domestic and fire flow demands.

Based on our review of the Geotechnical/Hydrogeological Investigation dated 2021 completed by Peto MacCallum Ltd. (Peto) (provided under separate cover), adequate supply is anticipated to be available to service the proposed private wells. The site is not located within a Well Head Protection Area (WHPA) or an Intake Protection Zone (IPZ). The approximate location of each proposed well is noted on the Preliminary Site Servicing and Grading Plan (Drawing SSG-1) appended. The ultimate location of the individual wells will be determined during the detailed design and/or building permit stage.

Following well installation, a pump test will be required to confirm that an adequate water supply can be provided and that the sustained pumping rate will not have an adverse impact on other wells in the area. Refer to the geotechnical/hydrogeological report for additional information related to well supply.



3 Sanitary Sewage System

3.1 EXISTING SANITARY SEWAGE SYSTEM

There is no municipal sanitary infrastructure within Melville Court, Line 5 North or within the vicinity of the site. As such, each lot will require an on-site wastewater treatment system.

3.2 PROPOSED SANITARY SEWAGE SYSTEM

Two preliminary septic system designs (Options #1 and #2) have been prepared based on varying T-times of the soil for a 4-bedroom dwelling with an estimated footprint area of 266 m² and a design flow of 2,700 L/day. Based on review of the geotechnical/hydrogeological report prepared by Peto, in-situ soils have percolation rates (“T”-times) ranging between 8 to 12 min/cm and 20 to 50 min/cm depending on the borehole locations.

A summary of preliminary design details is provided below with additional calculations provided in Appendix B.

3.2.1 Septic System Option #1

Septic system option #1 is proposed for Lots 5 to 14 where T-Times are approximately 12 min/cm. This will consist of a 6,000 L septic tank, 36.0 m² filter bed area and 270 m² loading area. Septic bed design for Lots 5 to 14 will not be impacted by the deeper groundwater elevations in this area.

3.2.2 Septic System Option #2

Septic system option #2 is proposed for Lots 1 to 4 and 15 and 16 where the average T-times are 35 min/cm. This will consist of a 6,000 L septic tank, 36 m² filter bed and 338 m² loading area.

The high groundwater level at BH1 on the east side of the site is 0.9 m below ground surface. Septic bed designs for Lots 15 and 16 will be raised slightly to retain a minimum clearance of 0.9 m between high groundwater level and the underside of the filter bed layer. Septic bed design for Lots 1 to 4 will not be impacted by the deeper groundwater elevations in this area.

Locations of proposed septic systems are illustrated on the Drawing SSG-1. It is noted that, if required, the septic bed footprint could be further reduced by providing additional treatment and/or use of filter bed medium type bed material. Detailed design of the septic systems will be completed at the detailed design and/or building permit application stage.



4 Preliminary Stormwater Management Plan

4.1 DESIGN CRITERIA

This preliminary SWM plan is subject to the review and approval of the Township and the LSRCA. Applicable SWM design criteria for the proposed development are presented below.

4.1.1 Stormwater Quality Control

Water quality controls must be provided to satisfy the MECP's *Stormwater Management Practices Planning and Design Manual*. This corresponds to providing Enhanced Protection Level water quality protection of 80% total suspended solids (TSS) removal. Enhanced level treatment will be provided via enhanced ditches and within the existing SWM facility.

4.1.2 Stormwater Quantity Control

The Township requires that post-development peak flow rates be controlled to pre-development levels at any given outlet location to ensure no adverse impacts for downstream landowners. As such, water quantity controls will be provided by the existing downstream SWM facility which has sufficient capacity to attenuate post-development peak flow rates to pre-development rates.

4.1.3 Water Balance

As the development area is over 500 m², the proposed development is categorized as a "major development" under the LSPP. Therefore, best efforts must be demonstrated to maintain pre-development infiltration rates in the post-development scenario through the completion of a water balance assessment.

A water balance assessment has been completed as part of the *Geotechnical/Hydrogeological Investigation* by Peto MacCallum Ltd. (dated June 2021).

4.1.4 Phosphorus Budget

The proposed development is expected to be subject to the Lake Simcoe Phosphorus Offsetting Policy (LSPOP), which requires all major development to control 100% of the phosphorus generated from the site. Any remaining phosphorus load that cannot be controlled/removed will require a cash contribution for off-site mitigation.



4.2 BACKGROUND INFORMATION

Information relating to existing topography, ground cover and drainage patterns was obtained through a review of available background studies, available plans, and base mapping. Detailed contour mapping was prepared using detailed topographic survey completed by JoeTopo in October 2021.

4.3 EXISTING CONDITIONS

The site is approximately 6.6 ha and predominately drains from west to east towards existing roadside ditching on Melville Court and Line 5 North where it is collected and conveyed southeast to an existing SWM facility (dry pond). Controlled flows from the dry pond discharge to the existing outlet drainage channel which conveys flows southwest to an existing Highway 11 culvert crossing, ultimately draining into Lake Simcoe. The majority of the site consists of farmland with some woodland areas fronting Line 5 North. Soils in the area are classified as Guerin, Vasey and Sargent sandy loam soils which are part of Hydrologic Soil Group (HSG) AB. This is consistent with the findings of the geotechnical/hydrogeological report prepared by Peto. The geotechnical/hydrogeological report confirms a topsoil thickness of 220 to 280 mm throughout the site as well as a silt and sand layer extending from 0.8 to 2.9 m depth. Groundwater levels range from 0.9 to 4.8 meters below ground surface.

Based on review of available background reports and correspondence, the channel receives drainage from approximately 65.9 ha of upstream drainage area and is subject to reoccurring flooding between the existing SWM facility and Highway 11 culvert crossing.

4.3.1 Existing Dry Pond

There is an existing dry pond located on Melville Court, southeast of the site which was originally constructed as part of the Hastings Estate Residential Subdivision. Based on review of detailed topographic survey information completed in 2015, the total available active storage volume in the pond is approximately 3,347 m³ (at top of pond elevation 288.98 m). The pond is controlled via a 300 mm dia. storm pipe which discharges into the existing conveyance channel at elevation 287.19 m. The pond was designed to provide water quantity control for approximately 9.9 ha of drainage area (including the subject site) per the Hasting Subdivision Storm Drainage Area Plan provided in Appendix C.

4.3.2 Existing Drainage Issues

In 2015, the Oro-Medonte Comprehensive Stormwater Management Master Plan (CSWM) first identified the existing drainage channel has a limited conveyance capacity which results in flooding issues. In 2016, Tatham completed the *Guthrie Stormwater and Drainage Improvement*



Project Design Brief which analyzed hydrologic and hydraulic behavior of the area and identified the following drainage issues and recommendations:

- The existing stormwater management facility was deemed to be ineffective from a water quality control perspective as it does not include the extended detention or permanent pool necessary to remove accumulated phosphorus and sediment. It was also confirmed to have too much storage volume for the drainage area directed to it and could therefore be downsized;
- The existing channel from the Highway 11 culvert crossing upstream to the Melville Court/Line 5 intersection was deemed to be too flat in sections and not able to convey the expected design flows. This results in the more frequent flooding of private properties than desired. The proximity of the channel to the private properties in conjunction with the location of the SWM facility also provided a flow constraint;
- If the existing channel were to be relocated, improvements could be made to naturalize the channel better from a water quality conveyance perspective; and
- The existing (600 mm dia.) culvert between the existing channel, adjacent to the SWM facility that travels down the side lot lines to the Highway 11 roadside ditch, was deemed to be only of the capacity of the 5-year storm and result in backing up of flows for greater events which would increase the frequency of local flooding.

It was later determined that re-grading and re-aligning of the existing drainage channel would provide negligible benefits to reducing flood issues in the area. Therefore, this option was not proposed.

As such, best efforts will be provided to address existing flooding issues and water quantity treatment control measures and demonstrate how the proposed development will not negatively impact the existing flooding issues.

4.3.3 Existing Hydrology

For the purpose of generating existing condition peak flow rates, 6 catchments (Catchment 101, 102, A1, A1B, A5, & A6) upstream of the existing SWM facility (Outlet #1) and the existing drainage channel (Outlet #2) have been delineated and assessed. Visual OTTHYMO 6.2 (VO) hydrologic modelling software was used to generate peak flows contributing to the SWM facility and drainage channel for the 1:2-year through 1:100-year 4-Hour Chicago (CHI) and 24-hour SCS Type II (SCS) storm distributions. Site specific intensity-duration frequency (IDF) data from the MTO IDF Curve Look-up tool has been utilized as per Township Standards.

Catchment 101 is approximately 56.0 ha and comprised of farmland, woodland, a small impervious area and a portion of the subject lands and drains to the existing drainage channel,



bypassing the existing SWM facility. Catchment 102 is approximately 6.4 ha and includes the majority of the subject lands and comprises of pastureland cover and drains to the existing dry pond via existing ditching within Melville Court. 3.5 ha of external drainage area, originally delineated in support of the Hastings Subdivision development (Catchments A1, A1B, A5, and A6) also drain to the existing dry pond. Applied design parameters, ID's and areas for these catchments have been taken directly from the Hastings Drainage Plan.

The total drainage area contributing to Outlet #1 is 9.9 ha which is consistent with the Hasting Subdivision Storm Drainage Plan while the total drainage area contributing to Outlet #2 is approximately 65.9 ha. Stage storage information including existing outlet structure details quantity storage volumes within the existing SWM facility has been generated using detailed topographic survey information completed by C.C. Tatham & Associated Ltd. in 2015.

Existing drainage patterns are depicted on the Pre-Development Drainage Plan Drawing (DP-1), and the Overall Existing Drainage Plan (Drawing DP-3), provided in Appendix F.

A summary of existing conditions peak flow rates and storage volumes at Outlet #1 is provided in Table 1. A summary of existing peak flow rates at Outlet #2 is provided in Table 2. Hydrologic model results are provided in Appendix C.

Table 1: Existing Conditions SWM facility Operating Characteristics Summary – Outlet #1

| STORM EVENT | PEAK FLOWS (VO NODE 401) (m ³ /s) | | STORAGE VOLUMES (m ³) | | WATER SURFACE ELEVATION (m) | |
|-------------|--|-------------|--------------------------------------|-------------|--------------------------------|-------------|
| | 4-HOUR CHICAGO | 24-HOUR SCS | 4-HOUR CHICAGO | 24-HOUR SCS | 4-HOUR CHICAGO | 24-HOUR SCS |
| 1:2-Year | 0.07 | 0.12 | 96 | 175 | 287.55 | 287.60 |
| 1:5-Year | 0.10 | 0.15 | 145 | 333 | 287.58 | 287.68 |
| 1:10-Year | 0.13 | 0.16 | 184 | 503 | 287.60 | 287.77 |
| 1:25-Year | 0.14 | 0.19 | 264 | 780 | 287.65 | 287.91 |
| 1:50-Year | 0.15 | 0.21 | 359 | 1,028 | 287.70 | 288.03 |
| 1:100-Year | 0.16 | 0.22 | 486 | 1,294 | 287.76 | 288.15 |

Under existing conditions, the existing dry pond attenuates peak flows to a maximum release rate of 0.23 m³/s during the 1:100-year 24-Hour SCS storm event. The maximum storage volume required to attenuate the 1:100-year peak flow is 1,294 m³ which occurs at an elevation of



288.15 m, providing approximately 0.83 m of freeboard. Considering the required freeboard depth above the design high water level is 0.30 m per MECP standards, there is 0.53 m of additional quantity storage depth available within the existing SWM facility which can be utilized by providing greater peak flow attenuation.

Table 2: Existing Conditions Peak Flow Summary - Outlet #2

| STORM EVENT | PEAK FLOWS (VO NODE 902) (m ³ /s) | |
|------------------|---|-------------|
| | 4-HOUR CHICAGO | 24-HOUR SCS |
| 1:2-Year | 0.20 | 0.50 |
| 1:5-Year | 0.37 | 0.88 |
| 1:10-Year | 0.52 | 1.15 |
| 1:25-Year | 0.73 | 1.52 |
| 1:50-Year | 0.89 | 1.82 |
| 1:100-Year | 1.06 | 2.12 |
| Regional (Hazel) | 5.91 | |

Note 1: Regional storm (Hazel) peak flow referenced in this table is the uncontrolled condition under AMC III conditions (i.e. no SWM facility and no infiltration).

The existing condition peak flow rate at Outlet #2 is 2.12 m³/s under the 24-Hour SCS 1:100-year storm event. This represents the total peak flow in the channel downstream of the SWM pond from external drainage areas and controlled flows from the pond.

4.4 PROPOSED CONDITIONS

Under proposed conditions, the majority of the proposed development will continue to drain to the existing SWM facility and has been accounted for in Catchment 202. A site imperviousness of 20.0% has been applied to the subject lands which includes the total rooftop and driveway areas and an allowance for construction of accessory buildings and structures based on our review of the conceptual draft plan and Township Zoning By-law. Minor and major storm event runoff from Catchment 202 will flow overland, to the existing dry pond via existing ditches. The remaining site area which fronts Line 5 North will drain uncontrolled to the existing drainage channel and has been accounted for in Catchment 201. External drainage areas A1, A1B, A5 & A6 will be maintained under proposed conditions.



Due to the proposed development, the total drainage area contributing to the existing SWM facility increases by 0.1 ha while the total drainage area contributing to the existing drainage channel increases by 0.3 ha. Post-development drainage patterns and outlet locations are depicted on the Post-Development Drainage Plan (Drawing DP-2), appended.

4.4.1 Proposed Hydrology

A post-development hydrology model has been prepared using VO hydrologic modelling software to determine peak flows rates at Outlet #1 and Outlet #2. Two hydrologic scenarios have been modelled to assess how the existing SWM facility would function under proposed conditions and determine how a retrofit to the facility would improve its operating efficiency.

Scenario #1 - No Change to Existing SWM Facility

Under Scenario #1, the existing SWM facility will be maintained. A summary of peak flows and storage volumes at Outlet #1 is provided in Table 3 below while a summary of peak flows at Outlet #2 is provided in Table 4. Additional details are provided in Appendix C.

Table 3: Proposed Conditions Operating Characteristics Summary - Outlet #1

| STORM EVENT | PEAK FLOW (VO NODE 401) (m ³ /s) | | STORAGE VOLUME (m ³) | | WATER SURFACE ELEVATION (m) | |
|-------------|---|-------------|----------------------------------|-------------|-----------------------------|-------------|
| | 4-HOUR CHICAGO | 24-HOUR SCS | 4-HOUR CHICAGO | 24-HOUR SCS | 4-HOUR CHICAGO | 24-HOUR SCS |
| 1:2-Year | 0.07 | 0.13 | 97 | 191 | 287.55 | 287.60 |
| 1:5-Year | 0.11 | 0.16 | 150 | 450 | 287.58 | 287.74 |
| 1:10-Year | 0.13 | 0.18 | 196 | 698 | 287.61 | 287.87 |
| 1:25-Year | 0.15 | 0.21 | 351 | 1,058 | 287.69 | 288.04 |
| 1:50-Year | 0.17 | 0.23 | 508 | 1,369 | 287.77 | 288.18 |
| 1:100-Year | 0.18 | 0.25 | 683 | 1,693 | 287.86 | 288.33 |

Under Scenario #1 the existing SWM facility will attenuate peak flows to a maximum release rate of 0.25 m³/s during the 1:100-year 24-Hour SCS storm event which is marginally greater than existing conditions. The maximum storage volume required to attenuate the 1:100-year peak flow is 1,693 m³ which occurs at an elevation of 288.33 m, providing 0.65 m of freeboard.



Therefore, under proposed conditions, the existing SWM facility would not operate efficiently as there would be 0.35 m of additional quantity storage depth available in the pond.

Table 4: Proposed Conditions Peak Flow Summary - Outlet #2

| STORM EVENT | PEAK FLOWS (VO NODE 902) (m ³ /s) | |
|------------------|---|-------------|
| | 4-HOUR CHICAGO | 24-HOUR SCS |
| 1:2-Year | 0.21 | 0.54 |
| 1:5-Year | 0.40 | 0.92 |
| 1:10-Year | 0.56 | 1.20 |
| 1:25-Year | 0.76 | 1.58 |
| 1:50-Year | 0.93 | 1.88 |
| 1:100-Year | 1.10 | 2.20 |
| Regional (Hazel) | 6.03 | |

Note 1: Regional storm (Hazel) peak flow referenced in this table is the uncontrolled condition under AMC III conditions (i.e. no SWM facility and no infiltration).

As shown in Table 4, the proposed condition peak flow rate at Outlet #2 would marginally increase to 2.20 m³/s during the 24-Hour SCS 1:100-year storm event under Scenario #1.

To improve the existing drainage deficiencies noted in the area and the operating efficiency of the existing SWM facility, there is opportunity to implement a small retrofit to the pond involving modification to the outlet control pipe, which will result in reduction of peak flow rates from the pond in comparison to existing conditions by utilizing the additional quantity storage volume available in the pond. The preliminary retrofit design and a summary of peak flow rates and storage volumes is discussed further below.

Scenario #2 - Pond Retrofit Works

To provide more peak flow attenuation and maximize the available storage volume within the existing dry pond, thereby improving overall drainage conditions, a 242 mm dia. orifice plate attached to the existing 300 mm dia. outlet pipe at elevation 287.19 is proposed to be installed within a CSP riser structure. A summary of peak outflows and storage volumes within the retrofitted SWM facility at Outlet #1 is provided in Table 5 while a summary of peak flows at Outlet #2 is provided in Table 6. Additional details are provided in Appendix C.



Table 5: Proposed Conditions Operating Characteristics Summary - Outlet #1 with Retrofit

| STORM EVENT | PEAK FLOWS (VO NODE 401) (m ³ /s) | | STORAGE VOLUMES (m ³) | | WATER SURFACE ELEVATION (m) | |
|-------------|---|----------------------|-----------------------------------|-------------|-----------------------------|-------------|
| | 4-HOUR CHICAGO | 24-HOUR SCS | 4-HOUR CHICAGO | 24-HOUR SCS | 4-HOUR CHICAGO | 24-HOUR SCS |
| 1:2-Year | 0.05 (<i>0.07</i>) | 0.08 (<i>0.12</i>) | 141 | 392 | 287.58 | 287.71 |
| 1:5-Year | 0.08 (<i>0.10</i>) | 0.10 (<i>0.15</i>) | 293 | 835 | 287.66 | 287.94 |
| 1:10-Year | 0.09 (<i>0.13</i>) | 0.11 (<i>0.16</i>) | 480 | 1,197 | 287.76 | 288.11 |
| 1:25-Year | 0.10 (<i>0.14</i>) | 0.13 (<i>0.19</i>) | 760 | 1,699 | 287.90 | 288.33 |
| 1:50-Year | 0.11 (<i>0.15</i>) | 0.14 (<i>0.21</i>) | 1,006 | 2,118 | 288.02 | 288.51 |
| 1:100-Year | 0.12 (<i>0.16</i>) | 0.15 (<i>0.23</i>) | 1,270 | 2,555 | 288.14 | 288.68 |

Note 1: Values in italics represent existing condition peak flow rates

Table 6: Proposed Conditions Peak Flow Summary - Outlet #2 with Retrofit

| STORM EVENT | PEAK FLOWS (VO NODE 902) (m ³ /s) | |
|------------------|---|----------------------|
| | 4-HOUR CHICAGO | 24-HOUR SCS |
| 1:2-Year | 0.21 (<i>0.20</i>) | 0.50 (<i>0.50</i>) |
| 1:5-Year | 0.39 (<i>0.37</i>) | 0.86 (<i>0.88</i>) |
| 1:10-Year | 0.52 (<i>0.52</i>) | 1.13 (<i>1.15</i>) |
| 1:25-Year | 0.71 (<i>0.73</i>) | 1.49 (<i>1.52</i>) |
| 1:50-Year | 0.87 (<i>0.89</i>) | 1.79 (<i>1.82</i>) |
| 1:100-Year | 1.03 (<i>1.06</i>) | 2.09 (<i>2.12</i>) |
| Regional (Hazel) | 6.03 (<i>5.91</i>) | |

Note 1: Values in italics represent existing condition peak flow rates

Note 2: Regional storm (Hazel) peak flow referenced in this table is the uncontrolled condition under AMC III conditions (i.e. no SWM facility and no infiltration).



As shown in Table 5, the 1:100-year 24-Hour SCS storm event peak flow is reduced by approximately 65% from 0.23 to 0.15 m³/s due to the proposed retrofit. The maximum storage volume under the 1:100-year 24-Hour SCS storm event is increased from 1,293 m³ to 2,555 m³ (at elevation 288.68 m) providing 0.30 m of freeboard. It is noted that peak flows at Outlet #2 marginally increase during the 1:2-year through 1-5:year 4-Hour Chicago storm return periods but are reduced for all remaining return periods, which represents best efforts and is therefore sufficient.

An emergency overflow weir has been designed to provide safe conveyance of the uncontrolled peak flow out of the pond, in the event the outlet structure becomes blocked. The uncontrolled peak flow occurs during the 1:100-year 24-Hour SCS storm event at a rate of 1.02 m³/s. A 3.1 m wide overflow weir at elevation 288.68 conveys 1.05 m³/s at the top of pond elevation (elevation 288.98), providing safe conveyance of the uncontrolled peak flow in an emergency situation.

4.4.2 Water Quality

“Enhanced” Level 1 water quality control (corresponding to 80% TSS removal) is required for the developed areas of the site (driveway & accessory structures). Water quality controls are not required for rooftop areas as runoff generated from these areas is generally considered to be clean, and therefore, not subject to water quality controls.

Water quality controls for the portion of the site draining to Outlet #1 will be provided via a treatment train comprised of vegetated filter strips, enhanced swales/ditches within the Melville Court right-of-Way (ROW) and the existing SWM facility. TSS removal efficiencies of vegetated filter strips and enhanced swales/ditches have been applied based on those outlined in the LID SWM Planning and Design Guide published by Sustainable Technologies Evaluation Program (STEP). A water quality assessment of the SWM facility has been completed per Section 3.3.2 of the MECP SWM Planning and Design Manual which determined that based on an impervious level of 19.6%, the required storage volume for basic level protection is 488 m³ which is provided within the retrofitted SWM facility at elevation 287.76. This exceeds the 25 mm storm runoff volume of 429 m³ and therefore governs. It was determined that the equivalent drawdown time of the extended detention volume is less than 24-hours however, considering that existing operating characteristics of the SWM facility will be improved due to the proposed retrofit, the low level of imperviousness of the contributing drainage area and, that pre-treatment will be provided upstream, adequate water quality treatment is expected to be provided for the proposed development. As such, a combined TSS removal rate of 84% has been applied for Outlet #1. Additional analysis of the function of the SWM facility from a quality control perspective will be provided at detailed design.



Water quality controls for the portion of the site draining to Outlet #2 will be provided via a treatment train comprised of an enhanced lot-line swales and enhanced swales/ditches within Line 5 North. Based on review of Peto's geotechnical report, low permeable soils and raised groundwater levels are understood to be present. As such, to be conservative, a combined TSS removal rate 64% has been applied for Outlet #2. The 0.33 ha of open space area (Block 17) at the northwest quadrant of the site will remain predominately pervious and relatively undisturbed due to the proposed development and, therefore, is not subject to water quality controls. Therefore, the equivalent TSS removal rate for the proposed development is 79% which represents best efforts.

Supporting calculations are provided in Appendix C while additional details will be provided at detailed design.

4.4.3 LSRCA Volume Control & LID Design

In accordance with LSRCA requirements, projects defined as 'major development' are required to meet the volume control requirements outlined in Section 2.2.2 of the LSRCA 2016 SWM guidelines. As such, best efforts must be provided to infiltrate, filter, or re-use runoff generated from impervious areas on site.

Due to multiple site constraints (presented in Section 2.2.2.2 of the LSRCA SWM Guidelines) the site will not be able to provide runoff volume control for the 25 mm runoff volume. The constraints consist of:

- zoning, setbacks or other land use requirements;
- high groundwater levels;
- property or infrastructure restrictions; and
- excessive cost.

As such, an alternative treatment option implemented via low impact development (LID) practices has been proposed to infiltrate the equivalent 5 mm storm event runoff volume from impervious areas on site in accordance with LSRCA guidelines.

Considering a total site impervious area of approximately 1.25 ha, the equivalent single-event 5 mm storm event volume is approximately 62.5 m³. 14 infiltration trenches comprised of clear stone have been sized to infiltrate the 25 mm storm event runoff from 50% of rooftop area. Trenches are proposed within lots 1 to 14 where static groundwater levels are understood to be suitable for infiltration-based controls. As mentioned, raised groundwater levels and lower permeable soils are understood to occur at lots 15 & 16 and therefore, infiltration based practices have not been proposed in these locations. In addition, based on review of Peto's geotechnical



report, the estimated hydraulic conductivity rates vary greatly across the site from 1×10^{-3} (high permeability) and 1×10^{-6} (low permeability). Therefore, underdrains which will discharge to the Melville Court ditch may be required for lots with low permeability (1 through 4). During detailed design, infiltration rates for each proposed LID device will be confirmed via in-situ infiltration testing.

As such, considering a single rooftop area of 326 m², the equivalent 25 mm storm event volume is approximately 4.1 m³. Therefore, the total infiltration volume provided is approximately 57.4 m³. Additional infiltration measures including enhancements to the existing Melville Court ditch, enhanced lot line swales and/or vegetated filter strips within the front yards and directing downspouts to pervious areas will also be provided and are expected to provide, at minimum, the remaining 5.1 m³ of infiltration volume.

Approximate locations of the proposed infiltration trenches are provided on Drawing SSG-1. Additional details will be provided at the detailed design stage.

4.4.4 Water Balance

A preliminary water balance assessment has been completed by Peto MacCallum Ltd. to assess the potential impact of the development on local groundwater and surface water resources. Their analysis determined the annual pre-development infiltration volume for the site is 15,437 m³/year. The increase in impervious area under post development conditions results in approximately 13,960 m³/year of infiltration, resulting in an infiltration deficit of approximately 1,477 m³/year.

As mentioned, best efforts will be provided to infiltrate the equivalent 5 mm storm event volume from all impervious areas. Based on review of historical rainfall data for the Shanty Bay rainfall station, the 5 mm storm event accounts for approximately 50% of rainfall events, which is approximately equivalent to 484 mm of the average annual rainfall. Therefore, the equivalent annual site rainfall capture is approximately 6,050 m³/year (1.25 ha x 484 mm). This mitigates the infiltration deficit and exceeds water balance requirements.

Refer to the original report for additional information.

4.4.5 Phosphorus Budget

In order to comply with LSPOP requirements, a phosphorus budget for the site has been completed using the loading rates and removal efficiencies from the MECP Phosphorus Budget tool, for the Oro Creeks South Subwatershed, through a spreadsheet method, summarized below in Table 3.



Existing Conditions

Under existing conditions, the site has been modelled as hay-pasture and forest land-uses for the purpose of the phosphorus budget calculations.

Applying the relevant loading rate of 0.07 kg/ha/year for hay-pasture and 0.05 kg/ha/year for forest, the pre-development phosphorus load is 0.44 kg/year.

Proposed Conditions

Under proposed conditions, the site has also been modelled as a single land-use category (low intensity development) with a loading rate of 0.13 kg/ha/year. As such the post-development phosphorous load, without mitigation is 0.85 kg/year.

Phosphorous mitigation will be provided via a treatment train comprised of infiltration trenches, enhanced grassed swales and/or vegetated filter strips and enhanced swales/ditches and dry pond with removal efficiency rates, outlined in the LID SWM Planning and Design Guide published by STEP, of 60%, 20%, 25% and 10% respectively.

As such, applying the relevant loading and removal efficiency rates, the post-development phosphorus load with controls is 0.47 kg/year.

A summary of the phosphorus loading rates for each scenario is provided in Table 7. Additional details and outputs are provided in Appendix D. As shown, phosphorus loading is reduced under proposed conditions.

Table 7: Phosphorus Loading Summary

| SCENARIO | AREA (ha) | PHOSPHORUS LOADING (kg/year) |
|-------------------------------------|-----------|------------------------------|
| Pre-Development | 6.58 | 0.44 |
| Post-Development (Without Controls) | 6.58 | 0.85 |
| Post-Development (With Controls) | 6.58 | 0.47 |



5 Siltation & Erosion Control Plan

Siltation and erosion controls will be implemented for all construction activities, including demolition, earthworks, material stockpiling, pavement construction, and grading operations. Details of the sedimentation and erosion control will be provided during detailed design and are summarized as follows:

- heavy duty silt control fences will be erected to control sediment movement to abutting properties and the Melville Court and Line 5 North right-of-ways;
- a stone mud mat will be installed at the construction entrances from Melville Court and Line 5 North; and
- regular inspection of control measures will be implemented and repairs made as necessary during construction.



6 Grading & Landscaping

The grading of the proposed development will match to the existing grades along the limits of the development. The site will be graded to suit the existing boundary conditions surrounding the site. Refer to the Drawing SSG-1 provided in Appendix F for additional details.



7 Utilities

All appropriate utility companies (electrical, gas, telecommunications) have been contacted. Enbridge has a gas main on the south side of Melville Court; they would need to install a main on the north side of the street to avoid 14 long services. Currently, there is no gas main in front of lots 15 and 16, but they are serviceable with a gas main extension. Bell has copper cable servicing the houses on the south side of Melville Court. They will likely provide copper cable to the new houses. Rogers states that the subdivision requires new fibre build and new node for servicing. They only have existing fibre support for commercial units in this area. We are awaiting a response from HydroOne. Correspondence with these companies can be found in Appendix E. Further coordination will be required during the detailed design stage.



8 Summary

8.1 WATER SUPPLY & DISTRIBUTION

The site will be serviced by private wells for each lot.

8.2 SANITARY SEWER COLLECTION

The site will be serviced by individual septic bed systems. Two options for septic bed systems have been provided based on in-situ soil T-times. Detailed design of the septic beds will be completed at the detailed design and/or building permit application stage.

8.3 STORMWATER MANAGEMENT PLAN

The SWM plan demonstrates the proposed development will not result in negative impacts with respect to stormwater. Peak flow attenuation and water quantity controls for the proposed development will be provided within the existing SWM facility, which will be retrofitted with a proposed 242 mm dia. orifice plate attached to the 300 mm dia. outlet pipe and installed within a CSP riser structure to further reduce pond outflows, prevent outlet blockage and improve downstream drainage conditions. Water quality controls will be provided via a treatment train comprised of vegetated filter strips, enhanced swales/ditches and the existing SWM facility. Lot-level infiltration controls will be implemented to achieve runoff volume control, phosphorus mitigation and achieve water balance requirements.

8.4 SILTATION & EROSION CONTROL

Siltation and erosion controls will be provided with the proper construction mitigation efforts.

8.5 GRADING & LANDSCAPING

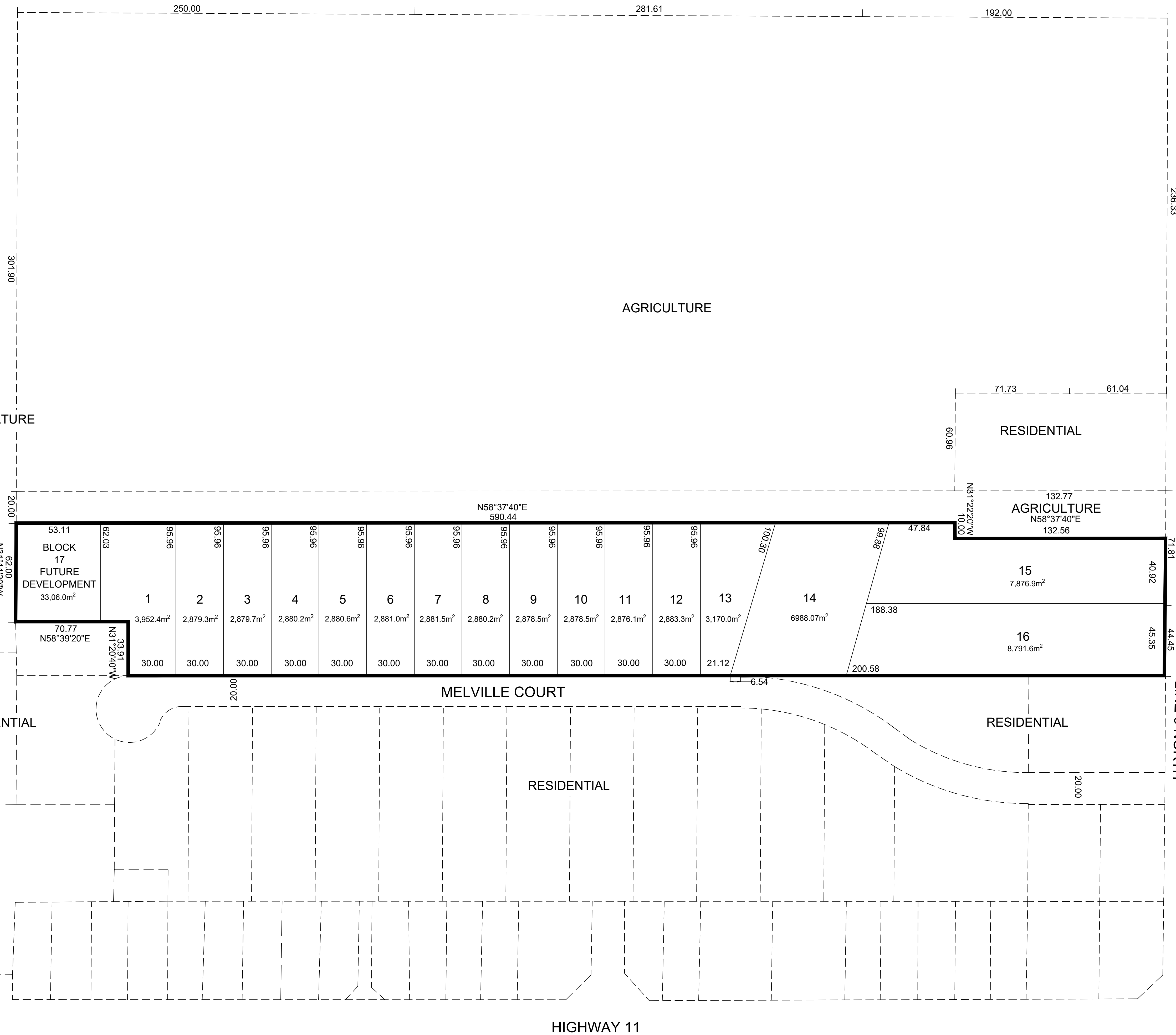
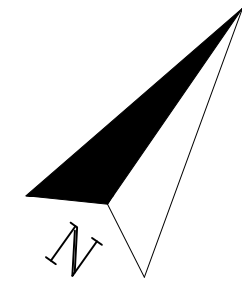
The grading of the proposed development will match to existing perimeter grades along the limits of the development.

8.6 UTILITIES

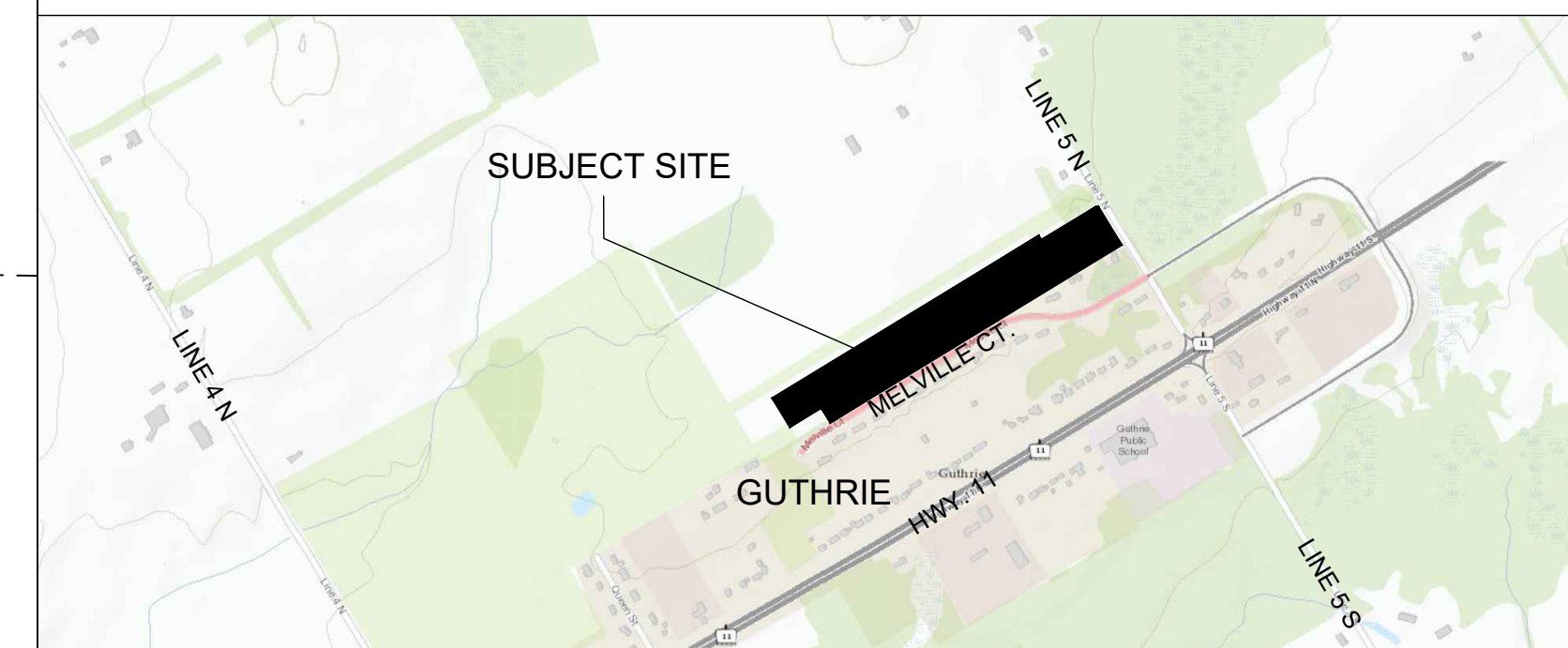
All utilities (electrical, gas, telecommunications) are expected to be available from Melville Court and Line 5 North to service the proposed development.



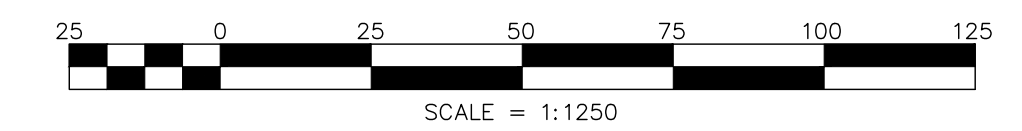
Appendix A: Draft Plan



KEY MAP



DRAFT PLAN OF SUBDIVISION
 EAST HALF OF LOT 20, CONCESSION 5
 FORMERLY IN THE
 TOWNSHIP OF ORO
 NOW IN THE
 TOWNSHIP OF ORO-MEDONTE
 COUNTY OF SIMCOE
 2010



LEGEND

SUBJECT LANDS

OWNER'S CERTIFICATE

I, THE UNDERSIGNED, BEING THE REGISTERED OWNER OF THE SUBJECT LANDS, HEREBY AUTHORIZE INNOVATIVE PLANNING SOLUTIONS, TO PREPARE THIS DRAFT PLAN OF SUBDIVISION AND TO SUBMIT SAME TO THE COUNTY OF SIMCOE FOR APPROVAL.

DATE _____ DONCOR DEVELOPMENT INC.

SURVEYOR'S CERTIFICATE

I CERTIFY THAT THE BOUNDARIES OF THE LANDS TO BE SUBDIVIDED AND THEIR RELATIONSHIP TO ADJACENT LANDS ARE ACCURATELY AND CORRECTLY SHOWN.

DATE _____

ADDITIONAL INFORMATION REQUIRED UNDER SECTION 51(17) OF THE PLANNING ACT

- | | |
|------------------|--|
| a) SHOWN ON PLAN | h) MUNICIPAL WATER |
| b) SEE KEY PLAN | i) SAND |
| c) SHOWN ON PLAN | j) SHOWN ON PLAN |
| d) RESIDENTIAL | k) ALL MUNICIPAL SERVICES TO BE PROVIDED |
| e) SHOWN ON PLAN | l) NONE |
| f) SHOWN ON PLAN | |
| g) SHOWN ON PLAN | |

LAND USE STATISTICS

| Land Use | Lot / Block No. | Area (ha.) | Units |
|------------------------|-----------------|------------|-------|
| RESIDENTIAL SINGLE LOT | 1-16 | 6.24 | 16 |
| FUTURE DEVELOPMENT | 17 | 0.33 | |
| TOTAL | | 6.57 | 16 |

SOURCE:

- 1.0m CONTOUR INTERVALS: FIRST BASE SOLUTIONS, VuMAP, 2020

SCHEDULE OF REVISIONS

| No. | Date | Description | By |
|-----|--------------|--------------|------|
| 1 | Jan. 3, 2022 | Application; | A.S. |
| | | | |
| | | | |
| | | | |

IPS INNOVATIVE PLANNING SOLUTIONS
 PLANNERS • PROJECT MANAGERS • LAND DEVELOPERS
 647 WELHAM ROAD, UNIT 9, BARRIE, ONTARIO, L4N 0B7
 tel: 705 • 812 • 3281 fax: 705 • 812 • 3438 e: info@ipsconsultinginc.com www.ipsconsultinginc.com

Date: Jan 3, 2022 Drawn By: A.S.
 File: 06-188 Checked: D.V. / J.H.

Appendix B: Sanitary Sewage System Calculations

| | | | |
|----------|---|-----------|------------------|
| PROJECT: | Melville Court Township of Oro-Medonte | FILE: | 420427 |
| | | DATE: | January 18, 2022 |
| SUBJECT: | On-Site Sewage System Design: In-ground Filter Bed Daily Design Flow and Septic Tank Sizing: Option #1 | DESIGNED: | LB |
| | | CHECKED: | MB |

HOUSE DESCRIPTION

| | | | | | |
|---------------------------------------|---------------|-----------------|--------------|--------------|----------|
| Number of Bedrooms: | Existing 0 | Proposed 4 | Total 4 | | |
| Finished Floor Area (m ²) | Existing 0 | Proposed 266 | Total 266 | | |
| Fixture Units (FU) | Existing | Proposed | Total | FU / Fixture | Total FU |
| Bathroom Group | 0 | 3 | 3.0 | 6.0 | 18.0 |
| Toilet (Water Closet) | 0 | 0 | 0.0 | 4.0 | 0.0 |
| Washbasin | 0 | 1 | 1.0 | 1.5 | 1.5 |
| Shower | 0 | 0 | 0.0 | 1.5 | 0.0 |
| Kitchen Sink(s) | 0 | 1 | 1.0 | 1.5 | 1.5 |
| Bar Sink | 0 | 0 | 0.0 | 1.5 | 0.0 |
| Dishwasher (not connected to sink) | 0 | 0 | 0.0 | 1.0 | 0.0 |
| Clothes Washer | 0 | 1 | 1.0 | 1.5 | 1.5 |
| Laundry Tub | 0 | 1 | 1.0 | 1.5 | 1.5 |
| Floor Drain | 0 | 1 | 1.0 | 1.0 | 1.0 |
| Other (If Applicable) | 0 | 0 | 0.0 | 0.0 | 0.0 |
| Other (If Applicable) | 0 | 0 | 0.0 | 0.0 | 0.0 |
| Total | | | | | 25.0 |

Notes: Fixture Units per fixture from OBC Table 7.4.9.3.
Number of bedrooms, fixture units, and floor area approximated based on email correspondence with IPS (December 16, 2021)

DAILY DESIGN FLOW

| | | | |
|-------------------------------|--------------------------------------|--|-------------|
| Base Design Flow (BDF) | OBC Table 8.2.1.3.A. | Based on No. of Bedrooms up to 4 | 2,000 L/day |
| Extra Bedroom Surcharge (EBS) | OBC Table 8.2.1.3.A. | 500 L/day for each bed room over 5 | 0 L/day |
| Floor Area Surcharge (FAS) | OBC Table 8.2.1.3.A. | 100L for each 10 m ² (or part of it) over 200 m ² but less than 400 m ² | 700 L/day |
| | | 75L for each 10 m ² (or part of it) over 400 m ² but less than 600 m ² | 0 L/day |
| | | 50L for each 10 m ² (or part of it) over 600 m ² | 0 L/day |
| | | Total | 700 L/day |
| Fixture Unit Surcharge (FUS) | OBC Table 8.2.1.3.A. | 50 L/day for each fixture unit over 20 fixture units | 250 L/day |
| Total Daily Design Flow (Q) | Q = BDF + maximum of EBS, FAS or FUS | | 2,700 L/day |

SEPTIC TANK

| | | | |
|-----------------------------|--|--------|---------|
| Min. Septic Tank Volume (V) | OBC 8.2.2.3.(a) | V = 2Q | 5,400 L |
| Use | 6,000 L septic tank by Brooklin Concrete (or approved equal) | | |

Filter Bed

Filter bed area calculated as follows: $A = Q/LR$

| | | | |
|-------|------|--------------------------|---|
| Where | A = | 36 m ² | Minimum area of filter bed media (Calculated value) |
| | Q = | 2,700 L/day | Daily Design Flow (Calculated above) |
| | LR = | 75 L/m ² /day | Loading Rate 75 L/m ² /day when Q ≤ 3,000 L/day or 50 L/m ² /day when Q > 3,000 L/day |



| | | | |
|----------|---|-----------|----------------------------------|
| PROJECT: | Melville Court Township of Oro-Medonte | FILE: | 420427 DATE: January 18, 2022 |
| SUBJECT: | On-Site Sewage System Design: In-ground Filter Bed Daily Design Flow and Septic Tank Sizing: Option #1 | DESIGNED: | LB CHECKED: MB |

Extended Contact Area

Filter Medium Base Area Calculated as follows: $BA = QT/850$ (ref OBC 8.7.5.3. (6))
(Expanded Contact Area)

Where $BA = 38 \text{ m}^2$ Minimum required loading area (Calculated Value)
 $Q = 2,700 \text{ L/day}$ Daily Design Flow (Calculated above)
 $T = 12 \text{ min/cm}$ T-Time from report by Peto MacCallum dated June 7, 2021

Loading Area $Q = 2,700 \text{ L/day}$ (Total Daily Design Flow)
 $LR = 10 \text{ L/m}^2/\text{day}$ Based on T time of native soil: $1 < T \leq 20$ and OBC Table 8.7.4.1.
 $= 270 \text{ m}^2$

| | | | |
|----------|--|-----------|------------------|
| PROJECT: | Melville Court Township of Oro-Medonte | FILE: | 420427 |
| | | DATE: | January 18, 2021 |
| SUBJECT: | On-site Sewage System Design: Partially Raised Filter Bed Daily Design Flow and Septic Tank Sizing: Option #2 | DESIGNED: | LB |
| | | CHECKED: | MB |

HOUSE DESCRIPTION

| | | | | | |
|---------------------------------------|---------------|-----------------|--------------|--------------|----------|
| Number of Bedrooms: | Existing 0 | Proposed 4 | Total 4 | | |
| Finished Floor Area (m ²) | Existing 0 | Proposed 266 | Total 266 | | |
| Fixture Units (FU) | Existing | Proposed | Total | FU / Fixture | Total FU |
| Bathroom Group | 0 | 3 | 3.0 | 6.0 | 18.0 |
| Toilet (Water Closet) | 0 | 0 | 0.0 | 4.0 | 0.0 |
| Washbasin | 0 | 1 | 1.0 | 1.5 | 1.5 |
| Shower | 0 | 0 | 0.0 | 1.5 | 0.0 |
| Kitchen Sink(s) | 0 | 1 | 1.0 | 1.5 | 1.5 |
| Bar Sink | 0 | 0 | 0.0 | 1.5 | 0.0 |
| Dishwasher (not connected to sink) | 0 | 0 | 0.0 | 1.0 | 0.0 |
| Clothes Washer | 0 | 1 | 1.0 | 1.5 | 1.5 |
| Laundry Tub | 0 | 1 | 1.0 | 1.5 | 1.5 |
| Floor Drain | 0 | 1 | 1.0 | 1.0 | 1.0 |
| Other (If Applicable) | 0 | 0 | 0.0 | 0.0 | 0.0 |
| Other (If Applicable) | 0 | 0 | 0.0 | 0.0 | 0.0 |
| Total | | | | | 25.0 |

Notes: Fixture Units per fixture from OBC Table 7.4.9.3.
Number of bedrooms, fixture units, and floor area approximated based on email correspondence with IPS (December 16, 2021)

DAILY DESIGN FLOW

| | | | |
|-------------------------------|--------------------------------------|--|--|
| Base Design Flow (BDF) | OBC Table 8.2.1.3.A. | Based on No. of Bedrooms up to 4 | 2,000 L/day |
| Extra Bedroom Surcharge (EBS) | OBC Table 8.2.1.3.A. | 500 L/day for each bedroom over 5 | 0 L/day |
| Floor Area Surcharge (FAS) | OBC Table 8.2.1.3.A. | 100L for each 10 m ² (or part of it) over 200 m ² but less than 400 m ² 75L for each 10 m ² (or part of it) over 400 m ² but less than 600 m ² 50L for each 10 m ² (or part of it) over 600 m ² Total | 700 L/day 0 L/day 0 L/day 700 L/day |
| Fixture Unit Surcharge (FUS) | OBC Table 8.2.1.3.A. | 50 L/day for each fixture unit over 20 fixture units | 250 L/day |
| Total Daily Design Flow (Q) | Q = BDF + maximum of EBS, FAS or FUS | | 2,700 L/day |

SEPTIC TANK

| | | | |
|-----------------------------|-----------------|--------|---------|
| Min. Septic Tank Volume (V) | OBC 8.2.2.3.(a) | V = 2Q | 5,400 L |
|-----------------------------|-----------------|--------|---------|

Use 6,000 L septic tank by Brooklin Concrete (or approved equal)

Filter Bed

Filter bed area calculated as follows: $A = Q/LR$

| | | | |
|-------|------|--------------------------|---|
| Where | A = | 36 m ² | Minimum area of filter bed media (Calculated value) |
| | Q = | 2,700 L/day | Daily Design Flow (Calculated above) |
| | LR = | 75 L/m ² /day | Loading Rate 75 L/m ² /day when Q ≤ 3,000 L/day or 50 L/m ² /day when Q > 3,000 L/day |



| | | | |
|----------|--|-----------|------------------|
| PROJECT: | Melville Court Township of Oro-Medonte | FILE: | 420427 |
| | | DATE: | January 18, 2021 |
| SUBJECT: | On-site Sewage System Design: Partially Raised Filter Bed Daily Design Flow and Septic Tank Sizing: Option #2 | DESIGNED: | LB |
| | | CHECKED: | MB |

Extended Contact Area

Filter Medium Base Area Calculated as follows: $BA = QT/850$ (ref OBC 8.7.5.3. (6))
(Expanded Contact Area)

Where $BA = 111 \text{ m}^2$ Minimum required loading area (Calculated Value)
 $Q = 2,700 \text{ L/day}$ Daily Design Flow (Calculated above)
 $T = 35 \text{ min/cm}$ T-Time from report by Peto MacCallum dated June 7, 2021

Loading Area $Q = 2,700 \text{ L/day}$ (Total Daily Design Flow)
 $LR = 8 \text{ L/m}^2/\text{day}$ Based on T time of native soil: $20 < T \leq 35$ and OBC Table 8.7.4.1.
 $= 338 \text{ m}^2$

Appendix C: Stormwater Management Calculations

Existing Conditions Hydrology

Visual OTTHYMO Model Parameter Calculations (NasHYD)

Project Details

| | |
|----------------|--------|
| Melville Court | 420427 |
|----------------|--------|

Data Sources

Detailed Soil Survey Reports for Ontario, LSRCA Technical Guidelines for Stormwater Management Submissions (2016), MTO Drainage Management Manual (1997)

Prepared By

| | |
|----|------------|
| LB | 12-15-2021 |
|----|------------|

Pre-Development Condition

| | |
|----------------------|-------|
| Watershed: | LSRCA |
| Catchment ID: | 101 |
| Catchment Area (ha): | 56.01 |
| Impervious %: | 5% |

Average Curve Number (CN), Runoff Coefficient (C) and Initial Abstraction (IA)

| Soil Symbol | Gus | | | | | | | | | | | | | |
|-------------------------|-----------|--------|----|------|--------|----|---|--------|----|---|--------|----|---|--|
| Soil Series | Guerin | | | | | | | | | | | | | |
| Hydrologic Soils Group | AB | | | | | | | | | | | | | |
| Soil Texture | Sand Loam | | | | | | | | | | | | | |
| Runoff Coefficient Type | 1 | | | | | | | | | | | | | |
| Area (ha) | 56.01 | | | | | | | | | | | | | |
| Percentage of Catchment | 100% | | | | | | | | | | | | | |
| Land Cover Category | IA | A (ha) | CN | C | A (ha) | CN | C | A (ha) | CN | C | A (ha) | CN | C | |
| Impervious | 2 | 2.83 | 98 | 0.95 | | | | | | | | | | |
| Gravel | 3 | | 81 | 0.09 | | | | | | | | | | |
| Woodland | 10 | 22.64 | 46 | 0.08 | | | | | | | | | | |
| Pasture/Lawns | 5 | | 59 | 0.10 | | | | | | | | | | |
| Meadows | 8 | | 51 | 0.09 | | | | | | | | | | |
| Cultivated | 7 | 30.54 | 68 | 0.22 | | | | | | | | | | |
| Waterbody | 12 | | 50 | 0.05 | | | | | | | | | | |
| Average CN | 60.62 | | | | | | | | | | | | | |
| Average C | 0.20 | | | | | | | | | | | | | |
| Average IA | 7.96 | | | | | | | | | | | | | |

Time to Peak Calculations

| | |
|-------------------------------|----------------|
| Max. Catchment Elev. (m): | 304.00 |
| Min. Catchment Elev. (m): | 288.00 |
| Catchment Length (m): | 1400 |
| Catchment Slope (%): | 1.14% |
| Method: | Airport Method |
| Time of Concentration (mins): | 105.01 |

Summary

| | |
|-------------------------------|-------|
| Catchment CN: | 60.6 |
| Catchment C: | 0.20 |
| Catchment IA (mm): | 7.96 |
| Time of Concentration (hrs): | 1.75 |
| Catchment Time to Peak (hrs): | 1.17 |
| Catchment Time Step (mins): | 14.00 |

Visual OTTHYMO Model Parameter Calculations (NasHYD)

Project Details

| | |
|----------------|--------|
| Melville Court | 420427 |
|----------------|--------|

Data Sources

Detailed Soil Survey Reports for Ontario, LSRCA Technical Guidelines for Stormwater Management Submissions (2016), MTO Drainage Management Manual (1997)

Prepared By

| | |
|----|------------|
| LB | 12-15-2021 |
|----|------------|

Pre-Development Condition

| | |
|----------------------|-------|
| Watershed: | LSRCA |
| Catchment ID: | 102 |
| Catchment Area (ha): | 6.35 |
| Impervious %: | |

Average Curve Number (CN), Runoff Coefficient (C) and Initial Abstraction (IA)

| Soil Symbol | Vas1 | | | | | | | | | | | | | |
|-------------------------|-----------|--------|----|------|--------|----|---|--------|----|---|--------|----|---|--|
| Soil Series | Vasey | | | | | | | | | | | | | |
| Hydrologic Soils Group | AB | | | | | | | | | | | | | |
| Soil Texture | Sand Loam | | | | | | | | | | | | | |
| Runoff Coefficient Type | 1 | | | | | | | | | | | | | |
| Area (ha) | 6.35 | | | | | | | | | | | | | |
| Percentage of Catchment | 100% | | | | | | | | | | | | | |
| Land Cover Category | IA | A (ha) | CN | C | A (ha) | CN | C | A (ha) | CN | C | A (ha) | CN | C | |
| Impervious | 2 | | 98 | 0.95 | | | | | | | | | | |
| Gravel | 3 | | 81 | 0.09 | | | | | | | | | | |
| Woodland | 10 | | 46 | 0.08 | | | | | | | | | | |
| Pasture/Lawns | 5 | 6.35 | 59 | 0.10 | | | | | | | | | | |
| Meadows | 8 | | 51 | 0.09 | | | | | | | | | | |
| Cultivated | 7 | | 68 | 0.22 | | | | | | | | | | |
| Waterbody | 12 | | 50 | 0.05 | | | | | | | | | | |
| Average CN | 59.00 | | | | | | | | | | | | | |
| Average C | 0.10 | | | | | | | | | | | | | |
| Average IA | 5.00 | | | | | | | | | | | | | |

Time to Peak Calculations

| | |
|-------------------------------|----------------|
| Max. Catchment Elev. (m): | 298.00 |
| Min. Catchment Elev. (m): | 288.52 |
| Catchment Length (m): | 900 |
| Catchment Slope (%): | 1.05% |
| Method: | Airport Method |
| Time of Concentration (mins): | 96.14 |

Summary

| | |
|-------------------------------|-------|
| Catchment CN: | 59.0 |
| Catchment C: | 0.10 |
| Catchment IA (mm): | 5.00 |
| Time of Concentration (hrs): | 1.60 |
| Catchment Time to Peak (hrs): | 1.07 |
| Catchment Time Step (mins): | 12.82 |

Visual OTTHYMO Model Parameter Calculations (NasHYD)

Project Details

| | |
|----------------|--------|
| Melville Court | 420427 |
|----------------|--------|

Data Sources

Detailed Soil Survey Reports for Ontario, LSRCA Technical Guidelines for Stormwater Management Submissions (2016), MTO Drainage Management Manual (1997)

Prepared By

| | |
|----|------------|
| LB | 12-15-2021 |
|----|------------|

Pre-Development Condition

| | |
|----------------------|-------|
| Watershed: | LSRCA |
| Catchment ID: | A6 |
| Catchment Area (ha): | 0.58 |
| Impervious %: | 10% |

Average Curve Number (CN), Runoff Coefficient (C) and Initial Abstraction (IA)

| Soil Symbol | Stsl-s | | | | | | | | | | | | |
|-------------------------|-----------|--------|----|------|--------|----|---|--------|----|---|--------|----|---|
| Soil Series | Sargent | | | | | | | | | | | | |
| Hydrologic Soils Group | AB | | | | | | | | | | | | |
| Soil Texture | Sand Loam | | | | | | | | | | | | |
| Runoff Coefficient Type | 1 | | | | | | | | | | | | |
| Area (ha) | 0.58 | | | | | | | | | | | | |
| Percentage of Catchment | 100% | | | | | | | | | | | | |
| Land Cover Category | IA | A (ha) | CN | C | A (ha) | CN | C | A (ha) | CN | C | A (ha) | CN | C |
| Impervious | 2 | | 98 | 0.95 | | | | | | | | | |
| Gravel | 3 | | 81 | 0.14 | | | | | | | | | |
| Woodland | 10 | | 46 | 0.12 | | | | | | | | | |
| Pasture/Lawns | 5 | | 59 | 0.15 | | | | | | | | | |
| Meadows | 8 | | 51 | 0.14 | | | | | | | | | |
| Cultivated | 7 | | 68 | 0.30 | | | | | | | | | |
| Waterbody | 12 | | 50 | 0.05 | | | | | | | | | |
| Average CN | 74.00 | | | | | | | | | | | | |
| Average C | | | | | | | | | | | | | |
| Average IA | | | | | | | | | | | | | |

Time to Peak Calculations

| | |
|-------------------------------|----------------|
| Max. Catchment Elev. (m): | 288.00 |
| Min. Catchment Elev. (m): | 287.50 |
| Catchment Length (m): | 10 |
| Catchment Slope (%): | 5.00% |
| Method: | Airport Method |
| Time of Concentration (mins): | 6.67 |

Summary

| | |
|-------------------------------|------|
| Catchment CN: | 74.0 |
| Catchment C: | |
| Catchment IA (mm): | 3.82 |
| Time of Concentration (hrs): | 0.11 |
| Catchment Time to Peak (hrs): | 0.07 |
| Catchment Time Step (mins): | 0.89 |

Active coordinate

44° 27' 45" N, 79° 34' 14" W (44.462500,-79.570833)

Retrieved: Wed, 12 Aug 2020 15:37:01 GMT



Location summary

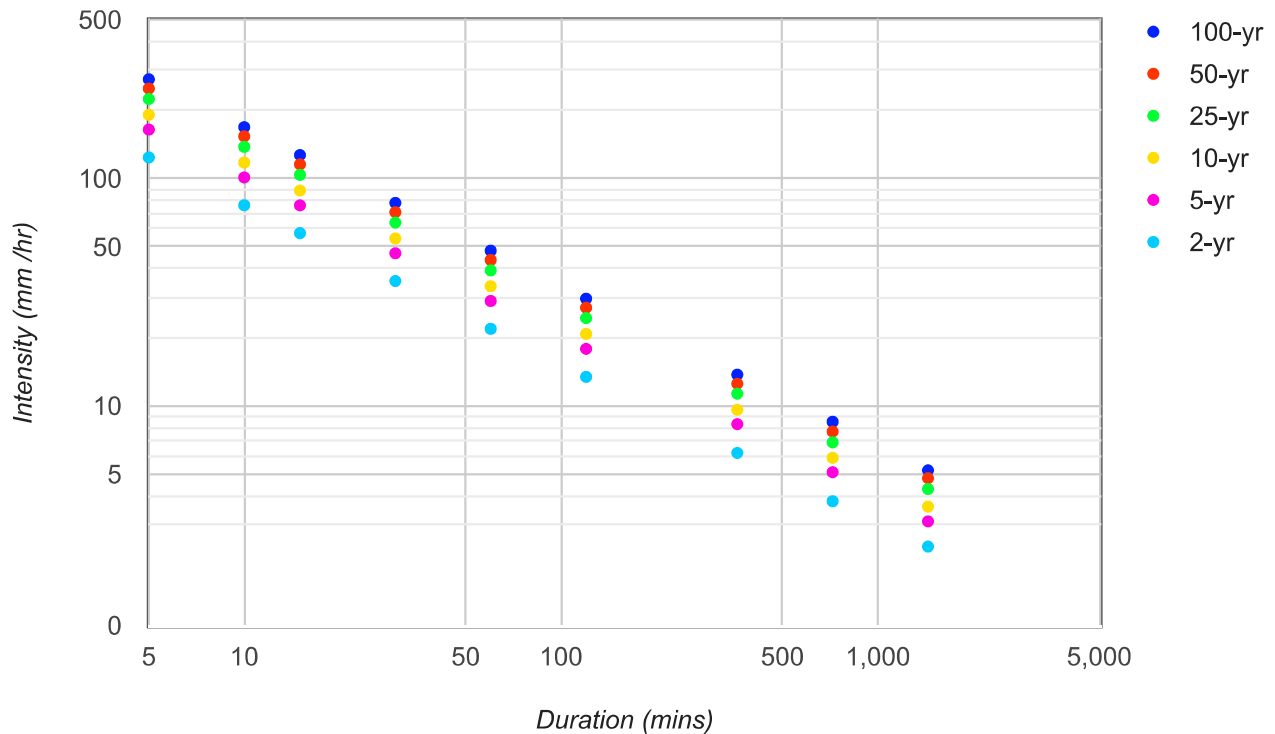
These are the locations in the selection.

IDF Curve: 44° 27' 45" N, 79° 34' 14" W (44.462500,-79.570833)

Results

An IDF curve was found.

Coordinate: 44.462500, -79.570833
IDF curve year: 2010



Coefficient summary

IDF Curve: 44° 27' 45" N, 79° 34' 14" W (44.462500,-79.570833)

Retrieved: Wed, 12 Aug 2020 15:37:01 GMT

Data year: 2010

IDF curve year: 2010

| Return period | 2-yr | 5-yr | 10-yr | 25-yr | 50-yr | 100-yr |
|---------------|--------|--------|--------|--------|--------|--------|
| A | 21.8 | 28.9 | 33.6 | 39.4 | 43.8 | 48.1 |
| B | -0.699 | -0.699 | -0.699 | -0.699 | -0.699 | -0.699 |

Statistics**Rainfall intensity (mm hr⁻¹)**

| Duration | 5-min | 10-min | 15-min | 30-min | 1-hr | 2-hr | 6-hr | 12-hr | 24-hr |
|---------------|-------|--------|--------|--------|------|------|------|-------|-------|
| 2-yr | 123.8 | 76.3 | 57.5 | 35.4 | 21.8 | 13.4 | 6.2 | 3.8 | 2.4 |
| 5-yr | 164.2 | 101.1 | 76.2 | 46.9 | 28.9 | 17.8 | 8.3 | 5.1 | 3.1 |
| 10-yr | 190.8 | 117.6 | 88.5 | 54.5 | 33.6 | 20.7 | 9.6 | 5.9 | 3.6 |
| 25-yr | 223.8 | 137.9 | 103.8 | 64.0 | 39.4 | 24.3 | 11.3 | 6.9 | 4.3 |
| 50-yr | 248.8 | 153.3 | 115.4 | 71.1 | 43.8 | 27.0 | 12.5 | 7.7 | 4.8 |
| 100-yr | 273.2 | 168.3 | 126.8 | 78.1 | 48.1 | 29.6 | 13.7 | 8.5 | 5.2 |

Rainfall depth (mm)

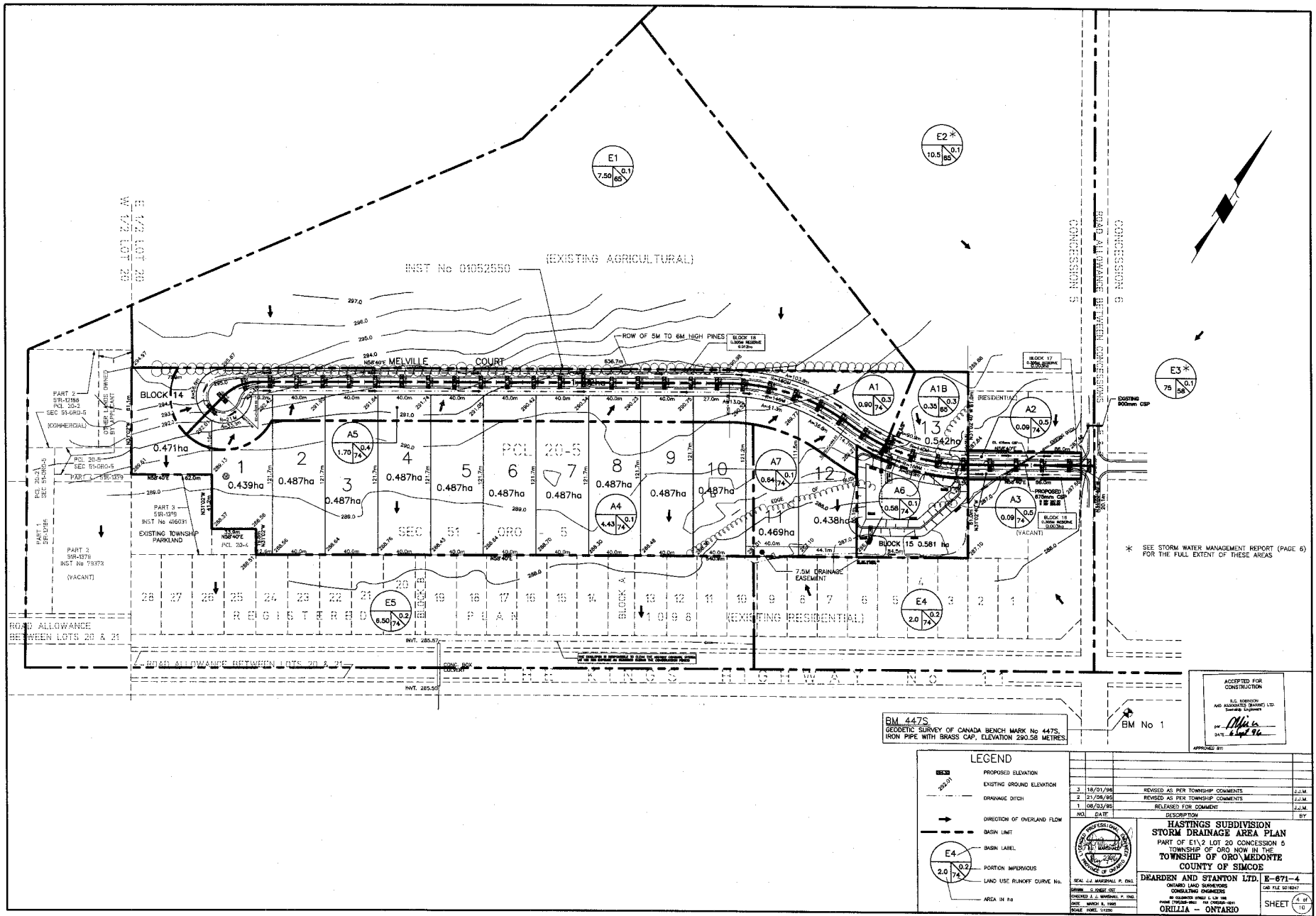
| Duration | 5-min | 10-min | 15-min | 30-min | 1-hr | 2-hr | 6-hr | 12-hr | 24-hr |
|---------------|-------|--------|--------|--------|------|------|------|-------|-------|
| 2-yr | 10.3 | 12.7 | 14.4 | 17.7 | 21.8 | 26.9 | 37.4 | 46.1 | 56.7 |
| 5-yr | 13.7 | 16.9 | 19.0 | 23.5 | 28.9 | 35.6 | 49.6 | 61.1 | 75.2 |
| 10-yr | 15.9 | 19.6 | 22.1 | 27.3 | 33.6 | 41.4 | 57.6 | 71.0 | 87.5 |
| 25-yr | 18.6 | 23.0 | 26.0 | 32.0 | 39.4 | 48.5 | 67.6 | 83.2 | 102.6 |
| 50-yr | 20.7 | 25.5 | 28.9 | 35.6 | 43.8 | 54.0 | 75.1 | 92.5 | 114.0 |
| 100-yr | 22.8 | 28.0 | 31.7 | 39.0 | 48.1 | 59.3 | 82.5 | 101.6 | 125.2 |

Terms of Use

You agree to the [Terms of Use](#) of this site by reviewing, using, or interpreting these data.

[Ontario Ministry of Transportation | Terms and Conditions | About](#)

Last Modified: September 2016



* SEE STORM WATER MANAGEMENT REPORT (PAGE 6) FOR THE FULL EXTENT OF THESE AREAS

BM 4475
 GEODETIC SURVEY OF CANADA BENCH MARK No 4475.
 IRON PIPE WITH BRASS CAP, ELEVATION 290.58 METRES.

ACCEPTED FOR CONSTRUCTION
 A.C. HENSON AND ASSOCIATES (ENGINEERS) LTD.
 DATE: 6/17/96
 APPROVED BY: [Signature]

LEGEND

| | |
|--|----------------------------|
| | PROPOSED ELEVATION |
| | EXISTING GROUND ELEVATION |
| | DRAINAGE DITCH |
| | DIRECTION OF OVERLAND FLOW |
| | BASIN LIMIT |
| | BASIN LABEL |
| | PORTION IMPERMEABLE |
| | LAND USE RUNOFF CURVE NO. |
| | AREA IN % |

| NO. | DATE | DESCRIPTION | BY |
|-----|----------|----------------------------------|--------|
| 1 | 06/03/95 | RELEASED FOR COMMENT | J.J.M. |
| 2 | 07/28/95 | REVISED AS PER TOWNSHIP COMMENTS | J.J.M. |
| 3 | 10/01/95 | REVISED AS PER TOWNSHIP COMMENTS | J.J.M. |

**HASTINGS SUBDIVISION
 STORM DRAINAGE AREA PLAN
 PART OF E1/2 LOT 20 CONCESSION 5
 TOWNSHIP OF ORO NOW IN THE
 TOWNSHIP OF ORO MIDDLETON
 COUNTY OF SIMCOE**

DEARDEN AND STANTON LTD. E-671-4
 ONTARIO LAND SURVEYORS
 CONSULTING ENGINEERS
 100 COLLEGE STREET, SUITE 100
 ORILLIA - ONTARIO

DATE: MARCH 6, 1995
 SCALE: AS SHOWN

CAD FILE NO: [blank]
 SHEET 4 OF 10



| | | | |
|---------------|----------------------------|--------------|------------|
| Project: | Melville Court Development | Designed By: | LB |
| File: | 420427 | Checked By: | NHF |
| Municipality: | Township of Oro-Medonte | Date: | 2021-12-23 |

Existing SWM Facility - Pond Volume Table

Active Pool Dry Pond
 Bottom Elev. Varies
 Side Slopes Varies

| Water Level Elev. (m) | Depth (m) | Areas | | Storage | |
|--------------------------|--------------|---------------------------|--------------------------------|---------------------------|----------------------------------|
| | | Area (m ²) | Avg. Area (m ²) | Live (m ³) | Accum. Live (m ³) |
| 287.50 | 0.00 | 1793.90 | 0.00 | 0.0 | 0.0 |
| 287.60 | 0.10 | 1857.08 | 1825.49 | 182.5 | 182.5 |
| 287.70 | 0.20 | 1920.26 | 1888.67 | 188.9 | 371.4 |
| 287.80 | 0.30 | 1983.45 | 1951.86 | 195.2 | 566.6 |
| 287.90 | 0.40 | 2046.63 | 2015.04 | 201.5 | 768.1 |
| 288.00 | 0.50 | 2109.81 | 2078.22 | 207.8 | 975.9 |
| 288.10 | 0.60 | 2172.99 | 2141.40 | 214.1 | 1190.1 |
| 288.20 | 0.70 | 2236.18 | 2204.59 | 220.5 | 1410.5 |
| 288.30 | 0.80 | 2299.36 | 2267.77 | 226.8 | 1637.3 |
| 288.40 | 0.90 | 2362.54 | 2330.95 | 233.1 | 1870.4 |
| 288.50 | 1.00 | 2425.72 | 2394.13 | 239.4 | 2109.8 |
| 288.60 | 1.10 | 2488.91 | 2457.32 | 245.7 | 2355.5 |
| 288.70 | 1.20 | 2552.09 | 2520.50 | 252.0 | 2607.6 |
| 288.80 | 1.30 | 2615.27 | 2583.68 | 258.4 | 2866.0 |
| 288.90 | 1.40 | 2678.45 | 2646.86 | 264.7 | 3130.6 |
| 288.98 | 1.48 | 2729.00 | 2672.14 | 481.0 | 3346.9 |

Note: Bold areas measured using Civil 3D (based on ex. Conditions (2015) topo survey contours)

Note: Pond bottom slopes gradually toward outlet from 287.50 to 287.19. Bottom elevation of 287.50 set for volume calculation purposes based on topographic survey information.



| | | | |
|---------------|----------------------------|--------------|------------|
| Project: | Melville Court Development | Designed By: | LB |
| File: | 420427 | Checked By: | NHF |
| Municipality: | Township of Oro-Medonte | Date: | 2021-12-23 |

Existing SWM Facility - Pond Discharge Table

OUTLET CONTROL

Outlet Pipe

diameter = 300 mm
 area = 0.0707 m²
 Orifice C = 0.80
 Invert = 287.19 m

Orifice Equation

$$Q = C \times A \times (2gH)^{0.5}$$

WEIR CONTROL

Emergency Spillway

Length of Weir N/A m
 Weir Sill Elevation 288.98 m
 Weir constant K Varies
 Side Slope (H:V) N/A

Weir Control Discharge Rates Calculated as per Weir Control Design Sheet

where

Q = flow rate (cms) A = area of opening(sq. m)
 C = constant H = net head on the orifice
 g = Acceleration due to gravity

| Water Level (m) | Orifice Plate #1 | | Emergency Spillway | | Hydraulic Control | Total Discharge (cms) |
|--------------------|------------------|--------------------------------|--------------------|--------------------------------|-------------------|--------------------------|
| | Head (m) | Discharge (m ³) | Head (m) | Discharge (m ³) | | |
| 287.60 | 0.26 | 0.1277 | 0.00 | | Orifice Plate #1 | 0.1277 |
| 287.70 | 0.36 | 0.1503 | 0.00 | | Orifice Plate #1 | 0.1503 |
| 287.80 | 0.46 | 0.1699 | 0.00 | | Orifice Plate #1 | 0.1699 |
| 287.90 | 0.56 | 0.1874 | 0.00 | | Orifice Plate #1 | 0.1874 |
| 288.00 | 0.66 | 0.2035 | 0.00 | | Orifice Plate #1 | 0.2035 |
| 288.10 | 0.76 | 0.2184 | 0.00 | | Orifice Plate #1 | 0.2184 |
| 288.20 | 0.86 | 0.2323 | 0.00 | | Orifice Plate #1 | 0.2323 |
| 288.30 | 0.96 | 0.2454 | 0.00 | | Orifice Plate #1 | 0.2454 |
| 288.40 | 1.06 | 0.2579 | 0.00 | | Orifice Plate #1 | 0.2579 |
| 288.50 | 1.16 | 0.2698 | 0.00 | | Orifice Plate #1 | 0.2698 |
| 288.60 | 1.26 | 0.2812 | 0.00 | | Orifice Plate #1 | 0.2812 |
| 288.70 | 1.36 | 0.2921 | 0.00 | | Orifice Plate #1 | 0.2921 |
| 288.80 | 1.46 | 0.3027 | 0.00 | | Orifice Plate #1 | 0.3027 |
| 288.90 | 1.56 | 0.3128 | 0.00 | | Orifice Plate #1 | 0.3128 |
| 288.98 | 1.64 | 0.3208 | 0.00 | | Orifice Plate #1 | 0.3208 |

Note: Invert of outlet pipe (287.19) is below base of pond as per topographic survey.

Note: Shaded cells have been calculated using partially full inlet control equations (Hydraulic Structures, C.D.Smith, University of Saskatchewan)



| | | | |
|---------------|----------------------------|--------------|------------|
| Project: | Melville Court Development | Designed By: | LB |
| File: | 420427 | Checked By: | NHF |
| Municipality: | Township of Oro-Medonte | Date: | 2021-12-23 |

Existing SWM Facility - Stage-Storage-Discharge Table

| Water Level (m) | Orifice Plate Discharge (m³/s) | Emergency Discharge (m³/s) | Hydraulic Control | Total Discharge (m³/s) | Total Storage (ha-m) |
|----------------------------|--|--|------------------------------|--|---------------------------------|
| 287.60 | 0.1277 | | Orifice Plate #1 | 0.128 | 0.018 |
| 287.70 | 0.1503 | | Orifice Plate #1 | 0.150 | 0.037 |
| 287.80 | 0.1699 | | Orifice Plate #1 | 0.170 | 0.057 |
| 287.90 | 0.1874 | | Orifice Plate #1 | 0.187 | 0.077 |
| 288.00 | 0.2035 | | Orifice Plate #1 | 0.203 | 0.098 |
| 288.10 | 0.2184 | | Orifice Plate #1 | 0.218 | 0.119 |
| 288.20 | 0.2323 | | Orifice Plate #1 | 0.232 | 0.141 |
| 288.30 | 0.2454 | | Orifice Plate #1 | 0.245 | 0.164 |
| 288.40 | 0.2579 | | Orifice Plate #1 | 0.258 | 0.187 |
| 288.50 | 0.2698 | | Orifice Plate #1 | 0.270 | 0.211 |
| 288.60 | 0.2812 | | Orifice Plate #1 | 0.281 | 0.236 |
| 288.70 | 0.2921 | | Orifice Plate #1 | 0.292 | 0.261 |
| 288.80 | 0.3027 | | Orifice Plate #1 | 0.303 | 0.287 |
| 288.90 | 0.3128 | | Orifice Plate #1 | 0.313 | 0.313 |
| 288.98 | 0.3208 | | Orifice Plate #1 | 0.321 | 0.335 |

Existing Pond Outlet Hydraulics - Outlet Pipe

$$\text{Outlet Pipe} = C \times A \times (2gH)^{0.5}$$

$$C = 0.8$$

| Depth Increment m | (invert of Control) | (Top of Pond) |
|-------------------------|-----------------------|-------------------------|
| | Min Elevation m | Max Elevation Max |
| 0.1 | 287.19 | 289.10 |

| Water Elevation m | Depth from Min. Elevation m | Pipe Partially Full Inlet Control Flow (Smith) m ³ /s |
|-------------------------|--------------------------------------|--|
| 287.19 | 0.00 | |
| 287.20 | 0.10 | 0.001 |
| 287.30 | 0.20 | 0.009 |
| 287.40 | 0.30 | 0.031 |
| 287.50 | 0.40 | 0.058 |
| 287.60 | 0.50 | 0.128 |
| 287.70 | 0.60 | 0.150 |
| 287.80 | 0.70 | 0.170 |
| 287.90 | 0.80 | 0.187 |
| 288.00 | 0.90 | 0.203 |
| 288.10 | 1.00 | 0.218 |
| 288.20 | 1.10 | 0.232 |
| 288.30 | 1.20 | 0.245 |

Pipe Flowing Partially Full or full

| | |
|---------------------------------------|--------|
| Coefficient Metric = 1, Imperial=1.49 | 1 |
| Pipe Diameter (m) | 0.3 |
| Mannings N Full | 0.013 |
| Pipe Slope (m/m) | |
| Elevation (m) | 287.19 |
| Inlet Type: 1= Projecting 2= Flush | 1 |

Inlet Control (Smith) Projecting Inlet

$$\text{MMF Model: } y = (a \cdot b + c \cdot x^d) / (b + x^d)$$

Coefficient Data:

| | |
|-----|----------|
| a = | 1.68E-02 |
| b = | 1.77E+00 |
| c = | 3.06E+00 |
| d = | 2.27E+00 |

Inlet Control (Smith) Flush Inlet

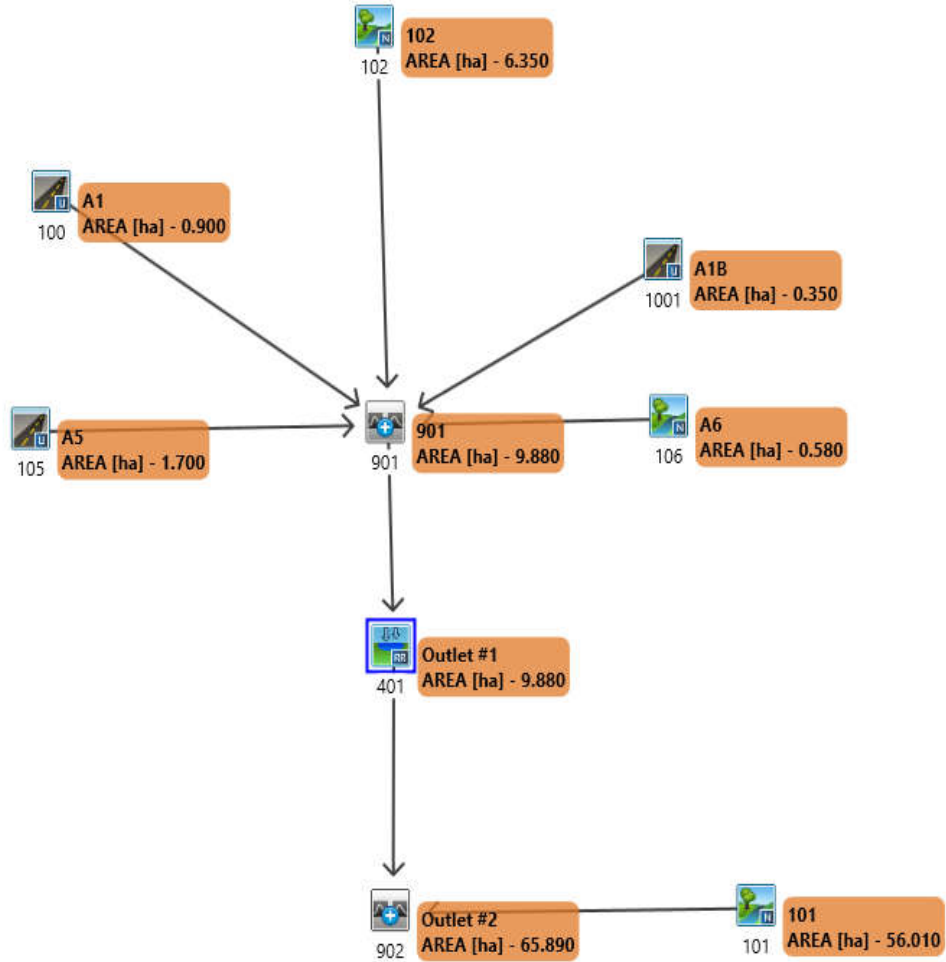
$$\text{MMF Model: } y = (a \cdot b + c \cdot x^d) / (b + x^d)$$

Coefficient Data:

| | |
|-----|-----------|
| a = | -1.08E-03 |
| b = | 2.30E+00 |
| c = | 3.95E+00 |
| d = | 2.08E+00 |

| | | | |
|---------|----------------------------------|------|------------|
| PROJECT | Melville Court Development | FILE | 420427 |
| | | DATE | 12-22-2021 |
| SUBJECT | Existing Conditions VO Schematic | NAME | LB |
| | | PAGE | 1 OF 1 |

EXISTING CONDITIONS



NASHYD



ROUTE PIPE



DUHYD



STANDHYD



ROUTE CHANNEL



DIVERT HYD



ADDHYD



ROUTE RESERVOIR



SHIFTHYD


```

* CALIB STANDHYD      0100  1 10.0   0.90  0.08  1.33  17.75  0.40  0.000
  [I%=30.0:S%= 2.00]
*
* CHIC STORM          10.0
  [ Ptot= 43.85 mm ]
*
* CALIB STANDHYD      0105  1 10.0   1.70  0.13  1.33  17.28  0.39  0.000
  [I%=25.0:S%= 2.00]
*
* ADD [ 0100+ 1001]  0901  3 10.0   1.25  0.09  1.33  16.24  n/a  0.000
*
* ADD [ 0901+ 0102]  0901  1 10.0   7.60  0.09  1.33  8.53  n/a  0.000
*
* ADD [ 0901+ 0105]  0901  3 10.0   9.30  0.22  1.33  10.13  n/a  0.000
*
* ADD [ 0901+ 0106]  0901  1 2.0    9.88  0.25  1.33  10.26  n/a  0.000
*
** Reservoir
OUTFLOW:              0401  1 1.0    9.88  0.10  1.57  10.26  n/a  0.000
*
* ADD [ 0101+ 0401]  0902  3 1.0   65.89  0.37  3.03  6.95  n/a  0.000

```

```

=====
V  V  I  SSSSS  U  U  A  L          (v 6.2.2008)
V  V  I  SS    U  U  A  A  L
V  V  I  SS    U  U  AAAAA L
V  V  I  SS    U  U  A  A  L
VV    I  SSSSS  UUUUU  A  A  LLLLL

  000  TTTT  TTTT  H  H  Y  Y  M  M  000  TM
  O  O  T    T  H  H  Y  Y  MM MM  O  O
  O  O  T    T  H  H  Y  Y  M  M  O  O
  000  T    T  H  H  Y  Y  M  M  000

```

Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2021 Smart City Water Inc
 All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
 Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\810287de-4038
 Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\810287de-4038

DATE: 01-19-2022 TIME: 07:47:50

USER: _____
 COMMENTS: _____

 ** SIMULATION : Run 03 - 10 Year 4 Hour Chica **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|---|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| CHIC STORM [Ptot= 50.98 mm] | | 10.0 | | | | | | |
| ** CALIB NASHYD [CN=60.6 [N = 3.0:Tp 1.17] | 0101 | 1 14.0 | 56.01 | 0.43 | 3.03 | 8.84 | 0.17 | 0.000 |
| CHIC STORM [Ptot= 50.98 mm] | | 10.0 | | | | | | |
| ** CALIB NASHYD [CN=59.0 [N = 3.0:Tp 1.07] | 0102 | 1 13.0 | 6.35 | 0.05 | 2.82 | 9.50 | 0.19 | 0.000 |
| CHIC STORM [Ptot= 50.98 mm] | | 10.0 | | | | | | |
| ** CALIB NASHYD | 0106 | 1 2.0 | 0.58 | 0.04 | 1.40 | 16.30 | 0.32 | 0.000 |

```

[CN=74.0
[ N = 3.0:Tp 0.11]
*
* CHIC STORM          10.0
  [ Ptot= 50.98 mm ]
*
* CALIB STANDHYD      1001  1 10.0   0.35  0.02  1.33  15.79  0.31  0.000
  [I%=10.0:S%= 2.00]
*
* CHIC STORM          10.0
  [ Ptot= 50.98 mm ]
*
* CALIB STANDHYD      0100  1 10.0   0.90  0.09  1.33  21.63  0.42  0.000
  [I%=30.0:S%= 2.00]
*
* CHIC STORM          10.0
  [ Ptot= 50.98 mm ]
*
* CALIB STANDHYD      0105  1 10.0   1.70  0.15  1.33  21.29  0.42  0.000
  [I%=25.0:S%= 2.00]
*
* ADD [ 0100+ 1001]  0901  3 10.0   1.25  0.11  1.33  20.00  n/a  0.000
*
* ADD [ 0901+ 0102]  0901  1 10.0   7.60  0.11  1.33  11.23  n/a  0.000
*
* ADD [ 0901+ 0105]  0901  3 10.0   9.30  0.27  1.33  13.07  n/a  0.000
*
* ADD [ 0901+ 0106]  0901  1 2.0    9.88  0.30  1.33  13.26  n/a  0.000
*
** Reservoir
OUTFLOW:              0401  1 1.0    9.88  0.13  1.62  13.26  n/a  0.000
OVERFLOW:            0401  3 1.0    0.00  0.00  0.00  0.00  n/a  0.000
*
* ADD [ 0101+ 0401]  0902  3 1.0   65.89  0.52  2.80  9.50  n/a  0.000

```

```

=====
V  V  I  SSSSS  U  U  A  L          (v 6.2.2008)
V  V  I  SS    U  U  A  A  L
V  V  I  SS    U  U  AAAAA L
V  V  I  SS    U  U  A  A  L
VV    I  SSSSS  UUUUU  A  A  LLLLL

  000  TTTT  TTTT  H  H  Y  Y  M  M  000  TM
  O  O  T    T  H  H  Y  Y  MM MM  O  O
  O  O  T    T  H  H  Y  Y  M  M  O  O
  000  T    T  H  H  Y  Y  M  M  000

```

Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2021 Smart City Water Inc
 All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
 Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\5047036c-dbfef
 Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\5047036c-dbfef

DATE: 01-19-2022 TIME: 07:47:50

USER: _____
 COMMENTS: _____

 ** SIMULATION : Run 04 - 25 Year 4 Hour Chica **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|---|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| CHIC STORM [Ptot= 59.78 mm] | | 10.0 | | | | | | |
| ** CALIB NASHYD [CN=60.6 [N = 3.0:Tp 1.17] | 0101 | 1 14.0 | 56.01 | 0.60 | 3.03 | 12.31 | 0.21 | 0.000 |

```

* CHIC STORM 10.0
[ Ptot= 59.78 mm ]
** CALIB NASHYD 0102 1 13.0 6.35 0.08 2.82 12.97 0.22 0.000
[CN=59.0
[ N = 3.0:Tp 1.07]
* CHIC STORM 10.0
[ Ptot= 59.78 mm ]
** CALIB NASHYD 0106 1 2.0 0.58 0.05 1.40 21.55 0.36 0.000
[CN=74.0
[ N = 3.0:Tp 0.11]
* CHIC STORM 10.0
[ Ptot= 59.78 mm ]
* CALIB STANDHYD 1001 1 10.0 0.35 0.02 1.33 20.41 0.34 0.000
[I%=10.0:S%= 2.00]
* CHIC STORM 10.0
[ Ptot= 59.78 mm ]
* CALIB STANDHYD 0100 1 10.0 0.90 0.11 1.33 26.71 0.45 0.000
[I%=30.0:S%= 2.00]
* CHIC STORM 10.0
[ Ptot= 59.78 mm ]
* CALIB STANDHYD 0105 1 10.0 1.70 0.19 1.33 26.54 0.44 0.000
[I%=25.0:S%= 2.00]
* ADD [ 0100+ 1001] 0901 3 10.0 1.25 0.13 1.33 24.94 n/a 0.000
* ADD [ 0901+ 0102] 0901 1 10.0 7.60 0.14 1.33 14.94 n/a 0.000
* ADD [ 0901+ 0105] 0901 3 10.0 9.30 0.33 1.33 17.06 n/a 0.000
* ADD [ 0901+ 0106] 0901 1 2.0 9.88 0.37 1.33 17.33 n/a 0.000
** Reservoir
OUTFLOW: 0401 1 1.0 9.88 0.14 1.87 17.33 n/a 0.000
OVERFLOW: 0401 3 1.0 0.00 0.00 0.00 0.00 n/a 0.000
* ADD [ 0101+ 0401] 0902 3 1.0 65.89 0.73 2.80 13.06 n/a 0.000

```

```

V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL
000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
000 T T H H Y M M 000

```

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2021 Smart City Water Inc
All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\29fd2222-10ad
Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\29fd2222-10ad

DATE: 01-19-2022 TIME: 07:47:50

USER:

COMMENTS: _____

** SIMULATION : Run 05 - 50 Year 4 Hour Chica **

```

W/E COMMAND HYD ID DT AREA Qpeak Tpeak R.V. R.C. Qbase
min ha cms hrs mm
-----
START @ 0.00 hrs
CHIC STORM 10.0
[ Ptot= 66.46 mm ]
** CALIB NASHYD 0101 1 14.0 56.01 0.74 2.80 15.22 0.23 0.000
[CN=60.6
[ N = 3.0:Tp 1.17]
* CHIC STORM 10.0
[ Ptot= 66.46 mm ]
** CALIB NASHYD 0102 1 13.0 6.35 0.09 2.82 15.87 0.24 0.000
[CN=59.0
[ N = 3.0:Tp 1.07]
* CHIC STORM 10.0
[ Ptot= 66.46 mm ]
** CALIB NASHYD 0106 1 2.0 0.58 0.06 1.40 25.82 0.39 0.000
[CN=74.0
[ N = 3.0:Tp 0.11]
* CHIC STORM 10.0
[ Ptot= 66.46 mm ]
* CALIB STANDHYD 1001 1 10.0 0.35 0.02 1.33 24.16 0.36 0.000
[I%=10.0:S%= 2.00]
* CHIC STORM 10.0
[ Ptot= 66.46 mm ]
* CALIB STANDHYD 0100 1 10.0 0.90 0.13 1.33 30.74 0.46 0.000
[I%=30.0:S%= 2.00]
* CHIC STORM 10.0
[ Ptot= 66.46 mm ]
* CALIB STANDHYD 0105 1 10.0 1.70 0.21 1.33 30.73 0.46 0.000
[I%=25.0:S%= 2.00]
* ADD [ 0100+ 1001] 0901 3 10.0 1.25 0.15 1.33 28.90 n/a 0.000
* ADD [ 0901+ 0102] 0901 1 10.0 7.60 0.16 1.33 18.01 n/a 0.000
* ADD [ 0901+ 0105] 0901 3 10.0 9.30 0.37 1.33 20.34 n/a 0.000
* ADD [ 0901+ 0106] 0901 1 2.0 9.88 0.43 1.33 20.66 n/a 0.000
** Reservoir
OUTFLOW: 0401 1 1.0 9.88 0.15 2.37 20.66 n/a 0.000
OVERFLOW: 0401 3 1.0 0.00 0.00 0.00 0.00 n/a 0.000
* ADD [ 0101+ 0401] 0902 3 1.0 65.89 0.89 2.80 16.04 n/a 0.000

```

```

V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL
000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
000 T T H H Y M M 000

```

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2021 Smart City Water Inc
All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\67b8585-ffa6
Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\67b8585-ffa6

DATE: 01-19-2022 TIME: 07:47:51

USER:

COMMENTS: _____

** SIMULATION : Run 06 - 100 Year 4 Hour Chic **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|--------------------|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| ----- | | | | | | | | |
| CHIC STORM | | 10.0 | | | | | | |
| [Ptot= 72.98 mm] | | | | | | | | |
| ** CALIB NASHYD | 0101 | 1 14.0 | 56.01 | 0.90 | 2.80 | 18.28 | 0.25 | 0.000 |
| [CN=60.6 | | | | | | | | |
| [N = 3.0:Tp 1.17] | | | | | | | | |
| CHIC STORM | | 10.0 | | | | | | |
| [Ptot= 72.98 mm] | | | | | | | | |
| ** CALIB NASHYD | 0102 | 1 13.0 | 6.35 | 0.11 | 2.60 | 18.90 | 0.26 | 0.000 |
| [CN=59.0 | | | | | | | | |
| [N = 3.0:Tp 1.07] | | | | | | | | |
| CHIC STORM | | 10.0 | | | | | | |
| [Ptot= 72.98 mm] | | | | | | | | |
| ** CALIB NASHYD | 0106 | 1 2.0 | 0.58 | 0.08 | 1.40 | 30.18 | 0.41 | 0.000 |
| [CN=74.0 | | | | | | | | |
| [N = 3.0:Tp 0.11] | | | | | | | | |
| CHIC STORM | | 10.0 | | | | | | |
| [Ptot= 72.98 mm] | | | | | | | | |
| ** CALIB STANDHYD | 1001 | 1 10.0 | 0.35 | 0.03 | 1.33 | 28.01 | 0.38 | 0.000 |
| [I%=10.0:S%= 2.00] | | | | | | | | |
| CHIC STORM | | 10.0 | | | | | | |
| [Ptot= 72.98 mm] | | | | | | | | |
| ** CALIB STANDHYD | 0100 | 1 10.0 | 0.90 | 0.14 | 1.33 | 34.82 | 0.48 | 0.000 |
| [I%=30.0:S%= 2.00] | | | | | | | | |
| CHIC STORM | | 10.0 | | | | | | |
| [Ptot= 72.98 mm] | | | | | | | | |
| ** CALIB STANDHYD | 0105 | 1 10.0 | 1.70 | 0.24 | 1.33 | 34.97 | 0.48 | 0.000 |
| [I%=25.0:S%= 2.00] | | | | | | | | |
| ADD [0100+ 1001] | 0901 | 3 10.0 | 1.25 | 0.17 | 1.33 | 32.91 | n/a | 0.000 |
| ADD [0901+ 0102] | 0901 | 1 10.0 | 7.60 | 0.18 | 1.33 | 21.21 | n/a | 0.000 |
| ADD [0901+ 0105] | 0901 | 3 10.0 | 9.30 | 0.42 | 1.33 | 23.72 | n/a | 0.000 |
| ADD [0901+ 0106] | 0901 | 1 2.0 | 9.88 | 0.49 | 1.33 | 24.10 | n/a | 0.000 |
| ** Reservoir | | | | | | | | |
| OUTFLOW: | 0401 | 1 1.0 | 9.88 | 0.16 | 2.87 | 24.10 | n/a | 0.000 |
| OVERFLOW: | 0401 | 3 1.0 | 0.00 | 0.00 | 0.00 | 0.00 | n/a | 0.000 |
| ADD [0101+ 0401] | 0902 | 3 1.0 | 65.89 | 1.06 | 2.80 | 19.15 | n/a | 0.000 |

```

V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
000 T T H H Y M M 000

```

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2021 Smart City Water Inc
All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\LBuss\AppData\Local\Civica\XH5\18b1f826-8929-408e-9936-e2d934269cf1\6139d847-09cb
Summary filename: C:\Users\LBuss\AppData\Local\Civica\XH5\18b1f826-8929-408e-9936-e2d934269cf1\6139d847-09cb

DATE: 01-19-2022 TIME: 07:47:50

USER:

COMMENTS: _____

** SIMULATION : Run 07 - 2 Year 24 Hour SCS **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|--------------------|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| ----- | | | | | | | | |
| MASS STORM | | 15.0 | | | | | | |
| [Ptot= 56.53 mm] | | | | | | | | |
| ** CALIB NASHYD | 0101 | 1 14.0 | 56.01 | 0.41 | 12.83 | 11.04 | 0.20 | 0.000 |
| [CN=60.6 | | | | | | | | |
| [N = 3.0:Tp 1.17] | | | | | | | | |
| MASS STORM | | 15.0 | | | | | | |
| [Ptot= 56.53 mm] | | | | | | | | |
| ** CALIB NASHYD | 0102 | 1 13.0 | 6.35 | 0.05 | 12.78 | 11.64 | 0.21 | 0.000 |
| [CN=59.0 | | | | | | | | |
| [N = 3.0:Tp 1.07] | | | | | | | | |
| MASS STORM | | 15.0 | | | | | | |
| [Ptot= 56.53 mm] | | | | | | | | |
| ** CALIB NASHYD | 0106 | 1 2.0 | 0.58 | 0.04 | 11.77 | 19.56 | 0.35 | 0.000 |
| [CN=74.0 | | | | | | | | |
| [N = 3.0:Tp 0.11] | | | | | | | | |
| MASS STORM | | 15.0 | | | | | | |
| [Ptot= 56.53 mm] | | | | | | | | |
| ** CALIB STANDHYD | 1001 | 1 10.0 | 0.35 | 0.01 | 11.83 | 18.65 | 0.33 | 0.000 |
| [I%=10.0:S%= 2.00] | | | | | | | | |
| MASS STORM | | 15.0 | | | | | | |
| [Ptot= 56.53 mm] | | | | | | | | |
| ** CALIB STANDHYD | 0100 | 1 10.0 | 0.90 | 0.06 | 11.67 | 24.80 | 0.44 | 0.000 |
| [I%=30.0:S%= 2.00] | | | | | | | | |
| MASS STORM | | 15.0 | | | | | | |
| [Ptot= 56.53 mm] | | | | | | | | |
| ** CALIB STANDHYD | 0105 | 1 10.0 | 1.70 | 0.10 | 11.67 | 24.56 | 0.43 | 0.000 |
| [I%=25.0:S%= 2.00] | | | | | | | | |
| ADD [0100+ 1001] | 0901 | 3 10.0 | 1.25 | 0.07 | 11.67 | 23.08 | n/a | 0.000 |
| ADD [0901+ 0102] | 0901 | 1 10.0 | 7.60 | 0.08 | 11.67 | 13.52 | n/a | 0.000 |
| ADD [0901+ 0105] | 0901 | 3 10.0 | 9.30 | 0.18 | 11.67 | 15.54 | n/a | 0.000 |
| ADD [0901+ 0106] | 0901 | 1 2.0 | 9.88 | 0.20 | 11.73 | 15.78 | n/a | 0.000 |
| ** Reservoir | | | | | | | | |
| OUTFLOW: | 0401 | 1 1.0 | 9.88 | 0.12 | 11.97 | 15.78 | n/a | 0.000 |
| OVERFLOW: | 0401 | 3 1.0 | 0.00 | 0.00 | 0.00 | 0.00 | n/a | 0.000 |
| ADD [0101+ 0401] | 0902 | 3 1.0 | 65.89 | 0.50 | 12.83 | 11.75 | n/a | 0.000 |

```

V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL

```

```

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
000 T T H H Y M M 000

```

Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2021 Smart City Water Inc
 All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\vojn.dat
 Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\35649a2f-e9f5
 Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\35649a2f-e9f5

DATE: 01-19-2022 TIME: 07:47:50

USER:

COMMENTS: _____

 ** SIMULATION : Run 08 - 5 Year 24 Hour SCS **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|---|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| MASS STORM [Ptot= 74.97 mm] | | | | | 15.0 | | | |
| ** CALIB NASHYD [CN=60.6 [N = 3.0:Tp 1.17] | 0101 | 1 14.0 | 56.01 | 0.73 | 12.83 | 19.34 | 0.26 | 0.000 |
| MASS STORM [Ptot= 74.97 mm] | | | | | 15.0 | | | |
| ** CALIB NASHYD [CN=59.0 [N = 3.0:Tp 1.07] | 0102 | 1 13.0 | 6.35 | 0.09 | 12.78 | 19.86 | 0.26 | 0.000 |
| MASS STORM [Ptot= 74.97 mm] | | | | | 15.0 | | | |
| ** CALIB NASHYD [CN=74.0 [N = 3.0:Tp 0.11] | 0106 | 1 2.0 | 0.58 | 0.06 | 11.77 | 31.55 | 0.42 | 0.000 |
| MASS STORM [Ptot= 74.97 mm] | | | | | 15.0 | | | |
| ** CALIB STANDHYD [I%=10.0:S%= 2.00] | 1001 | 1 10.0 | 0.35 | 0.02 | 11.83 | 29.21 | 0.39 | 0.000 |
| MASS STORM [Ptot= 74.97 mm] | | | | | 15.0 | | | |
| ** CALIB STANDHYD [I%=30.0:S%= 2.00] | 0100 | 1 10.0 | 0.90 | 0.08 | 11.67 | 36.09 | 0.48 | 0.000 |
| MASS STORM [Ptot= 74.97 mm] | | | | | 15.0 | | | |
| ** CALIB STANDHYD [I%=25.0:S%= 2.00] | 0105 | 1 10.0 | 1.70 | 0.14 | 11.67 | 36.30 | 0.48 | 0.000 |
| ADD [0100+ 1001] | 0901 | 3 10.0 | 1.25 | 0.10 | 11.67 | 34.16 | n/a | 0.000 |
| ADD [0901+ 0102] | 0901 | 1 10.0 | 7.60 | 0.12 | 11.83 | 22.21 | n/a | 0.000 |

```

* ADD [ 0901+ 0105] 0901 3 10.0 9.30 0.26 11.67 24.79 n/a 0.000
* ADD [ 0901+ 0106] 0901 1 2.0 9.88 0.31 11.77 25.18 n/a 0.000
** Reservoir
OUTFLOW: 0401 1 1.0 9.88 0.15 12.37 25.18 n/a 0.000
OVERFLOW: 0401 3 1.0 0.00 0.00 0.00 n/a 0.000
* ADD [ 0101+ 0401] 0902 3 1.0 65.89 0.88 12.83 20.22 n/a 0.000

```

```

V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL

```

```

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
000 T T H H Y M M 000

```

Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2021 Smart City Water Inc
 All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\vojn.dat
 Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\35649a2f-e9f5
 Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\35649a2f-e9f5

DATE: 01-19-2022 TIME: 07:47:50

USER:

COMMENTS: _____

 ** SIMULATION : Run 09 - 10 Year 24 Hour SCS **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|---|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| MASS STORM [Ptot= 87.24 mm] | | | | | 15.0 | | | |
| ** CALIB NASHYD [CN=60.6 [N = 3.0:Tp 1.17] | 0101 | 1 14.0 | 56.01 | 0.99 | 12.83 | 25.71 | 0.29 | 0.000 |
| MASS STORM [Ptot= 87.24 mm] | | | | | 15.0 | | | |
| ** CALIB NASHYD [CN=59.0 [N = 3.0:Tp 1.07] | 0102 | 1 13.0 | 6.35 | 0.12 | 12.78 | 26.13 | 0.30 | 0.000 |
| MASS STORM [Ptot= 87.24 mm] | | | | | 15.0 | | | |
| ** CALIB NASHYD [CN=74.0 [N = 3.0:Tp 0.11] | 0106 | 1 2.0 | 0.58 | 0.08 | 11.77 | 40.28 | 0.46 | 0.000 |
| MASS STORM [Ptot= 87.24 mm] | | | | | 15.0 | | | |
| ** CALIB STANDHYD [I%=10.0:S%= 2.00] | 1001 | 1 10.0 | 0.35 | 0.03 | 11.83 | 36.95 | 0.42 | 0.000 |
| MASS STORM [Ptot= 87.24 mm] | | | | | 15.0 | | | |

```

* CALIB STANDHYD      0100  1 10.0   0.90   0.10 11.67  44.16 0.51   0.000
  [I%=30.0:S%= 2.00]
*
* MASS STORM
  [ Ptot= 87.24 mm ]      15.0
*
* CALIB STANDHYD      0105  1 10.0   1.70   0.17 11.67  44.71 0.51   0.000
  [I%=25.0:S%= 2.00]
*
* ADD [ 0100+ 1001] 0901  3 10.0   1.25   0.12 11.67  42.14 n/a   0.000
*
* ADD [ 0901+ 0102] 0901  1 10.0   7.60   0.16 11.83  28.76 n/a   0.000
*
* ADD [ 0901+ 0105] 0901  3 10.0   9.30   0.32 11.67  31.68 n/a   0.000
*
* ADD [ 0901+ 0106] 0901  1  2.0   9.88   0.40 11.77  32.18 n/a   0.000
*
** Reservoir
  OUTFLOW:              0401  1  1.0   9.88   0.16 12.80  32.18 n/a   0.000
*
* ADD [ 0101+ 0401] 0902  3  1.0   65.89  1.15 12.83  26.68 n/a   0.000

```

```

[ N = 3.0:Tp 0.11]
*
* MASS STORM
  [ Ptot=102.29 mm ]      15.0
*
* CALIB STANDHYD      1001  1 10.0   0.35   0.04 11.67  47.09 0.46   0.000
  [I%=10.0:S%= 2.00]
*
* MASS STORM
  [ Ptot=102.29 mm ]      15.0
*
* CALIB STANDHYD      0100  1 10.0   0.90   0.12 11.67  54.58 0.53   0.000
  [I%=30.0:S%= 2.00]
*
* MASS STORM
  [ Ptot=102.29 mm ]      15.0
*
* CALIB STANDHYD      0105  1 10.0   1.70   0.21 11.67  55.57 0.54   0.000
  [I%=25.0:S%= 2.00]
*
* ADD [ 0100+ 1001] 0901  3 10.0   1.25   0.16 11.67  52.48 n/a   0.000
*
* ADD [ 0901+ 0102] 0901  1 10.0   7.60   0.21 11.83  37.51 n/a   0.000
*
* ADD [ 0901+ 0105] 0901  3 10.0   9.30   0.41 11.83  40.81 n/a   0.000
*
* ADD [ 0901+ 0106] 0901  1  2.0   9.88   0.51 11.77  41.45 n/a   0.000
*
** Reservoir
  OUTFLOW:              0401  1  1.0   9.88   0.19 13.18  41.45 n/a   0.000
  OUTFLOW:              0401  3  1.0   0.00   0.00 0.00   0.00 n/a   0.000
*
* ADD [ 0101+ 0401] 0902  3  1.0   65.89  1.52 12.83  35.36 n/a   0.000

```

```

V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUU A A LLLLL

```

```

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
000 T T H H Y M M 000

```

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2021 Smart City Water Inc
All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\db856e58-e8f9
Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\db856e58-e8f9

DATE: 01-19-2022 TIME: 07:47:50
USER:

COMMENTS: _____

** SIMULATION : Run 10 - 25 Year 24 Hour SCS **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|---|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| MASS STORM [Ptot=102.29 mm] | | 15.0 | | | | | | |
| ** CALIB NASHYD [CN=60.6 [N = 3.0:Tp 1.17] | 0101 | 1 14.0 | 56.01 | 1.33 | 12.83 | 34.29 | 0.34 | 0.000 |
| MASS STORM [Ptot=102.29 mm] | | 15.0 | | | | | | |
| ** CALIB NASHYD [CN=59.0 [N = 3.0:Tp 1.07] | 0102 | 1 13.0 | 6.35 | 0.16 | 12.78 | 34.57 | 0.34 | 0.000 |
| MASS STORM [Ptot=102.29 mm] | | 15.0 | | | | | | |
| ** CALIB NASHYD [CN=74.0 | 0106 | 1 2.0 | 0.58 | 0.10 | 11.77 | 51.63 | 0.50 | 0.000 |

```

V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUU A A LLLLL

```

```

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
000 T T H H Y M M 000

```

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2021 Smart City Water Inc
All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\c9d835c0-b078
Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\c9d835c0-b078

DATE: 01-19-2022 TIME: 07:47:50
USER:

COMMENTS: _____

** SIMULATION : Run 11 - 50 Year 24 Hour SCS **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|---|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| MASS STORM [Ptot=113.66 mm] | | 15.0 | | | | | | |
| ** CALIB NASHYD [CN=60.6 [N = 3.0:Tp 1.17] | 0101 | 1 14.0 | 56.01 | 1.61 | 12.83 | 41.25 | 0.36 | 0.000 |

```

MASS STORM 15.0
[ Ptot=113.66 mm ]
** CALIB NASHYD 0102 1 13.0 6.35 0.20 12.78 41.40 0.36 0.000
[CN=59.0
[ N = 3.0:Tp 1.07]
MASS STORM 15.0
[ Ptot=113.66 mm ]
** CALIB NASHYD 0106 1 2.0 0.58 0.12 11.77 60.57 0.53 0.000
[CN=74.0
[ N = 3.0:Tp 0.11]
MASS STORM 15.0
[ Ptot=113.66 mm ]
** CALIB STANDHYD 1001 1 10.0 0.35 0.05 11.67 55.14 0.49 0.000
[I%=10.0:S%= 2.00]
MASS STORM 15.0
[ Ptot=113.66 mm ]
** CALIB STANDHYD 0100 1 10.0 0.90 0.14 11.67 62.77 0.55 0.000
[I%=30.0:S%= 2.00]
MASS STORM 15.0
[ Ptot=113.66 mm ]
** CALIB STANDHYD 0105 1 10.0 1.70 0.24 11.83 64.11 0.56 0.000
[I%=25.0:S%= 2.00]
ADD [ 0100+ 1001] 0901 3 10.0 1.25 0.19 11.67 60.64 n/a 0.000
ADD [ 0901+ 0102] 0901 1 10.0 7.60 0.24 11.83 44.56 n/a 0.000
ADD [ 0901+ 0105] 0901 3 10.0 9.30 0.49 11.83 48.13 n/a 0.000
ADD [ 0901+ 0106] 0901 1 2.0 9.88 0.60 11.77 48.86 n/a 0.000
** Reservoir
OUTFLOW: 0401 1 1.0 9.88 0.21 13.37 48.86 n/a 0.000
OVERFLOW: 0401 3 1.0 0.00 0.00 0.00 0.00 n/a 0.000
ADD [ 0101+ 0401] 0902 3 1.0 65.89 1.82 12.83 42.39 n/a 0.000

```

```

=====
V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL

```

```

000 TTTTT TTTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
000 T T H H Y M M 000

```

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2021 Smart City Water Inc
All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\vojn.dat
Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\7ac27cbf-e558
Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\7ac27cbf-e558

DATE: 01-19-2022 TIME: 07:47:50
USER:

COMMENTS: _____

** SIMULATION : Run 12 - 100 Year 24 Hour SCS **

```

W/E COMMAND HYD ID DT AREA Qpeak Tpeak R.V. R.C. Qbase
min ha cms hrs mm cms
START @ 0.00 hrs
-----
MASS STORM 15.0
[ Ptot=124.82 mm ]
** CALIB NASHYD 0101 1 14.0 56.01 1.90 12.83 48.42 0.39 0.000
[CN=60.6
[ N = 3.0:Tp 1.17]
MASS STORM 15.0
[ Ptot=124.82 mm ]
** CALIB NASHYD 0102 1 13.0 6.35 0.23 12.78 48.45 0.39 0.000
[CN=59.0
[ N = 3.0:Tp 1.07]
MASS STORM 15.0
[ Ptot=124.82 mm ]
** CALIB NASHYD 0106 1 2.0 0.58 0.14 11.77 69.60 0.56 0.000
[CN=74.0
[ N = 3.0:Tp 0.11]
MASS STORM 15.0
[ Ptot=124.82 mm ]
** CALIB STANDHYD 1001 1 10.0 0.35 0.06 11.67 63.34 0.51 0.000
[I%=10.0:S%= 2.00]
MASS STORM 15.0
[ Ptot=124.82 mm ]
** CALIB STANDHYD 0100 1 10.0 0.90 0.15 11.67 71.06 0.57 0.000
[I%=30.0:S%= 2.00]
MASS STORM 15.0
[ Ptot=124.82 mm ]
** CALIB STANDHYD 0105 1 10.0 1.70 0.36 11.67 72.74 0.58 0.000
[I%=25.0:S%= 2.00]
ADD [ 0100+ 1001] 0901 3 10.0 1.25 0.21 11.67 68.90 n/a 0.000
ADD [ 0901+ 0102] 0901 1 10.0 7.60 0.28 11.83 51.81 n/a 0.000
ADD [ 0901+ 0105] 0901 3 10.0 9.30 0.63 11.67 55.63 n/a 0.000
ADD [ 0901+ 0106] 0901 1 2.0 9.88 0.74 11.70 56.45 n/a 0.000
** Reservoir
OUTFLOW: 0401 1 1.0 9.88 0.22 13.52 56.45 n/a 0.000
OVERFLOW: 0401 3 1.0 0.00 0.00 0.00 0.00 n/a 0.000
ADD [ 0101+ 0401] 0902 3 1.0 65.89 2.12 12.83 49.63 n/a 0.000

```

```

=====
V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL

```

```

000 TTTTT TTTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
000 T T H H Y M M 000

```

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2021 Smart City Water Inc
All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\vojn.dat
Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\5c7c965-d945
Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\5c7c965-d945

DATE: 01-19-2022

TIME: 07:47:51

USER:

COMMENTS: _____

** SIMULATION : Run 13 - 25 mm **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|--|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| ----- | | | | | | | | |
| READ STORM | | | 10.0 | | | | | |
| [Ptot= 25.02 mm] | | | | | | | | |
| fname : C:\Users\LBuss\AppData\Local\Temp\d95ec570-3058-4f9a-a149-a08543fbf702\754080c7-048d-4c71-b842-e68 | | | | | | | | |
| remark: 25MMCHI | | | | | | | | |
| ** CALIB NASHYD | 0101 | 1 14.0 | 56.01 | 0.08 | 3.73 | 1.58 | 0.06 | 0.000 |
| [CN=60.6] | | | | | | | | |
| [N = 3.0:Tp 1.17] | | | | | | | | |
| READ STORM | | | 10.0 | | | | | |
| [Ptot= 25.02 mm] | | | | | | | | |
| fname : C:\Users\LBuss\AppData\Local\Temp\d95ec570-3058-4f9a-a149-a08543fbf702\754080c7-048d-4c71-b842-e68 | | | | | | | | |
| remark: 25MMCHI | | | | | | | | |
| ** CALIB NASHYD | 0102 | 1 13.0 | 6.35 | 0.01 | 3.25 | 2.04 | 0.08 | 0.000 |
| [CN=59.0] | | | | | | | | |
| [N = 3.0:Tp 1.07] | | | | | | | | |
| READ STORM | | | 10.0 | | | | | |
| [Ptot= 25.02 mm] | | | | | | | | |
| fname : C:\Users\LBuss\AppData\Local\Temp\d95ec570-3058-4f9a-a149-a08543fbf702\754080c7-048d-4c71-b842-e68 | | | | | | | | |
| remark: 25MMCHI | | | | | | | | |
| ** CALIB NASHYD | 0106 | 1 2.0 | 0.58 | 0.01 | 1.57 | 4.07 | 0.16 | 0.000 |
| [CN=74.0] | | | | | | | | |
| [N = 3.0:Tp 0.11] | | | | | | | | |
| READ STORM | | | 10.0 | | | | | |
| [Ptot= 25.02 mm] | | | | | | | | |
| fname : C:\Users\LBuss\AppData\Local\Temp\d95ec570-3058-4f9a-a149-a08543fbf702\754080c7-048d-4c71-b842-e68 | | | | | | | | |
| remark: 25MMCHI | | | | | | | | |
| * CALIB STANDHYD | 1001 | 1 10.0 | 0.35 | 0.00 | 1.50 | 4.89 | 0.20 | 0.000 |
| [I%=10.0:S%= 2.00] | | | | | | | | |
| READ STORM | | | 10.0 | | | | | |
| [Ptot= 25.02 mm] | | | | | | | | |
| fname : C:\Users\LBuss\AppData\Local\Temp\d95ec570-3058-4f9a-a149-a08543fbf702\754080c7-048d-4c71-b842-e68 | | | | | | | | |
| remark: 25MMCHI | | | | | | | | |
| * CALIB STANDHYD | 0100 | 1 10.0 | 0.90 | 0.03 | 1.50 | 8.61 | 0.34 | 0.000 |
| [I%=30.0:S%= 2.00] | | | | | | | | |
| READ STORM | | | 10.0 | | | | | |
| [Ptot= 25.02 mm] | | | | | | | | |
| fname : C:\Users\LBuss\AppData\Local\Temp\d95ec570-3058-4f9a-a149-a08543fbf702\754080c7-048d-4c71-b842-e68 | | | | | | | | |
| remark: 25MMCHI | | | | | | | | |
| * CALIB STANDHYD | 0105 | 1 10.0 | 1.70 | 0.05 | 1.50 | 8.04 | 0.32 | 0.000 |
| [I%=25.0:S%= 2.00] | | | | | | | | |
| ADD [0100+ 1001] | 0901 | 3 10.0 | 1.25 | 0.03 | 1.50 | 7.57 | n/a | 0.000 |
| ADD [0901+ 0102] | 0901 | 1 10.0 | 7.60 | 0.04 | 1.50 | 2.95 | n/a | 0.000 |
| ADD [0901+ 0105] | 0901 | 3 10.0 | 9.30 | 0.08 | 1.50 | 3.88 | n/a | 0.000 |
| ADD [0901+ 0106] | 0901 | 1 2.0 | 9.88 | 0.09 | 1.50 | 3.89 | n/a | 0.000 |
| ** Reservoir | | | | | | | | |
| OUTFLOW: | 0401 | 1 1.0 | 9.88 | 0.04 | 1.72 | 3.89 | n/a | 0.000 |
| OVERFLOW: | 0401 | 3 1.0 | 0.00 | 0.00 | 0.00 | 0.00 | n/a | 0.000 |
| ADD [0101+ 0401] | 0902 | 3 1.0 | 65.89 | 0.10 | 3.50 | 1.93 | n/a | 0.000 |

FINISH

 ** SIMULATION:Run 01 - 2 Year 4 Hour Chicago **

| RESERVOIR(0401) | | | | |
|--|---------|---------|---------|------|
| OVERFLOW IS OFF | | | | |
| IN= 2---> OUT= 1 | | | | |
| DT= 1.0 min | | | | |
| OUTFLOW | STORAGE | OUTFLOW | STORAGE | |
| (cms) | (ha.m.) | (cms) | (ha.m.) | |
| 0.0000 | 0.0000 | 0.2450 | 0.1640 | |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 | |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 | |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 | |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 | |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 | |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 | |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 | |
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) | |
| INFLOW : ID= 2 (0901) | 9.880 | 0.171 | 1.33 | 6.31 |
| OUTFLOW: ID= 1 (0401) | 9.880 | 0.068 | 1.50 | 6.31 |
| PEAK FLOW REDUCTION [Qout/Qin](%)= 40.14 | | | | |
| TIME SHIFT OF PEAK FLOW (min)= 10.00 | | | | |
| MAXIMUM STORAGE USED (ha.m.)= 0.0096 | | | | |

 ** SIMULATION:Run 02 - 5 year 4 Hour Chicago **

| RESERVOIR(0401) | | | | |
|--|---------|---------|---------|-------|
| OVERFLOW IS OFF | | | | |
| IN= 2---> OUT= 1 | | | | |
| DT= 1.0 min | | | | |
| OUTFLOW | STORAGE | OUTFLOW | STORAGE | |
| (cms) | (ha.m.) | (cms) | (ha.m.) | |
| 0.0000 | 0.0000 | 0.2450 | 0.1640 | |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 | |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 | |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 | |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 | |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 | |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 | |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 | |
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) | |
| INFLOW : ID= 2 (0901) | 9.880 | 0.246 | 1.33 | 20.26 |
| OUTFLOW: ID= 1 (0401) | 9.880 | 0.103 | 1.57 | 10.26 |
| PEAK FLOW REDUCTION [Qout/Qin](%)= 41.93 | | | | |
| TIME SHIFT OF PEAK FLOW (min)= 14.00 | | | | |
| MAXIMUM STORAGE USED (ha.m.)= 0.0145 | | | | |

 ** SIMULATION:Run 03 - 10 Year 4 Hour Chicago **

| RESERVOIR(0401) | | | | |
|--|---------|---------|---------|-------|
| OVERFLOW IS OFF | | | | |
| IN= 2---> OUT= 1 | | | | |
| DT= 1.0 min | | | | |
| OUTFLOW | STORAGE | OUTFLOW | STORAGE | |
| (cms) | (ha.m.) | (cms) | (ha.m.) | |
| 0.0000 | 0.0000 | 0.2450 | 0.1640 | |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 | |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 | |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 | |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 | |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 | |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 | |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 | |
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) | |
| INFLOW : ID= 2 (0901) | 9.880 | 0.301 | 1.33 | 13.26 |
| OUTFLOW: ID= 1 (0401) | 9.880 | 0.129 | 1.62 | 13.26 |
| PEAK FLOW REDUCTION [Qout/Qin](%)= 42.71 | | | | |
| TIME SHIFT OF PEAK FLOW (min)= 17.00 | | | | |
| MAXIMUM STORAGE USED (ha.m.)= 0.0184 | | | | |

 ** SIMULATION:Run 04 - 25 Year 4 Hour Chicago **

| RESERVOIR(0401) | | | | |
|--|---------|---------|---------|-------|
| OVERFLOW IS OFF | | | | |
| IN= 2---> OUT= 1 | | | | |
| DT= 1.0 min | | | | |
| OUTFLOW | STORAGE | OUTFLOW | STORAGE | |
| (cms) | (ha.m.) | (cms) | (ha.m.) | |
| 0.0000 | 0.0000 | 0.2450 | 0.1640 | |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 | |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 | |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 | |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 | |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 | |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 | |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 | |
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) | |
| INFLOW : ID= 2 (0901) | 9.880 | 0.370 | 1.33 | 17.33 |
| OUTFLOW: ID= 1 (0401) | 9.880 | 0.138 | 1.87 | 17.33 |
| PEAK FLOW REDUCTION [Qout/Qin](%)= 37.19 | | | | |
| TIME SHIFT OF PEAK FLOW (min)= 32.00 | | | | |
| MAXIMUM STORAGE USED (ha.m.)= 0.0264 | | | | |

 ** SIMULATION:Run 05 - 50 Year 4 Hour Chicago **

| RESERVOIR(0401) | | | | |
|--|---------|---------|---------|-------|
| OVERFLOW IS OFF | | | | |
| IN= 2---> OUT= 1 | | | | |
| DT= 1.0 min | | | | |
| OUTFLOW | STORAGE | OUTFLOW | STORAGE | |
| (cms) | (ha.m.) | (cms) | (ha.m.) | |
| 0.0000 | 0.0000 | 0.2450 | 0.1640 | |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 | |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 | |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 | |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 | |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 | |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 | |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 | |
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) | |
| INFLOW : ID= 2 (0901) | 9.880 | 0.429 | 1.33 | 20.66 |
| OUTFLOW: ID= 1 (0401) | 9.880 | 0.149 | 2.37 | 20.66 |
| PEAK FLOW REDUCTION [Qout/Qin](%)= 34.68 | | | | |
| TIME SHIFT OF PEAK FLOW (min)= 62.00 | | | | |
| MAXIMUM STORAGE USED (ha.m.)= 0.0359 | | | | |

 ** SIMULATION:Run 06 - 100 Year 4 Hour Chicago **

| RESERVOIR(0401) | | | | |
|--|---------|---------|---------|-------|
| OVERFLOW IS OFF | | | | |
| IN= 2---> OUT= 1 | | | | |
| DT= 1.0 min | | | | |
| OUTFLOW | STORAGE | OUTFLOW | STORAGE | |
| (cms) | (ha.m.) | (cms) | (ha.m.) | |
| 0.0000 | 0.0000 | 0.2450 | 0.1640 | |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 | |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 | |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 | |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 | |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 | |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 | |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 | |
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) | |
| INFLOW : ID= 2 (0901) | 9.880 | 0.486 | 1.33 | 24.10 |
| OUTFLOW: ID= 1 (0401) | 9.880 | 0.162 | 2.87 | 24.10 |
| PEAK FLOW REDUCTION [Qout/Qin](%)= 33.26 | | | | |
| TIME SHIFT OF PEAK FLOW (min)= 92.00 | | | | |
| MAXIMUM STORAGE USED (ha.m.)= 0.0486 | | | | |

 ** SIMULATION:Run 07 - 2 Year 24 Hour SCS **

| RESERVOIR(0401) | | | | |
|------------------|--|--|--|--|
| OVERFLOW IS OFF | | | | |
| IN= 2---> OUT= 1 | | | | |

DT= 1.0 min

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.2450 | 0.1640 |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 |

| INFLOW : ID= 2 (0901) | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|------------------------|-----------|-------------|-------------|-----------|
| OUTFLOW: ID= 1 (0401) | 9.880 | 0.204 | 11.73 | 15.78 |
| | 9.880 | 0.124 | 11.97 | 15.78 |

| PEAK FLOW REDUCTION [Qout/Qin](%) | TIME SHIFT OF PEAK FLOW (min) | MAXIMUM STORAGE USED (ha.m.) |
|-----------------------------------|-------------------------------|------------------------------|
| | | 60.73 |
| | | 14.00 |
| | | 0.0175 |

 ** SIMULATION:Run 08 - 5 Year 24 Hour SCS **

RESERVOIR(0401)
 IN= 2--> OUT= 1
 DT= 1.0 min

OVERFLOW IS OFF

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.2450 | 0.1640 |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 |

| INFLOW : ID= 2 (0901) | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|------------------------|-----------|-------------|-------------|-----------|
| OUTFLOW: ID= 1 (0401) | 9.880 | 0.313 | 11.77 | 25.18 |
| | 9.880 | 0.146 | 12.37 | 25.18 |

| PEAK FLOW REDUCTION [Qout/Qin](%) | TIME SHIFT OF PEAK FLOW (min) | MAXIMUM STORAGE USED (ha.m.) |
|-----------------------------------|-------------------------------|------------------------------|
| | | 46.49 |
| | | 36.00 |
| | | 0.0333 |

 ** SIMULATION:Run 09 - 10 Year 24 Hour SCS **

RESERVOIR(0401)
 IN= 2--> OUT= 1
 DT= 1.0 min

OVERFLOW IS OFF

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.2450 | 0.1640 |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 |

| INFLOW : ID= 2 (0901) | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|------------------------|-----------|-------------|-------------|-----------|
| OUTFLOW: ID= 1 (0401) | 9.880 | 0.398 | 11.77 | 32.18 |
| | 9.880 | 0.163 | 12.80 | 32.18 |

| PEAK FLOW REDUCTION [Qout/Qin](%) | TIME SHIFT OF PEAK FLOW (min) | MAXIMUM STORAGE USED (ha.m.) |
|-----------------------------------|-------------------------------|------------------------------|
| | | 41.06 |
| | | 62.00 |
| | | 0.0503 |

 ** SIMULATION:Run 10 - 25 Year 24 Hour SCS **

RESERVOIR(0401)
 IN= 2--> OUT= 1
 DT= 1.0 min

OVERFLOW IS OFF

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.2450 | 0.1640 |

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.1280 | 0.0180 | 0.2580 | 0.1870 |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 |

| INFLOW : ID= 2 (0901) | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|------------------------|-----------|-------------|-------------|-----------|
| OUTFLOW: ID= 1 (0401) | 9.880 | 0.515 | 11.77 | 41.45 |
| | 9.880 | 0.188 | 13.18 | 41.45 |

| PEAK FLOW REDUCTION [Qout/Qin](%) | TIME SHIFT OF PEAK FLOW (min) | MAXIMUM STORAGE USED (ha.m.) |
|-----------------------------------|-------------------------------|------------------------------|
| | | 36.48 |
| | | 85.00 |
| | | 0.0780 |

 ** SIMULATION:Run 11 - 50 Year 24 Hour SCS **

RESERVOIR(0401)
 IN= 2--> OUT= 1
 DT= 1.0 min

OVERFLOW IS OFF

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.2450 | 0.1640 |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 |

| INFLOW : ID= 2 (0901) | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|------------------------|-----------|-------------|-------------|-----------|
| OUTFLOW: ID= 1 (0401) | 9.880 | 0.604 | 11.77 | 48.86 |
| | 9.880 | 0.207 | 13.37 | 48.86 |

| PEAK FLOW REDUCTION [Qout/Qin](%) | TIME SHIFT OF PEAK FLOW (min) | MAXIMUM STORAGE USED (ha.m.) |
|-----------------------------------|-------------------------------|------------------------------|
| | | 34.19 |
| | | 96.00 |
| | | 0.1030 |

 ** SIMULATION:Run 12 - 100 Year 24 Hour SCS **

RESERVOIR(0401)
 IN= 2--> OUT= 1
 DT= 1.0 min

OVERFLOW IS OFF

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.2450 | 0.1640 |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 |

| INFLOW : ID= 2 (0901) | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|------------------------|-----------|-------------|-------------|-----------|
| OUTFLOW: ID= 1 (0401) | 9.880 | 0.737 | 11.70 | 56.45 |
| | 9.880 | 0.225 | 13.52 | 56.45 |

| PEAK FLOW REDUCTION [Qout/Qin](%) | TIME SHIFT OF PEAK FLOW (min) | MAXIMUM STORAGE USED (ha.m.) |
|-----------------------------------|-------------------------------|------------------------------|
| | | 30.48 |
| | | 109.00 |
| | | 0.1294 |

 ** SIMULATION:Run 13 - 25 mm **

RESERVOIR(0401)
 IN= 2--> OUT= 1
 DT= 1.0 min

OVERFLOW IS OFF

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.2450 | 0.1640 |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 |

| | | | |
|--------|--------|--------|--------|
| 0.1870 | 0.0770 | 0.2920 | 0.2610 |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 |

| | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|------------------------|--------------|----------------|----------------|--------------|
| INFLOW : ID= 2 (0901) | 9.880 | 0.088 | 1.50 | 3.89 |
| OUTFLOW: ID= 1 (0401) | 9.880 | 0.040 | 1.72 | 3.89 |

PEAK FLOW REDUCTION [Qout/Qin](%)= 46.21
 TIME SHIFT OF PEAK FLOW (min)= 13.00
 MAXIMUM STORAGE USED (ha.m.)= 0.0057

=====

V V I SSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
VV I SSSS UUUU A A LLLLL

000 TTTTT TTTTT H H Y Y M M 000 TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
000 T T H H Y Y M M 000

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2021 Smart City Water Inc
All rights reserved.

***** S U M M A R Y O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\996988ba-2d1e
Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\996988ba-2d1e

DATE: 01-19-2022 TIME: 07:49:03

USER:

COMMENTS: _____

** SIMULATION : Run 01 - Hazel **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|---|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| ----- | | | | | | | | |
| READ STORM [Ptot=212.00 mm] fname : C:\Users\LBuss\AppData\Local\Temp\6ac572ca-ed69-4a4b-9290-f80646da26db\b3895df3-8984-4913-9a29-d81 remark: HAZEL.STM | | | | | 12.0 | | | |
| ** CALIB NASHYD [CN=78.0] [N = 3.0:Tp 1.17] | 0101 | 1 14.0 | 56.01 | 5.07 | 11.20 | 149.79 | 0.71 | 0.000 |
| READ STORM [Ptot=212.00 mm] fname : C:\Users\LBuss\AppData\Local\Temp\6ac572ca-ed69-4a4b-9290-f80646da26db\b3895df3-8984-4913-9a29-d81 remark: HAZEL.STM | | | | | 12.0 | | | |
| ** CALIB NASHYD [CN=77.0] [N = 3.0:Tp 1.07] | 0102 | 1 13.0 | 6.35 | 0.59 | 11.05 | 150.46 | 0.71 | 0.000 |
| READ STORM [Ptot=212.00 mm] fname : C:\Users\LBuss\AppData\Local\Temp\6ac572ca-ed69-4a4b-9290-f80646da26db\b3895df3-8984-4913-9a29-d81 remark: HAZEL.STM | | | | | 12.0 | | | |
| ** CALIB NASHYD [CN=87.0] [N = 3.0:Tp 0.07] | 0106 | 1 2.0 | 0.58 | 0.08 | 10.00 | 175.51 | 0.83 | 0.000 |
| READ STORM [Ptot=212.00 mm] fname : C:\Users\LBuss\AppData\Local\Temp\6ac572ca-ed69-4a4b-9290-f80646da26db\b3895df3-8984-4913-9a29-d81 remark: HAZEL.STM | | | | | 12.0 | | | |
| * CALIB STANDHYD [I%=10.0:S%= 2.00] | 1001 | 1 10.0 | 0.35 | 0.05 | 10.00 | 167.00 | 0.79 | 0.000 |
| READ STORM [Ptot=212.00 mm] fname : C:\Users\LBuss\AppData\Local\Temp\6ac572ca-ed69-4a4b-9290-f80646da26db\b3895df3-8984-4913-9a29-d81 remark: HAZEL.STM | | | | | 12.0 | | | |
| * CALIB STANDHYD | 0100 | 1 10.0 | 0.90 | 0.12 | 10.00 | 169.33 | 0.80 | 0.000 |

| | | | | | | | | |
|---|------|--------|-------|------|-------|--------|------|-------|
| * [I%=30.0:S%= 2.00] | | | | | | | | |
| READ STORM | | | | | 12.0 | | | |
| [Ptot=212.00 mm] fname : C:\Users\LBuss\AppData\Local\Temp\6ac572ca-ed69-4a4b-9290-f80646da26db\b3895df3-8984-4913-9a29-d81 remark: HAZEL.STM | | | | | | | | |
| * CALIB STANDHYD [I%=25.0:S%= 2.00] | 0105 | 1 10.0 | 1.70 | 0.23 | 10.00 | 173.51 | 0.82 | 0.000 |
| * ADD [0100+ 1001] | 0901 | 3 10.0 | 1.25 | 0.17 | 10.00 | 168.68 | n/a | 0.000 |
| * ADD [0901+ 0102] | 0901 | 1 10.0 | 7.60 | 0.71 | 11.00 | 153.45 | n/a | 0.000 |
| * ADD [0901+ 0105] | 0901 | 3 10.0 | 9.30 | 0.88 | 11.00 | 157.12 | n/a | 0.000 |
| * ADD [0901+ 0106] | 0901 | 1 2.0 | 9.88 | 0.94 | 11.00 | 158.20 | n/a | 0.000 |
| * ADD [0101+ 0901] | 0902 | 3 2.0 | 65.89 | 5.91 | 11.00 | 151.05 | n/a | 0.000 |
| FINISH | | | | | | | | |

=====

Proposed Conditions Hydrology

Visual OTTHYMO Model Parameter Calculations (NasHYD)

Project Details

| | |
|----------------|--------|
| Melville Court | 420427 |
|----------------|--------|

Data Sources

Detailed Soil Survey Reports for Ontario, LSRCA Technical Guidelines for Stormwater Management Submissions (2016), MTO Drainage Management Manual (1997)

Prepared By

| | |
|----|------------|
| LB | 12-15-2021 |
|----|------------|

Post Development Condition

| | |
|----------------------|-------|
| Watershed: | LSRCA |
| Catchment ID: | 201 |
| Catchment Area (ha): | 56.24 |
| Impervious %: | 6% |

Average Curve Number (CN), Runoff Coefficient (C) and Initial Abstraction (IA)

| Soil Symbol | Gus | | | | | | | | | | | | | |
|-------------------------|-----------|--------|----|------|--------|----|---|--------|----|---|--------|----|---|--|
| Soil Series | Guerin | | | | | | | | | | | | | |
| Hydrologic Soils Group | AB | | | | | | | | | | | | | |
| Soil Texture | Sand Loam | | | | | | | | | | | | | |
| Runoff Coefficient Type | 1 | | | | | | | | | | | | | |
| Area (ha) | 56.24 | | | | | | | | | | | | | |
| Percentage of Catchment | 100% | | | | | | | | | | | | | |
| Land Cover Category | IA | A (ha) | CN | C | A (ha) | CN | C | A (ha) | CN | C | A (ha) | CN | C | |
| Impervious | 2 | 3.17 | 98 | 0.95 | | | | | | | | | | |
| Gravel | 3 | | 81 | 0.09 | | | | | | | | | | |
| Woodland | 10 | 22.05 | 46 | 0.08 | | | | | | | | | | |
| Pasture/Lawns | 5 | | 59 | 0.10 | | | | | | | | | | |
| Meadows | 8 | | 51 | 0.09 | | | | | | | | | | |
| Cultivated | 7 | 31.16 | 68 | 0.22 | | | | | | | | | | |
| Waterbody | 12 | | 50 | 0.05 | | | | | | | | | | |
| Average CN | 61.23 | | | | | | | | | | | | | |
| Average C | 0.21 | | | | | | | | | | | | | |
| Average IA | 7.91 | | | | | | | | | | | | | |

Time to Peak Calculations

| | |
|-------------------------------|----------------|
| Max. Catchment Elev. (m): | 304.00 |
| Min. Catchment Elev. (m): | 288.00 |
| Catchment Length (m): | 1400 |
| Catchment Slope (%): | 1.14% |
| Method: | Airport Method |
| Time of Concentration (mins): | 104.25 |

Summary

| | |
|-------------------------------|-------|
| Catchment CN: | 61.2 |
| Catchment C: | 0.21 |
| Catchment IA (mm): | 7.91 |
| Time of Concentration (hrs): | 1.74 |
| Catchment Time to Peak (hrs): | 1.16 |
| Catchment Time Step (mins): | 13.90 |

Note: Catchment Impervious adjusted by 0.24 ha to account for development of Lots 15 & 16 - negligible impact to Ex. CN

Visual OTTHYMO Model Parameter Calculations (NasHYD)

Project Details

| | |
|----------------|--------|
| Melville Court | 420427 |
|----------------|--------|

Data Sources

| |
|--|
| Detailed Soil Survey Reports for Ontario, LSRCA Technical Guidelines for Stormwater Management Submissions (2016), MTO Drainage Management Manual (1997) |
|--|

Prepared By

| | |
|----|------------|
| LB | 12-15-2021 |
|----|------------|

Post Development Condition

| | |
|----------------------|-------|
| Watershed: | LSRCA |
| Catchment ID: | 202 |
| Catchment Area (ha): | 6.45 |
| Impervious %: | 14% |

Average Curve Number (CN), Runoff Coefficient (C) and Initial Abstraction (IA)

| Soil Symbol | Vasl | | | | | | | | | | | | |
|-------------------------|-----------|--------|----|------|--------|----|---|--------|----|---|--------|----|---|
| Soil Series | Vasey | | | | | | | | | | | | |
| Hydrologic Soils Group | AB | | | | | | | | | | | | |
| Soil Texture | Sand Loam | | | | | | | | | | | | |
| Runoff Coefficient Type | 1 | | | | | | | | | | | | |
| Area (ha) | 6.45 | | | | | | | | | | | | |
| Percentage of Catchment | 100% | | | | | | | | | | | | |
| Land Cover Category | IA | A (ha) | CN | C | A (ha) | CN | C | A (ha) | CN | C | A (ha) | CN | C |
| Impervious | 2 | 0.91 | 98 | 0.95 | | | | | | | | | |
| Gravel | 3 | | 81 | 0.09 | | | | | | | | | |
| Woodland | 10 | | 46 | 0.08 | | | | | | | | | |
| Pasture/Lawns | 5 | 4.01 | 59 | 0.10 | | | | | | | | | |
| Meadows | 8 | | 51 | 0.09 | | | | | | | | | |
| Cultivated | 7 | 1.53 | 68 | 0.22 | | | | | | | | | |
| Waterbody | 12 | | 50 | 0.05 | | | | | | | | | |
| Average CN | 66.64 | | | | | | | | | | | | |
| Average C | 0.25 | | | | | | | | | | | | |
| Average IA | 5.05 | | | | | | | | | | | | |

Time to Peak Calculations

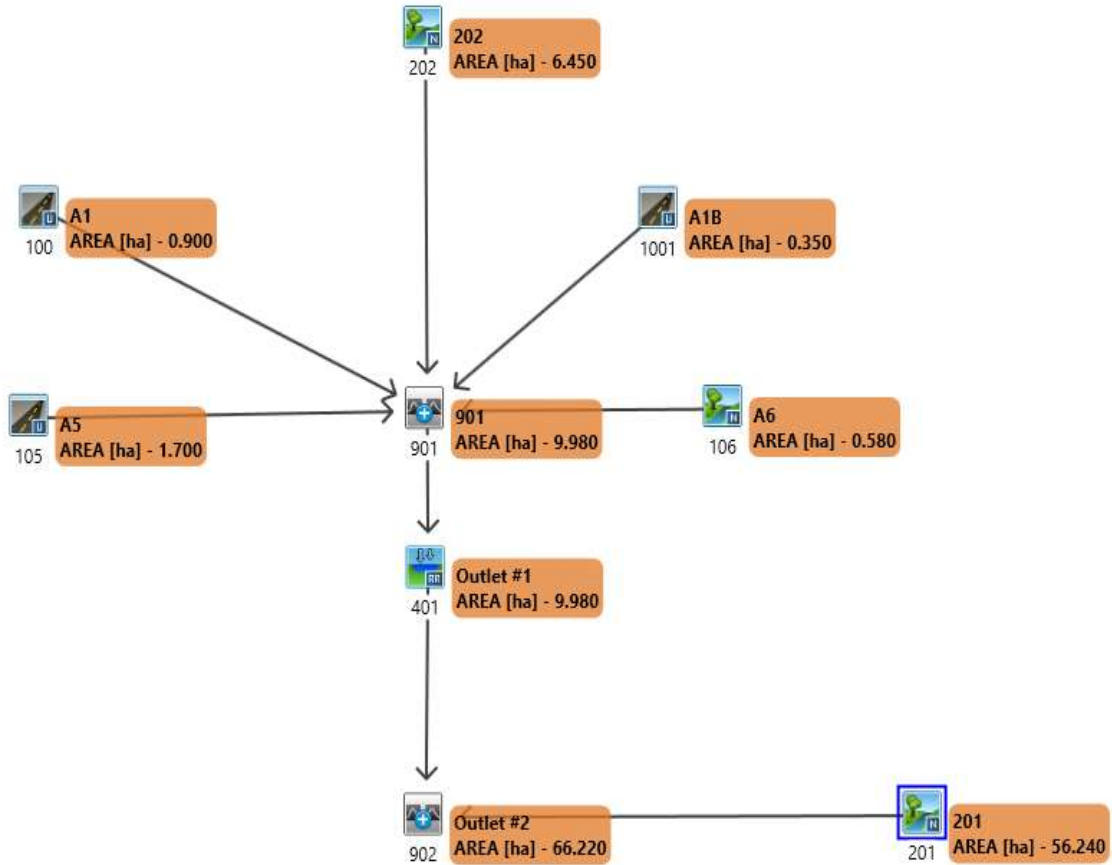
| | |
|-------------------------------|--------|
| Max. Catchment Elev. (m): | 298.00 |
| Min. Catchment Elev. (m): | 288.52 |
| Catchment Length (m): | 900 |
| Catchment Slope (%): | 1.05% |
| Method: Airport Method | |
| Time of Concentration (mins): | 81.87 |

Summary

| | |
|-------------------------------|-------|
| Catchment CN: | 66.6 |
| Catchment C: | 0.25 |
| Catchment IA (mm): | 5.05 |
| Time of Concentration (hrs): | 1.36 |
| Catchment Time to Peak (hrs): | 0.91 |
| Catchment Time Step (mins): | 10.92 |

| | | | |
|---------|----------------------------------|------|------------|
| PROJECT | Melville Court Development | FILE | 420427 |
| | | DATE | 12-27-2021 |
| SUBJECT | Proposed Conditions VO Schematic | NAME | LB |
| | | PAGE | 1 OF 1 |

PROPOSED CONDITIONS



NASHYD



ROUTE PIPE



DUHYD



STANDHYD



ROUTE CHANNEL



DIVERT HYD



ADDHYD



ROUTE RESERVOIR



SHIFTHYD


```

* CALIB STANDHYD      0105  1 10.0   1.70   0.13  1.33  17.28  0.39   0.000
  [I%=25.0:S%= 2.00]
*
* CHIC STORM          10.0
  [ Ptot= 43.85 mm ]
*
* CALIB STANDHYD      0100  1 10.0   0.90   0.08  1.33  17.75  0.40   0.000
  [I%=30.0:S%= 2.00]
*
* ADD [ 0100+ 1001]  0901  3 10.0   1.25   0.09  1.33  16.24  n/a   0.000
*
* ADD [ 0901+ 0105]  0901  1 10.0   2.95   0.22  1.33  16.84  n/a   0.000
*
* ADD [ 0901+ 0106]  0901  3  1.0   3.53   0.24  1.33  16.11  n/a   0.000
*
* ADD [ 0901+ 0202]  0901  1  1.0   9.98   0.25  1.33  11.55  n/a   0.000
*
** Reservoir
OUTFLOW:              0401  1  1.0   9.98   0.11  1.62  11.55  n/a   0.000
OVERFLOW:            0401  3  1.0   0.00   0.00  0.00   0.00  n/a   0.000
*
* ADD [ 0201+ 0401]  0902  3  1.0  66.22   0.40  2.80   7.28  n/a   0.000

```

```

** CALIB NASHYD      0106  1  1.0   0.58   0.04  1.40  16.30  0.32   0.000
  [CN=74.0
  [ N = 3.0:Tp 0.11]
*
* CHIC STORM          10.0
  [ Ptot= 50.98 mm ]
*
* CALIB STANDHYD      1001  1 10.0   0.35   0.02  1.33  15.79  0.31   0.000
  [I%=10.0:S%= 2.00]
*
* CHIC STORM          10.0
  [ Ptot= 50.98 mm ]
*
* CALIB STANDHYD      0105  1 10.0   1.70   0.15  1.33  21.29  0.42   0.000
  [I%=25.0:S%= 2.00]
*
* CHIC STORM          10.0
  [ Ptot= 50.98 mm ]
*
* CALIB STANDHYD      0100  1 10.0   0.90   0.09  1.33  21.63  0.42   0.000
  [I%=30.0:S%= 2.00]
*
* ADD [ 0100+ 1001]  0901  3 10.0   1.25   0.11  1.33  20.00  n/a   0.000
*
* ADD [ 0901+ 0105]  0901  1 10.0   2.95   0.26  1.33  20.74  n/a   0.000
*
* ADD [ 0901+ 0106]  0901  3  1.0   3.53   0.29  1.33  20.01  n/a   0.000
*
* ADD [ 0901+ 0202]  0901  1  1.0   9.98   0.30  1.33  14.94  n/a   0.000
*
** Reservoir
OUTFLOW:              0401  1  1.0   9.98   0.13  1.80  14.94  n/a   0.000
OVERFLOW:            0401  3  1.0   0.00   0.00  0.00   0.00  n/a   0.000
*
* ADD [ 0201+ 0401]  0902  3  1.0  66.22   0.56  2.80   9.93  n/a   0.000

```

```

=====
V  V  I  SSSSS  U  U  A  L          (v 6.2.2008)
V  V  I  SS    U  U  A  A  L
V  V  I  SS    U  U  AAAAA  L
V  V  I  SS    U  U  A  A  L
VV   I  SSSSS  UUUUU  A  A  LLLLL

  000  TTTT  TTTT  H  H  Y  Y  M  M  000  TM
  O  O  T  T  T  H  H  Y  Y  MM MM  O  O
  O  O  T  T  T  H  H  Y  Y  M  M  O  O
  000  T  T  H  H  Y  M  M  000

```

```

=====
V  V  I  SSSSS  U  U  A  L          (v 6.2.2008)
V  V  I  SS    U  U  A  A  L
V  V  I  SS    U  U  AAAAA  L
V  V  I  SS    U  U  A  A  L
VV   I  SSSSS  UUUUU  A  A  LLLLL

  000  TTTT  TTTT  H  H  Y  Y  M  M  000  TM
  O  O  T  T  T  H  H  Y  Y  MM MM  O  O
  O  O  T  T  T  H  H  Y  Y  M  M  O  O
  000  T  T  H  H  Y  M  M  000

```

Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2021 Smart City Water Inc
 All rights reserved.

Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2021 Smart City Water Inc
 All rights reserved.

***** S U M M A R Y O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
 Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\89b38bf4-2c6f
 Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\89b38bf4-2c6f

***** S U M M A R Y O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
 Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\278df801-4d88
 Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\278df801-4d88

DATE: 01-19-2022 TIME: 08:00:39
 USER:

DATE: 01-19-2022 TIME: 08:00:39
 USER:

COMMENTS: _____

COMMENTS: _____

 ** SIMULATION : Run 03 - 10 Year 4 Hour Chica **

 ** SIMULATION : Run 04 - 25 year 4 Hour Chica **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|---|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| CHIC STORM [Ptot= 50.98 mm] | | 10.0 | | | | | | |
| ** CALIB NASHYD [CN=61.2 [N = 3.0:Tp 1.16] | 0201 | 1 14.0 | 56.24 | 0.44 | 3.03 | 9.04 | 0.18 | 0.000 |
| CHIC STORM [Ptot= 50.98 mm] | | 10.0 | | | | | | |
| ** CALIB NASHYD [CN=66.6 [N = 3.0:Tp 0.91] | 0202 | 1 11.0 | 6.45 | 0.08 | 2.57 | 12.17 | 0.24 | 0.000 |
| CHIC STORM [Ptot= 50.98 mm] | | 10.0 | | | | | | |

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|----------------------------------|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| CHIC STORM [Ptot= 59.78 mm] | | 10.0 | | | | | | |
| ** CALIB NASHYD [CN=61.2 | 0201 | 1 14.0 | 56.24 | 0.61 | 2.80 | 12.57 | 0.21 | 0.000 |

```

* [ N = 3.0:Tp 1.16]
* CHIC STORM 10.0
* [ Ptot= 59.78 mm ]
** CALIB NASHYD 0202 1 11.0 6.45 0.11 2.57 16.45 0.28 0.000
* [CN=66.6 ]
* [ N = 3.0:Tp 0.91]
* CHIC STORM 10.0
* [ Ptot= 59.78 mm ]
** CALIB NASHYD 0106 1 1.0 0.58 0.05 1.40 21.57 0.36 0.000
* [CN=74.0 ]
* [ N = 3.0:Tp 0.11]
* CHIC STORM 10.0
* [ Ptot= 59.78 mm ]
** CALIB STANDHYD 1001 1 10.0 0.35 0.02 1.33 20.41 0.34 0.000
* [I%=10.0:S%= 2.00]
* CHIC STORM 10.0
* [ Ptot= 59.78 mm ]
** CALIB STANDHYD 0105 1 10.0 1.70 0.19 1.33 26.54 0.44 0.000
* [I%=25.0:S%= 2.00]
* CHIC STORM 10.0
* [ Ptot= 59.78 mm ]
** CALIB STANDHYD 0100 1 10.0 0.90 0.11 1.33 26.71 0.45 0.000
* [I%=30.0:S%= 2.00]
* ADD [ 0100+ 1001] 0901 3 10.0 1.25 0.13 1.33 24.94 n/a 0.000
* ADD [ 0901+ 0105] 0901 1 10.0 2.95 0.32 1.33 25.86 n/a 0.000
* ADD [ 0901+ 0106] 0901 3 1.0 3.53 0.36 1.33 25.15 n/a 0.000
* ADD [ 0901+ 0202] 0901 1 1.0 9.98 0.37 1.33 19.53 n/a 0.000
** Reservoir
OUTFLOW: 0401 1 1.0 9.98 0.15 2.75 19.53 n/a 0.000
OVERFLOW: 0401 3 1.0 0.00 0.00 0.00 0.00 n/a 0.000
* ADD [ 0201+ 0401] 0902 3 1.0 66.22 0.76 2.80 13.62 n/a 0.000

```

```

=====
V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUU A A LLLLL
000 TTTTT TTTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
000 T T H H Y M M 000

```

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2021 Smart City Water Inc
All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\vojn.dat
Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\76f288ba-1bc0
Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\76f288ba-1bc0

DATE: 01-19-2022 TIME: 08:00:39

USER:

COMMENTS: _____

** SIMULATION : Run 05 - 50 year 4 Hour Chica **

```

*****
W/E COMMAND HYD ID DT AREA Qpeak Tpeak R.V. R.C. Qbase
min ha cms hrs mm cms
-----
START @ 0.00 hrs
CHIC STORM 10.0
[ Ptot= 66.46 mm ]
** CALIB NASHYD 0201 1 14.0 56.24 0.76 2.80 15.53 0.23 0.000
* [CN=61.2 ]
* [ N = 3.0:Tp 1.16]
* CHIC STORM 10.0
* [ Ptot= 66.46 mm ]
** CALIB NASHYD 0202 1 11.0 6.45 0.13 2.38 19.97 0.30 0.000
* [CN=66.6 ]
* [ N = 3.0:Tp 0.91]
* CHIC STORM 10.0
* [ Ptot= 66.46 mm ]
** CALIB NASHYD 0106 1 1.0 0.58 0.06 1.40 25.83 0.39 0.000
* [CN=74.0 ]
* [ N = 3.0:Tp 0.11]
* CHIC STORM 10.0
* [ Ptot= 66.46 mm ]
** CALIB STANDHYD 1001 1 10.0 0.35 0.02 1.33 24.16 0.36 0.000
* [I%=10.0:S%= 2.00]
* CHIC STORM 10.0
* [ Ptot= 66.46 mm ]
** CALIB STANDHYD 0105 1 10.0 1.70 0.21 1.33 30.73 0.46 0.000
* [I%=25.0:S%= 2.00]
* CHIC STORM 10.0
* [ Ptot= 66.46 mm ]
** CALIB STANDHYD 0100 1 10.0 0.90 0.13 1.33 30.74 0.46 0.000
* [I%=30.0:S%= 2.00]
* ADD [ 0100+ 1001] 0901 3 10.0 1.25 0.15 1.33 28.90 n/a 0.000
* ADD [ 0901+ 0105] 0901 1 10.0 2.95 0.36 1.33 29.95 n/a 0.000
* ADD [ 0901+ 0106] 0901 3 1.0 3.53 0.41 1.33 29.27 n/a 0.000
* ADD [ 0901+ 0202] 0901 1 1.0 9.98 0.43 1.33 23.26 n/a 0.000
** Reservoir
OUTFLOW: 0401 1 1.0 9.98 0.16 2.98 23.26 n/a 0.000
OVERFLOW: 0401 3 1.0 0.00 0.00 0.00 0.00 n/a 0.000
* ADD [ 0201+ 0401] 0902 3 1.0 66.22 0.93 2.80 16.70 n/a 0.000

```

```

=====
V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUU A A LLLLL
000 TTTTT TTTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
000 T T H H Y M M 000

```

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2021 Smart City Water Inc
All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\vojn.dat
Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\8e28d8ed-1336
Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\8e28d8ed-1336

DATE: 01-19-2022 TIME: 08:00:39
 USER:

COMMENTS: _____

 ** SIMULATION : Run 06 - 100 Year 4 Hour Chic **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|---|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| CHIC STORM [Ptot= 72.98 mm] | 10.0 | | | | | | | |
| ** CALIB NASHYD [CN=61.2 [N = 3.0:Tp 1.16] | 0201 | 1 14.0 | 56.24 | 0.92 | 2.80 | 18.64 | 0.26 | 0.000 |
| CHIC STORM [Ptot= 72.98 mm] | 10.0 | | | | | | | |
| ** CALIB NASHYD [CN=66.6 [N = 3.0:Tp 0.91] | 0202 | 1 11.0 | 6.45 | 0.16 | 2.38 | 23.62 | 0.32 | 0.000 |
| CHIC STORM [Ptot= 72.98 mm] | 10.0 | | | | | | | |
| ** CALIB NASHYD [CN=74.0 [N = 3.0:Tp 0.11] | 0106 | 1 1.0 | 0.58 | 0.08 | 1.40 | 30.19 | 0.41 | 0.000 |
| CHIC STORM [Ptot= 72.98 mm] | 10.0 | | | | | | | |
| ** CALIB STANDHYD [I%=10.0:S%= 2.00] | 1001 | 1 10.0 | 0.35 | 0.03 | 1.33 | 28.01 | 0.38 | 0.000 |
| CHIC STORM [Ptot= 72.98 mm] | 10.0 | | | | | | | |
| ** CALIB STANDHYD [I%=25.0:S%= 2.00] | 0105 | 1 10.0 | 1.70 | 0.24 | 1.33 | 34.97 | 0.48 | 0.000 |
| CHIC STORM [Ptot= 72.98 mm] | 10.0 | | | | | | | |
| ** CALIB STANDHYD [I%=30.0:S%= 2.00] | 0100 | 1 10.0 | 0.90 | 0.14 | 1.33 | 34.82 | 0.48 | 0.000 |
| ADD [0100+ 1001] | 0901 | 3 10.0 | 1.25 | 0.17 | 1.33 | 32.91 | n/a | 0.000 |
| ADD [0901+ 0105] | 0901 | 1 10.0 | 2.95 | 0.41 | 1.33 | 34.10 | n/a | 0.000 |
| ADD [0901+ 0106] | 0901 | 3 1.0 | 3.53 | 0.47 | 1.33 | 33.45 | n/a | 0.000 |
| ADD [0901+ 0202] | 0901 | 1 1.0 | 9.98 | 0.49 | 1.33 | 27.10 | n/a | 0.000 |
| ** Reservoir OUTFLOW: | 0401 | 1 1.0 | 9.98 | 0.18 | 3.15 | 27.10 | n/a | 0.000 |
| ADD [0201+ 0401] | 0902 | 3 1.0 | 66.22 | 1.10 | 2.80 | 19.91 | n/a | 0.000 |

```

V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
000 T T H H Y M M 000
  
```

Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2021 Smart City Water Inc
 All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
 Output filename: C:\Users\LBuss\AppData\Local\Civica\XH5\18b1f826-8929-408e-9936-e2d934269cf1\c6e62356-3b9f
 Summary filename: C:\Users\LBuss\AppData\Local\Civica\XH5\18b1f826-8929-408e-9936-e2d934269cf1\c6e62356-3b9f

DATE: 01-19-2022 TIME: 08:00:39
 USER:

COMMENTS: _____

 ** SIMULATION : Run 07 - 2 Year 24 Hour SCS **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|---|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| MASS STORM [Ptot= 56.53 mm] | 15.0 | | | | | | | |
| ** CALIB NASHYD [CN=61.2 [N = 3.0:Tp 1.16] | 0201 | 1 14.0 | 56.24 | 0.42 | 12.83 | 11.27 | 0.20 | 0.000 |
| MASS STORM [Ptot= 56.53 mm] | 15.0 | | | | | | | |
| ** CALIB NASHYD [CN=66.6 [N = 3.0:Tp 0.91] | 0202 | 1 11.0 | 6.45 | 0.08 | 12.65 | 14.82 | 0.26 | 0.000 |
| MASS STORM [Ptot= 56.53 mm] | 15.0 | | | | | | | |
| ** CALIB NASHYD [CN=74.0 [N = 3.0:Tp 0.11] | 0106 | 1 1.0 | 0.58 | 0.04 | 11.78 | 16.03 | 0.28 | 0.000 |
| MASS STORM [Ptot= 56.53 mm] | 15.0 | | | | | | | |
| ** CALIB STANDHYD [I%=10.0:S%= 2.00] | 1001 | 1 10.0 | 0.35 | 0.01 | 11.83 | 18.65 | 0.33 | 0.000 |
| MASS STORM [Ptot= 56.53 mm] | 15.0 | | | | | | | |
| ** CALIB STANDHYD [I%=25.0:S%= 2.00] | 0105 | 1 10.0 | 1.70 | 0.10 | 11.67 | 24.56 | 0.43 | 0.000 |
| MASS STORM [Ptot= 56.53 mm] | 15.0 | | | | | | | |
| ** CALIB STANDHYD [I%=30.0:S%= 2.00] | 0100 | 1 10.0 | 0.90 | 0.06 | 11.67 | 24.80 | 0.44 | 0.000 |
| ADD [0100+ 1001] | 0901 | 3 10.0 | 1.25 | 0.07 | 11.67 | 23.08 | n/a | 0.000 |
| ADD [0901+ 0105] | 0901 | 1 10.0 | 2.95 | 0.17 | 11.67 | 23.93 | n/a | 0.000 |
| ADD [0901+ 0106] | 0901 | 3 1.0 | 3.53 | 0.19 | 11.67 | 23.21 | n/a | 0.000 |
| ADD [0901+ 0202] | 0901 | 1 1.0 | 9.98 | 0.21 | 11.77 | 17.79 | n/a | 0.000 |
| ** Reservoir OUTFLOW: | 0401 | 1 1.0 | 9.98 | 0.13 | 12.10 | 17.79 | n/a | 0.000 |
| OVERFLOW: | 0401 | 3 1.0 | 0.00 | 0.00 | 0.00 | 0.00 | n/a | 0.000 |
| ADD [0201+ 0401] | 0902 | 3 1.0 | 66.22 | 0.54 | 12.83 | 12.26 | n/a | 0.000 |


```

* CALIB STANDHYD      0105  1 10.0   1.70   0.17 11.67  44.71 0.51   0.000
  [I%=25.0:S%= 2.00]
*
* MASS STORM
  [ Ptot= 87.24 mm ]      15.0
*
* CALIB STANDHYD      0100  1 10.0   0.90   0.10 11.67  44.16 0.51   0.000
  [I%=30.0:S%= 2.00]
*
* ADD [ 0100+ 1001]    0901  3 10.0   1.25   0.12 11.67  42.14 n/a   0.000
*
* ADD [ 0901+ 0105]    0901  1 10.0   2.95   0.29 11.67  43.62 n/a   0.000
*
* ADD [ 0901+ 0106]    0901  3  1.0   3.53   0.36 11.77  43.07 n/a   0.000
*
* ADD [ 0901+ 0202]    0901  1  1.0   9.98   0.43 11.78  36.06 n/a   0.000
*
** Reservoir
OUTFLOW:              0401  1  1.0   9.98   0.18 13.12  36.06 n/a   0.000
OVERFLOW:             0401  3  1.0   0.00   0.00  0.00   0.00 n/a   0.000
*
* ADD [ 0201+ 0401]    0902  3  1.0  66.22   1.20 12.83  27.67 n/a   0.000

```

```

[CN=74.0
 [ N = 3.0:Tp 0.11]
*
* MASS STORM
  [ Ptot=102.29 mm ]      15.0
*
* CALIB STANDHYD      1001  1 10.0   0.35   0.04 11.67  47.09 0.46   0.000
  [I%=10.0:S%= 2.00]
*
* MASS STORM
  [ Ptot=102.29 mm ]      15.0
*
* CALIB STANDHYD      0105  1 10.0   1.70   0.21 11.67  55.57 0.54   0.000
  [I%=25.0:S%= 2.00]
*
* MASS STORM
  [ Ptot=102.29 mm ]      15.0
*
* CALIB STANDHYD      0100  1 10.0   0.90   0.12 11.67  54.58 0.53   0.000
  [I%=30.0:S%= 2.00]
*
* ADD [ 0100+ 1001]    0901  3 10.0   1.25   0.16 11.67  52.48 n/a   0.000
*
* ADD [ 0901+ 0105]    0901  1 10.0   2.95   0.37 11.67  54.26 n/a   0.000
*
* ADD [ 0901+ 0106]    0901  3  1.0   3.53   0.46 11.77  53.83 n/a   0.000
*
* ADD [ 0901+ 0202]    0901  1  1.0   9.98   0.55 11.78  46.25 n/a   0.000
*
** Reservoir
OUTFLOW:              0401  1  1.0   9.98   0.21 13.32  46.24 n/a   0.000
OVERFLOW:             0401  3  1.0   0.00   0.00  0.00   0.00 n/a   0.000
*
* ADD [ 0201+ 0401]    0902  3  1.0  66.22   1.58 12.83  36.59 n/a   0.000

```

```

=====
V  V  I  SSSSS  U  U  A  L          (v 6.2.2008)
V  V  I  SS    U  U  A  A  L
V  V  I  SS    U  U  AAAAA L
V  V  I  SS    U  U  A  A  L
VV    I  SSSSS  UUUUU  A  A  LLLLL
OOO  TTTT  TTTT  H  H  Y  Y  M  M  OOO  TM
O  O  T  T  H  H  Y  Y  MM  MM  O  O
O  O  T  T  H  H  Y  Y  M  M  O  O
OOO  T  T  H  H  Y  Y  M  M  OOO

```

```

=====
V  V  I  SSSSS  U  U  A  L          (v 6.2.2008)
V  V  I  SS    U  U  A  A  L
V  V  I  SS    U  U  AAAAA L
V  V  I  SS    U  U  A  A  L
VV    I  SSSSS  UUUUU  A  A  LLLLL
OOO  TTTT  TTTT  H  H  Y  Y  M  M  OOO  TM
O  O  T  T  H  H  Y  Y  MM  MM  O  O
O  O  T  T  H  H  Y  Y  M  M  O  O
OOO  T  T  H  H  Y  Y  M  M  OOO

```

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2021 Smart City Water Inc
All rights reserved.

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2021 Smart City Water Inc
All rights reserved.

***** SUMMARY OUTPUT *****

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\8f96b8db-0a62
Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\8f96b8db-0a62

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\01cb6be1-719a
Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\01cb6be1-719a

DATE: 01-19-2022 TIME: 08:00:39

DATE: 01-19-2022 TIME: 08:00:38

USER:

USER:

COMMENTS: _____

COMMENTS: _____

** SIMULATION : Run 10 - 25 Year 24 Hour SCS **

** SIMULATION : Run 11 - 50 Year 24 Hour SCS **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|---|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| MASS STORM [Ptot=102.29 mm] | | | | | | | | |
| ** CALIB NASHYD [CN=61.2 [N = 3.0:Tp 1.16] | 0201 | 1 14.0 | 56.24 | 1.37 | 12.83 | 34.87 | 0.34 | 0.000 |
| MASS STORM [Ptot=102.29 mm] | | | | | | | | |
| ** CALIB NASHYD [CN=66.6 [N = 3.0:Tp 0.91] | 0202 | 1 11.0 | 6.45 | 0.23 | 12.65 | 42.09 | 0.41 | 0.000 |
| MASS STORM [Ptot=102.29 mm] | | | | | | | | |
| ** CALIB NASHYD | 0106 | 1 1.0 | 0.58 | 0.10 | 11.78 | 43.36 | 0.42 | 0.000 |

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|---|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| MASS STORM [Ptot=113.66 mm] | | | | | | | | |
| ** CALIB NASHYD [CN=61.2 [N = 3.0:Tp 1.16] | 0201 | 1 14.0 | 56.24 | 1.66 | 12.83 | 41.91 | 0.37 | 0.000 |

```

* MASS STORM 15.0
[ Ptot=113.66 mm ]
** CALIB NASHYD 0202 1 11.0 6.45 0.27 12.65 49.98 0.44 0.000
[CN=66.6
[ N = 3.0:Tp 0.91]
* MASS STORM 15.0
[ Ptot=113.66 mm ]
** CALIB NASHYD 0106 1 1.0 0.58 0.12 11.78 51.06 0.45 0.000
[CN=74.0
[ N = 3.0:Tp 0.11]
* MASS STORM 15.0
[ Ptot=113.66 mm ]
** CALIB STANDHYD 1001 1 10.0 0.35 0.05 11.67 55.14 0.49 0.000
[I%=10.0:S%= 2.00]
* MASS STORM 15.0
[ Ptot=113.66 mm ]
** CALIB STANDHYD 0105 1 10.0 1.70 0.24 11.83 64.11 0.56 0.000
[I%=25.0:S%= 2.00]
* MASS STORM 15.0
[ Ptot=113.66 mm ]
** CALIB STANDHYD 0100 1 10.0 0.90 0.14 11.67 62.77 0.55 0.000
[I%=30.0:S%= 2.00]
* ADD [ 0100+ 1001] 0901 3 10.0 1.25 0.19 11.67 60.64 n/a 0.000
* ADD [ 0901+ 0105] 0901 1 10.0 2.95 0.43 11.67 62.64 n/a 0.000
* ADD [ 0901+ 0106] 0901 3 1.0 3.53 0.54 11.77 62.30 n/a 0.000
* ADD [ 0901+ 0202] 0901 1 1.0 9.98 0.65 11.80 54.34 n/a 0.000
** Reservoir
OUTFLOW: 0401 1 1.0 9.98 0.23 13.43 54.34 n/a 0.000
OVERFLOW: 0401 3 1.0 0.00 0.00 0.00 0.00 n/a 0.000
* ADD [ 0201+ 0401] 0902 3 1.0 66.22 1.88 12.83 43.79 n/a 0.000

```

```

V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL
000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
000 T T H H Y M M 000

```

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2021 Smart City Water Inc
All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\0ee8413a-fc6a
Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\0ee8413a-fc6a

DATE: 01-19-2022 TIME: 08:00:39

USER:

COMMENTS: _____

** SIMULATION : Run 12 -100 Year 24 Hour SCS **

```

W/E COMMAND HYD ID DT AREA Qpeak Tpeak R.V. R.C. Qbase
min ha cms hrs mm
-----
START @ 0.00 hrs
MASS STORM 15.0
[ Ptot=124.82 mm ]
** CALIB NASHYD 0201 1 14.0 56.24 1.96 12.83 49.17 0.39 0.000
[CN=61.2
[ N = 3.0:Tp 1.16]
* MASS STORM 15.0
[ Ptot=124.82 mm ]
** CALIB NASHYD 0202 1 11.0 6.45 0.32 12.65 58.04 0.46 0.000
[CN=66.6
[ N = 3.0:Tp 0.91]
* MASS STORM 15.0
[ Ptot=124.82 mm ]
** CALIB NASHYD 0106 1 1.0 0.58 0.14 11.78 58.88 0.47 0.000
[CN=74.0
[ N = 3.0:Tp 0.11]
* MASS STORM 15.0
[ Ptot=124.82 mm ]
** CALIB STANDHYD 1001 1 10.0 0.35 0.06 11.67 63.34 0.51 0.000
[I%=10.0:S%= 2.00]
* MASS STORM 15.0
[ Ptot=124.82 mm ]
** CALIB STANDHYD 0105 1 10.0 1.70 0.36 11.67 72.74 0.58 0.000
[I%=25.0:S%= 2.00]
* MASS STORM 15.0
[ Ptot=124.82 mm ]
** CALIB STANDHYD 0100 1 10.0 0.90 0.15 11.67 71.06 0.57 0.000
[I%=30.0:S%= 2.00]
* ADD [ 0100+ 1001] 0901 3 10.0 1.25 0.21 11.67 68.90 n/a 0.000
* ADD [ 0901+ 0105] 0901 1 10.0 2.95 0.57 11.67 71.11 n/a 0.000
* ADD [ 0901+ 0106] 0901 3 1.0 3.53 0.67 11.67 70.87 n/a 0.000
* ADD [ 0901+ 0202] 0901 1 1.0 9.98 0.78 11.77 62.58 n/a 0.000
** Reservoir
OUTFLOW: 0401 1 1.0 9.98 0.25 13.53 62.58 n/a 0.000
OVERFLOW: 0401 3 1.0 0.00 0.00 0.00 0.00 n/a 0.000
* ADD [ 0201+ 0401] 0902 3 1.0 66.22 2.20 12.83 51.19 n/a 0.000

```

```

V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL
000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
000 T T H H Y M M 000

```

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2021 Smart City Water Inc
All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\f39dfbbe-4663
Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\f39dfbbe-4663

DATE: 01-19-2022

TIME: 08:00:39

USER:

COMMENTS: _____

** SIMULATION : Run 13 - 25 mm **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|--|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| ----- | | | | | | | | |
| READ STORM | | | 10.0 | | | | | |
| [Ptot= 25.02 mm] | | | | | | | | |
| fname : C:\Users\LBuss\AppData\Local\Temp\b0e9d416-8a90-45d8-a051-ce89f11ec7a3\754080c7-048d-4c71-b842-e68 | | | | | | | | |
| remark: 25MMCHI | | | | | | | | |
| ** CALIB NASHYD | 0201 | 1 14.0 | 56.24 | 0.08 | 3.73 | 1.63 | 0.07 | 0.000 |
| [CN=61.2 | | | | | | | | |
| [N = 3.0:Tp 1.16] | | | | | | | | |
| READ STORM | | | 10.0 | | | | | |
| [Ptot= 25.02 mm] | | | | | | | | |
| fname : C:\Users\LBuss\AppData\Local\Temp\b0e9d416-8a90-45d8-a051-ce89f11ec7a3\754080c7-048d-4c71-b842-e68 | | | | | | | | |
| remark: 25MMCHI | | | | | | | | |
| ** CALIB NASHYD | 0202 | 1 11.0 | 6.45 | 0.02 | 2.93 | 2.71 | 0.11 | 0.000 |
| [CN=66.6 | | | | | | | | |
| [N = 3.0:Tp 0.91] | | | | | | | | |
| READ STORM | | | 10.0 | | | | | |
| [Ptot= 25.02 mm] | | | | | | | | |
| fname : C:\Users\LBuss\AppData\Local\Temp\b0e9d416-8a90-45d8-a051-ce89f11ec7a3\754080c7-048d-4c71-b842-e68 | | | | | | | | |
| remark: 25MMCHI | | | | | | | | |
| ** CALIB NASHYD | 0106 | 1 1.0 | 0.58 | 0.01 | 1.58 | 4.07 | 0.16 | 0.000 |
| [CN=74.0 | | | | | | | | |
| [N = 3.0:Tp 0.11] | | | | | | | | |
| READ STORM | | | 10.0 | | | | | |
| [Ptot= 25.02 mm] | | | | | | | | |
| fname : C:\Users\LBuss\AppData\Local\Temp\b0e9d416-8a90-45d8-a051-ce89f11ec7a3\754080c7-048d-4c71-b842-e68 | | | | | | | | |
| remark: 25MMCHI | | | | | | | | |
| * CALIB STANDHYD | 1001 | 1 10.0 | 0.35 | 0.00 | 1.50 | 4.89 | 0.20 | 0.000 |
| [I%=10.0:S%= 2.00] | | | | | | | | |
| READ STORM | | | 10.0 | | | | | |
| [Ptot= 25.02 mm] | | | | | | | | |
| fname : C:\Users\LBuss\AppData\Local\Temp\b0e9d416-8a90-45d8-a051-ce89f11ec7a3\754080c7-048d-4c71-b842-e68 | | | | | | | | |
| remark: 25MMCHI | | | | | | | | |
| * CALIB STANDHYD | 0105 | 1 10.0 | 1.70 | 0.05 | 1.50 | 8.04 | 0.32 | 0.000 |
| [I%=25.0:S%= 2.00] | | | | | | | | |
| READ STORM | | | 10.0 | | | | | |
| [Ptot= 25.02 mm] | | | | | | | | |
| fname : C:\Users\LBuss\AppData\Local\Temp\b0e9d416-8a90-45d8-a051-ce89f11ec7a3\754080c7-048d-4c71-b842-e68 | | | | | | | | |
| remark: 25MMCHI | | | | | | | | |
| * CALIB STANDHYD | 0100 | 1 10.0 | 0.90 | 0.03 | 1.50 | 8.61 | 0.34 | 0.000 |
| [I%=30.0:S%= 2.00] | | | | | | | | |
| ADD [0100+ 1001] | 0901 | 3 10.0 | 1.25 | 0.03 | 1.50 | 7.57 | n/a | 0.000 |
| ADD [0901+ 0105] | 0901 | 1 10.0 | 2.95 | 0.08 | 1.50 | 7.84 | n/a | 0.000 |
| ADD [0901+ 0106] | 0901 | 3 1.0 | 3.53 | 0.09 | 1.50 | 7.22 | n/a | 0.000 |
| ADD [0901+ 0202] | 0901 | 1 1.0 | 9.98 | 0.09 | 1.50 | 4.30 | n/a | 0.000 |
| ** Reservoir | | | | | | | | |
| OUTFLOW: | 0401 | 1 1.0 | 9.98 | 0.04 | 1.73 | 4.30 | n/a | 0.000 |
| OVERFLOW: | 0401 | 3 1.0 | 0.00 | 0.00 | 0.00 | 0.00 | n/a | 0.000 |
| ADD [0201+ 0401] | 0902 | 3 1.0 | 66.22 | 0.11 | 3.50 | 2.03 | n/a | 0.000 |

FINISH

 ** SIMULATION:Run 01 - 2 Year 4 Hour Chicago **

| RESERVOIR(0401) | | | | |
|--|---------|---------|---------|--|
| IN= 2--> OUT= 1 | | | | |
| DT= 1.0 min | | | | |
| OVERFLOW IS OFF | | | | |
| OUTFLOW | STORAGE | OUTFLOW | STORAGE | |
| (cms) | (ha.m.) | (cms) | (ha.m.) | |
| 0.0000 | 0.0000 | 0.2450 | 0.1640 | |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 | |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 | |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 | |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 | |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 | |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 | |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 | |
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) | |
| 9.980 | 0.171 | 1.33 | 7.07 | |
| 9.980 | 0.069 | 1.50 | 7.07 | |
| INFLW : ID= 2 (0901) | | | | |
| OUTFLOW: ID= 1 (0401) | | | | |
| PEAK FLOW REDUCTION [Qout/Qin](%)= 40.53 | | | | |
| TIME SHIFT OF PEAK FLOW (min)= 10.00 | | | | |
| MAXIMUM STORAGE USED (ha.m.)= 0.0097 | | | | |

 ** SIMULATION:Run 02 - 5 Year 4 Hour Chicago **

| RESERVOIR(0401) | | | | |
|--|---------|---------|---------|--|
| IN= 2--> OUT= 1 | | | | |
| DT= 1.0 min | | | | |
| OVERFLOW IS OFF | | | | |
| OUTFLOW | STORAGE | OUTFLOW | STORAGE | |
| (cms) | (ha.m.) | (cms) | (ha.m.) | |
| 0.0000 | 0.0000 | 0.2450 | 0.1640 | |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 | |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 | |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 | |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 | |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 | |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 | |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 | |
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) | |
| 9.980 | 0.247 | 1.33 | 11.55 | |
| 9.980 | 0.106 | 1.62 | 11.55 | |
| INFLW : ID= 2 (0901) | | | | |
| OUTFLOW: ID= 1 (0401) | | | | |
| PEAK FLOW REDUCTION [Qout/Qin](%)= 43.04 | | | | |
| TIME SHIFT OF PEAK FLOW (min)= 17.00 | | | | |
| MAXIMUM STORAGE USED (ha.m.)= 0.0150 | | | | |

 ** SIMULATION:Run 03 - 10 Year 4 Hour Chicago **

| RESERVOIR(0401) | | | | |
|--|---------|---------|---------|--|
| IN= 2--> OUT= 1 | | | | |
| DT= 1.0 min | | | | |
| OVERFLOW IS OFF | | | | |
| OUTFLOW | STORAGE | OUTFLOW | STORAGE | |
| (cms) | (ha.m.) | (cms) | (ha.m.) | |
| 0.0000 | 0.0000 | 0.2450 | 0.1640 | |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 | |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 | |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 | |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 | |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 | |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 | |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 | |
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) | |
| 9.980 | 0.302 | 1.33 | 14.94 | |
| 9.980 | 0.130 | 1.80 | 14.94 | |
| INFLW : ID= 2 (0901) | | | | |
| OUTFLOW: ID= 1 (0401) | | | | |
| PEAK FLOW REDUCTION [Qout/Qin](%)= 42.95 | | | | |
| TIME SHIFT OF PEAK FLOW (min)= 28.00 | | | | |
| MAXIMUM STORAGE USED (ha.m.)= 0.0196 | | | | |

 ** SIMULATION:Run 04 - 25 year 4 Hour Chicago **

| RESERVOIR(0401) | | | | |
|--|---------|---------|---------|--|
| IN= 2--> OUT= 1 | | | | |
| DT= 1.0 min | | | | |
| OVERFLOW IS OFF | | | | |
| OUTFLOW | STORAGE | OUTFLOW | STORAGE | |
| (cms) | (ha.m.) | (cms) | (ha.m.) | |
| 0.0000 | 0.0000 | 0.2450 | 0.1640 | |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 | |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 | |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 | |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 | |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 | |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 | |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 | |
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) | |
| 9.980 | 0.373 | 1.33 | 19.53 | |
| 9.980 | 0.148 | 2.75 | 19.53 | |
| INFLW : ID= 2 (0901) | | | | |
| OUTFLOW: ID= 1 (0401) | | | | |
| PEAK FLOW REDUCTION [Qout/Qin](%)= 39.64 | | | | |
| TIME SHIFT OF PEAK FLOW (min)= 85.00 | | | | |
| MAXIMUM STORAGE USED (ha.m.)= 0.0351 | | | | |

 ** SIMULATION:Run 05 - 50 year 4 Hour Chicago **

| RESERVOIR(0401) | | | | |
|--|---------|---------|---------|--|
| IN= 2--> OUT= 1 | | | | |
| DT= 1.0 min | | | | |
| OVERFLOW IS OFF | | | | |
| OUTFLOW | STORAGE | OUTFLOW | STORAGE | |
| (cms) | (ha.m.) | (cms) | (ha.m.) | |
| 0.0000 | 0.0000 | 0.2450 | 0.1640 | |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 | |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 | |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 | |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 | |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 | |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 | |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 | |
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) | |
| 9.980 | 0.432 | 1.33 | 23.26 | |
| 9.980 | 0.164 | 2.98 | 23.26 | |
| INFLW : ID= 2 (0901) | | | | |
| OUTFLOW: ID= 1 (0401) | | | | |
| PEAK FLOW REDUCTION [Qout/Qin](%)= 37.91 | | | | |
| TIME SHIFT OF PEAK FLOW (min)= 99.00 | | | | |
| MAXIMUM STORAGE USED (ha.m.)= 0.0508 | | | | |

 ** SIMULATION:Run 06 - 100 Year 4 Hour Chicago **

| RESERVOIR(0401) | | | | |
|--|---------|---------|---------|--|
| IN= 2--> OUT= 1 | | | | |
| DT= 1.0 min | | | | |
| OVERFLOW IS OFF | | | | |
| OUTFLOW | STORAGE | OUTFLOW | STORAGE | |
| (cms) | (ha.m.) | (cms) | (ha.m.) | |
| 0.0000 | 0.0000 | 0.2450 | 0.1640 | |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 | |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 | |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 | |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 | |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 | |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 | |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 | |
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) | |
| 9.980 | 0.490 | 1.33 | 27.10 | |
| 9.980 | 0.180 | 3.15 | 27.10 | |
| INFLW : ID= 2 (0901) | | | | |
| OUTFLOW: ID= 1 (0401) | | | | |
| PEAK FLOW REDUCTION [Qout/Qin](%)= 36.66 | | | | |
| TIME SHIFT OF PEAK FLOW (min)=109.00 | | | | |
| MAXIMUM STORAGE USED (ha.m.)= 0.0683 | | | | |

 ** SIMULATION:Run 07 - 2 Year 24 Hour SCS **

| RESERVOIR(0401) | | | | |
|------------------|--|--|--|--|
| IN= 2--> OUT= 1 | | | | |
| OVERFLOW IS OFF | | | | |

DT= 1.0 min

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.2450 | 0.1640 |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 |

| AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-----------|-------------|-------------|-----------|
| 9.980 | 0.214 | 11.77 | 17.79 |
| 9.980 | 0.129 | 12.10 | 17.79 |

| PEAK FLOW REDUCTION [Qout/Qin](%) | TIME SHIFT OF PEAK FLOW (min) | MAXIMUM STORAGE USED (ha.m.) |
|-----------------------------------|-------------------------------|------------------------------|
| 60.45 | 20.00 | 0.0191 |

| AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-----------|-------------|-------------|-----------|
| 9.980 | 0.552 | 11.78 | 46.25 |
| 9.980 | 0.209 | 13.32 | 46.24 |

| PEAK FLOW REDUCTION [Qout/Qin](%) | TIME SHIFT OF PEAK FLOW (min) | MAXIMUM STORAGE USED (ha.m.) |
|-----------------------------------|-------------------------------|------------------------------|
| 37.83 | 92.00 | 0.1060 |

**** SIMULATION:Run 08 - 5 Year 24 Hour SCS ****

RESERVOIR(0401)
 IN= 2--> OUT= 1
 DT= 1.0 min

OVERFLOW IS OFF

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.2450 | 0.1640 |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 |

| AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-----------|-------------|-------------|-----------|
| 9.980 | 0.334 | 11.78 | 28.31 |
| 9.980 | 0.158 | 12.88 | 28.31 |

| PEAK FLOW REDUCTION [Qout/Qin](%) | TIME SHIFT OF PEAK FLOW (min) | MAXIMUM STORAGE USED (ha.m.) |
|-----------------------------------|-------------------------------|------------------------------|
| 47.37 | 66.00 | 0.0450 |

**** SIMULATION:Run 09 - 10 Year 24 Hour SCS ****

RESERVOIR(0401)
 IN= 2--> OUT= 1
 DT= 1.0 min

OVERFLOW IS OFF

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.2450 | 0.1640 |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 |

| AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-----------|-------------|-------------|-----------|
| 9.980 | 0.426 | 11.78 | 36.06 |
| 9.980 | 0.181 | 13.12 | 36.06 |

| PEAK FLOW REDUCTION [Qout/Qin](%) | TIME SHIFT OF PEAK FLOW (min) | MAXIMUM STORAGE USED (ha.m.) |
|-----------------------------------|-------------------------------|------------------------------|
| 42.48 | 80.00 | 0.0698 |

**** SIMULATION:Run 10 - 25 Year 24 Hour SCS ****

RESERVOIR(0401)
 IN= 2--> OUT= 1
 DT= 1.0 min

OVERFLOW IS OFF

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.2450 | 0.1640 |

**** SIMULATION:Run 11 - 50 Year 24 Hour SCS ****

RESERVOIR(0401)
 IN= 2--> OUT= 1
 DT= 1.0 min

OVERFLOW IS OFF

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.2450 | 0.1640 |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 |

| AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-----------|-------------|-------------|-----------|
| 9.980 | 0.650 | 11.80 | 54.34 |
| 9.980 | 0.229 | 13.43 | 54.34 |

| PEAK FLOW REDUCTION [Qout/Qin](%) | TIME SHIFT OF PEAK FLOW (min) | MAXIMUM STORAGE USED (ha.m.) |
|-----------------------------------|-------------------------------|------------------------------|
| 35.30 | 98.00 | 0.1369 |

**** SIMULATION:Run 12 -100 Year 24 Hour SCS ****

RESERVOIR(0401)
 IN= 2--> OUT= 1
 DT= 1.0 min

OVERFLOW IS OFF

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.2450 | 0.1640 |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 |
| 0.1870 | 0.0770 | 0.2920 | 0.2610 |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 |

| AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-----------|-------------|-------------|-----------|
| 9.980 | 0.776 | 11.77 | 62.58 |
| 9.980 | 0.248 | 13.53 | 62.58 |

| PEAK FLOW REDUCTION [Qout/Qin](%) | TIME SHIFT OF PEAK FLOW (min) | MAXIMUM STORAGE USED (ha.m.) |
|-----------------------------------|-------------------------------|------------------------------|
| 31.96 | 106.00 | 0.1695 |

**** SIMULATION:Run 13 - 25 mm ****

RESERVOIR(0401)
 IN= 2--> OUT= 1
 DT= 1.0 min

OVERFLOW IS OFF

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.2450 | 0.1640 |
| 0.1280 | 0.0180 | 0.2580 | 0.1870 |
| 0.1500 | 0.0370 | 0.2700 | 0.2110 |
| 0.1700 | 0.0570 | 0.2810 | 0.2360 |

| | | | |
|--------|--------|--------|--------|
| 0.1870 | 0.0770 | 0.2920 | 0.2610 |
| 0.2030 | 0.0980 | 0.3030 | 0.2870 |
| 0.2180 | 0.1190 | 0.3130 | 0.3130 |
| 0.2320 | 0.1410 | 0.3210 | 0.3350 |

| | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|------------------------|--------------|----------------|----------------|--------------|
| INFLOW : ID= 2 (0901) | 9.980 | 0.088 | 1.50 | 4.30 |
| OUTFLOW: ID= 1 (0401) | 9.980 | 0.041 | 1.73 | 4.30 |

PEAK FLOW REDUCTION [Qout/Qin](%)= 46.65
 TIME SHIFT OF PEAK FLOW (min)= 14.00
 MAXIMUM STORAGE USED (ha.m.)= 0.0057

Proposed Conditions with Retrofit Hydrology

| | | | | |
|---|---------------|----------------------------|--------------|------------|
|  | Project: | Melville Court Development | Designed By: | LB |
| | File: | 420427 | Checked By: | NHF |
| | Municipality: | Township of Oro-Medonte | Date: | 12-23-2021 |

Retrofitted SWM Facility - Pond Volume Table

| | |
|--------------------|----------|
| Active Pool | Dry Pond |
| Bottom Elev. | Varies |
| Side Slopes | Varies |

| Water Level Elev. (m) | Depth (m) | Areas | | Storage | |
|--------------------------|--------------|---------------------------|--------------------------------|---------------------------|----------------------------------|
| | | Area (m ²) | Avg. Area (m ²) | Live (m ³) | Accum. Live (m ³) |
| 287.50 | 0.00 | 1793.90 | 0.00 | 0.0 | 0.0 |
| 287.60 | 0.10 | 1857.08 | 1825.49 | 182.5 | 182.5 |
| 287.70 | 0.20 | 1920.26 | 1888.67 | 188.9 | 371.4 |
| 287.80 | 0.30 | 1983.45 | 1951.86 | 195.2 | 566.6 |
| 287.90 | 0.40 | 2046.63 | 2015.04 | 201.5 | 768.1 |
| 288.00 | 0.50 | 2109.81 | 2078.22 | 207.8 | 975.9 |
| 288.10 | 0.60 | 2172.99 | 2141.40 | 214.1 | 1190.1 |
| 288.20 | 0.70 | 2236.18 | 2204.59 | 220.5 | 1410.5 |
| 288.30 | 0.80 | 2299.36 | 2267.77 | 226.8 | 1637.3 |
| 288.40 | 0.90 | 2362.54 | 2330.95 | 233.1 | 1870.4 |
| 288.50 | 1.00 | 2425.72 | 2394.13 | 239.4 | 2109.8 |
| 288.60 | 1.10 | 2488.91 | 2457.32 | 245.7 | 2355.5 |
| 288.70 | 1.20 | 2552.09 | 2520.50 | 252.0 | 2607.6 |
| 288.80 | 1.30 | 2615.27 | 2583.68 | 258.4 | 2866.0 |
| 288.98 | 1.48 | 2729.00 | 2672.14 | 481.0 | 3346.9 |

Note: Bold areas measured using Civil 3D (based on ex. Conditions (2015) topo survey contours)

Note: Pond bottom slopes gradually toward outlet from 287.50 to 287.19. Bottom elevation of 287.50 set for volume calculation purposes based on topographic survey information.



| | | | |
|---------------|----------------------------|--------------|------------|
| Project: | Melville Court Development | Designed By: | LB |
| File: | 420427 | Checked By: | NHF |
| Municipality: | Township of Oro-Medonte | Date: | 12-23-2021 |

Retrofitted SWM Facility - Pond Discharge Table

ORIFICE CONTROL

Orifice Plate #1

diameter = 242 mm
 area = 0.0460 m²
 Orifice C = 0.63
 Invert = 287.19 m

Orifice Equation

$$Q = C \times A \times (2gH)^{0.5}$$

WEIR CONTROL

Emergency Spillway

Length of Weir 3.1 m
 Weir Sill Elevation 288.68 m
 Weir constant K Varies
 Side Slope (H:V) 3

Weir Control Discharge Rates Calculated as per Weir Control Design Sheet

where

Q = flow rate (cms) A = area of opening(sq. m)
 C = constant H = net head on the orifice
 g = Acceleration due to gravity

| Water Level (m) | Orifice Plate #1 | | Emergency Spillway | | Hydraulic Control | Total Discharge (cms) |
|--------------------|------------------|--------------------------------|--------------------|--------------------------------|-------------------|--------------------------|
| | Head (m) | Discharge (m ³) | Head (m) | Discharge (m ³) | | |
| 287.60 | 0.29 | 0.0690 | 0.00 | | Orifice Plate #1 | 0.0690 |
| 287.70 | 0.39 | 0.0801 | 0.00 | | Orifice Plate #1 | 0.0801 |
| 287.80 | 0.49 | 0.0898 | 0.00 | | Orifice Plate #1 | 0.0898 |
| 287.90 | 0.59 | 0.0985 | 0.00 | | Orifice Plate #1 | 0.0985 |
| 288.00 | 0.69 | 0.1065 | 0.00 | | Orifice Plate #1 | 0.1065 |
| 288.10 | 0.79 | 0.1140 | 0.00 | | Orifice Plate #1 | 0.1140 |
| 288.20 | 0.89 | 0.1210 | 0.00 | | Orifice Plate #1 | 0.1210 |
| 288.30 | 0.99 | 0.1276 | 0.00 | | Orifice Plate #1 | 0.1276 |
| 288.40 | 1.09 | 0.1339 | 0.00 | | Orifice Plate #1 | 0.1339 |
| 288.50 | 1.19 | 0.1400 | 0.00 | | Orifice Plate #1 | 0.1400 |
| 288.60 | 1.29 | 0.1457 | 0.00 | | Orifice Plate #1 | 0.1457 |
| 288.70 | 1.39 | 0.1513 | 0.02 | | Orifice Plate #1 | 0.1513 |
| 288.80 | 1.49 | 0.1566 | 0.12 | | Orifice Plate #1 | 0.1566 |
| 288.98 | 1.67 | 0.1658 | 0.30 | | Orifice Plate #1 | 0.1658 |

Note: Invert of outlet pipe (287.19) is below base of pond as per topographic survey.

Note: Shaded cells have been calculated using partially full inlet control equations (Hydraulic Structures, C.D.Smith, University of Saskatchewan)

Note: GREEN shaded cells have been calculated using Trapezoidal & Rectangular Weir equations (Hydraulic Structures, C.D.Smith, University of Saskatchewan)

| | | | |
|---------------|----------------------------|--------------|------------|
| Project: | Melville Court Development | Designed By: | LB |
| File: | 420427 | Checked By: | NHF |
| Municipality: | Township of Oro-Medonte | Date: | 12-23-2021 |

SWM Facility #2 - Stage-Storage-Discharge Table

| Water Level (m) | Orifice Plate Discharge (m ³ /s) | Orifice Plate Discharge (m ³ /s) | Emergency Discharge (m ³ /s) | Hydraulic Control | Total Discharge (m ³ /s) | Total Storage (ha-m) |
|--------------------|---|---|---|----------------------|---|-------------------------|
| 287.50 | 0.0558 | 0.0000 | | Orifice Plate #1 | 0.000 | 0.000 |
| 287.60 | 0.0690 | 0.0000 | | Orifice Plate #1 | 0.069 | 0.018 |
| 287.70 | 0.0801 | 0.0000 | | Orifice Plate #1 | 0.080 | 0.037 |
| 287.80 | 0.0898 | 0.0000 | | Orifice Plate #1 | 0.090 | 0.057 |
| 287.90 | 0.0985 | 0.0000 | | Orifice Plate #1 | 0.099 | 0.077 |
| 288.00 | 0.1065 | 0.0000 | | Orifice Plate #1 | 0.107 | 0.098 |
| 288.10 | 0.1140 | 0.0000 | | Orifice Plate #1 | 0.114 | 0.119 |
| 288.20 | 0.1210 | 0.0000 | | Orifice Plate #1 | 0.121 | 0.141 |
| 288.30 | 0.1276 | 0.0000 | | Orifice Plate #1 | 0.128 | 0.164 |
| 288.40 | 0.1339 | 0.0000 | | Orifice Plate #1 | 0.134 | 0.187 |
| 288.50 | 0.1400 | 0.0000 | | Orifice Plate #1 | 0.140 | 0.211 |
| 288.60 | 0.1457 | 0.0000 | | Orifice Plate #1 | 0.146 | 0.236 |
| 288.70 | 0.1513 | 0.0000 | | Orifice Plate #1 | 0.151 | 0.261 |
| 288.80 | 0.1566 | 0.0000 | | Orifice Plate #1 | 0.157 | 0.287 |
| 288.98 | 0.1658 | 0.0000 | | Orifice Plate #1 | 0.166 | 0.335 |

Retrofitted SWM Facility - Orifice Plate #1

$$\text{Orifice Plate} = C \times A \times (2gH)^{0.5}$$

$$C = 0.63$$

| Depth Increment m | (invert of Control) | (Top of Pond) |
|-------------------------|-----------------------|-------------------------|
| | Min Elevation m | Max Elevation Max |
| 0.1 | 287.19 | 289.10 |

| Water Elevation m | Depth from Min. Elevation m | Pipe Partially Full Inlet Control Flow (Smith) m ³ /s |
|-------------------------|--------------------------------------|--|
| 287.19 | 0.00 | |
| 287.20 | 0.10 | 0.001 |
| 287.30 | 0.20 | 0.008 |
| 287.40 | 0.30 | 0.026 |
| 287.50 | 0.40 | 0.056 |
| 287.60 | 0.50 | 0.069 |
| 287.70 | 0.60 | 0.080 |
| 287.80 | 0.70 | 0.090 |
| 287.90 | 0.80 | 0.099 |
| 288.00 | 0.90 | 0.107 |
| 288.10 | 1.00 | 0.114 |
| 288.20 | 1.10 | 0.121 |
| 288.30 | 1.20 | 0.128 |

Pipe Flowing Partially Full or full

| | |
|---------------------------------------|--------|
| Coefficient Metric = 1, Imperial=1.49 | 1 |
| Pipe Diameter (m) | 0.242 |
| Mannings N Full | 0.013 |
| Pipe Slope (m/m) | |
| Elevation (m) | 287.19 |
| Inlet Type: 1= Projecting 2= Flush | 1 |

Inlet Control (Smith) Projecting Inlet

$$\text{MMF Model: } y = (a \cdot b + c \cdot x^d) / (b + x^d)$$

Coefficient Data:

| | |
|-----|----------|
| a = | 1.68E-02 |
| b = | 1.77E+00 |
| c = | 3.06E+00 |
| d = | 2.27E+00 |

Inlet Control (Smith) Flush Inlet

$$\text{MMF Model: } y = (a \cdot b + c \cdot x^d) / (b + x^d)$$

Coefficient Data:

| | |
|-----|-----------|
| a = | -1.08E-03 |
| b = | 2.30E+00 |
| c = | 3.95E+00 |
| d = | 2.08E+00 |

| | | | |
|---------|--|------|------------|
| PROJECT | Melville Court Development | FILE | 420427 |
| | | DATE | 12-23-2021 |
| SUBJECT | Preliminary Overflow Weir Calculations | NAME | LB |
| | | PAGE | 1 OF 1 |

Trapezoidal Broad Crested Weir

Source: Hydraulic Structures, C.D.Smith, University of Saskatchewan

Trapezoidal Weir

The trapezoidal weir is a combination of the rectangular weir and the triangular weir

Enter

| | | |
|---|---------------------------|---|
| W | Weir Bottom Width (m) | 3 |
| H | Head (m) | 0 |
| L | Weir Downstream Length | 5 |
| S | Side Slope (horizontal):1 | 5 |

Rectangular Weir

$$Q = CWH^{3/2}$$

| | |
|-----|--------|
| H/L | 0.057 |
| C | 1.5281 |

Result

Q Rectangular Weir Flow (m³/s) **0.7784**

Triangular Weir

$$Q = CH^{5/2} \tan(\theta/2)$$

| | | |
|------------------------|--------|---------|
| Notch Angle (one side) | 78.69 | degrees |
| Notch Angle (one side) | 1.3734 | radians |

$$\tan(\theta / 2) = 5.00$$

| | |
|----------------|--------|
| Triangular H/L | 0.057 |
| C | 1.0898 |

Result

Q Triangular Weir Flow (m³/s) **0.2686**

Total Rectangular + Triangular Weir

Q Total Flow (m³/s) 1.047


```

* CALIB STANDHYD      0105  1 10.0   1.70   0.13  1.33  17.28  0.39   0.000
  [I%=25.0:S%= 2.00]
*
* CHIC STORM          10.0
  [ Ptot= 43.85 mm ]
*
* CALIB STANDHYD      0100  1 10.0   0.90   0.08  1.33  17.75  0.40   0.000
  [I%=30.0:S%= 2.00]
*
* ADD [ 0100+ 1001]  0901  3 10.0   1.25   0.09  1.33  16.24  n/a   0.000
*
* ADD [ 0901+ 0105]  0901  1 10.0   2.95   0.22  1.33  16.84  n/a   0.000
*
* ADD [ 0901+ 0106]  0901  3  1.0   3.53   0.24  1.33  16.11  n/a   0.000
*
* ADD [ 0901+ 0202]  0901  1  1.0   9.98   0.25  1.33  11.55  n/a   0.000
*
** Reservoir
OUTFLOW:              0401  1  1.0   9.98   0.08  3.12  11.54  n/a   0.000
OVERFLOW:             0401  3  1.0   0.00   0.00  0.00   0.00  n/a   0.000
*
* ADD [ 0201+ 0401]  0902  3  1.0   66.22  0.39  3.03   7.28  n/a   0.000

```

```

** CALIB NASHYD      0106  1  1.0   0.58   0.04  1.40  16.30  0.32   0.000
  [CN=74.0
  [ N = 3.0:Tp 0.11]
*
* CHIC STORM          10.0
  [ Ptot= 50.98 mm ]
*
* CALIB STANDHYD      1001  1 10.0   0.35   0.02  1.33  15.79  0.31   0.000
  [I%=10.0:S%= 2.00]
*
* CHIC STORM          10.0
  [ Ptot= 50.98 mm ]
*
* CALIB STANDHYD      0105  1 10.0   1.70   0.15  1.33  21.29  0.42   0.000
  [I%=25.0:S%= 2.00]
*
* CHIC STORM          10.0
  [ Ptot= 50.98 mm ]
*
* CALIB STANDHYD      0100  1 10.0   0.90   0.09  1.33  21.63  0.42   0.000
  [I%=30.0:S%= 2.00]
*
* ADD [ 0100+ 1001]  0901  3 10.0   1.25   0.11  1.33  20.00  n/a   0.000
*
* ADD [ 0901+ 0105]  0901  1 10.0   2.95   0.26  1.33  20.74  n/a   0.000
*
* ADD [ 0901+ 0106]  0901  3  1.0   3.53   0.29  1.33  20.01  n/a   0.000
*
* ADD [ 0901+ 0202]  0901  1  1.0   9.98   0.30  1.33  14.94  n/a   0.000
*
** Reservoir
OUTFLOW:              0401  1  1.0   9.98   0.09  3.50  14.94  n/a   0.000
OVERFLOW:             0401  3  1.0   0.00   0.00  0.00   0.00  n/a   0.000
*
* ADD [ 0201+ 0401]  0902  3  1.0   66.22  0.52  3.03   9.93  n/a   0.000

```

```

V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL

OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
OOO T T H H Y M M OOO

```

```

V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL

OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
OOO T T H H Y M M OOO

```

Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2021 Smart City Water Inc
 All rights reserved.

Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2021 Smart City Water Inc
 All rights reserved.

***** SUMMARY OUTPUT *****

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
 Output filename: C:\Users\LBuss\AppData\Local\Civica\XH5\18b1f826-8929-408e-9936-e2d934269cf1\7db6a67-77b2
 Summary filename: C:\Users\LBuss\AppData\Local\Civica\XH5\18b1f826-8929-408e-9936-e2d934269cf1\7db6a67-77b2

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
 Output filename: C:\Users\LBuss\AppData\Local\Civica\XH5\18b1f826-8929-408e-9936-e2d934269cf1\50fe444-ba17
 Summary filename: C:\Users\LBuss\AppData\Local\Civica\XH5\18b1f826-8929-408e-9936-e2d934269cf1\50fe444-ba17

DATE: 01-19-2022 TIME: 08:05:00

DATE: 01-19-2022 TIME: 08:05:00

USER:

USER:

COMMENTS: _____

COMMENTS: _____

 ** SIMULATION : Run 03 - 10 Year 4 Hour Chica **

 ** SIMULATION : Run 04 - 25 Hour 4 Hour Chica **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|---|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| CHIC STORM [Ptot= 50.98 mm] | | 10.0 | | | | | | |
| ** CALIB NASHYD [CN=61.2 [N = 3.0:Tp 1.16] | 0201 | 1 14.0 | 56.24 | 0.44 | 3.03 | 9.04 | 0.18 | 0.000 |
| CHIC STORM [Ptot= 50.98 mm] | | 10.0 | | | | | | |
| ** CALIB NASHYD [CN=66.6 [N = 3.0:Tp 0.91] | 0202 | 1 10.0 | 6.45 | 0.08 | 2.50 | 12.17 | 0.24 | 0.000 |
| CHIC STORM [Ptot= 50.98 mm] | | 10.0 | | | | | | |

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|----------------------------------|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| CHIC STORM [Ptot= 59.78 mm] | | 10.0 | | | | | | |
| ** CALIB NASHYD [CN=61.2 | 0201 | 1 14.0 | 56.24 | 0.61 | 2.80 | 12.57 | 0.21 | 0.000 |


```

* [ N = 3.0:Tp 1.16]
* CHIC STORM 10.0
* [ Ptot= 59.78 mm ]
** CALIB NASHYD 0202 1 10.0 6.45 0.11 2.50 16.45 0.28 0.000
* [CN=66.6 ]
* [ N = 3.0:Tp 0.91]
* CHIC STORM 10.0
* [ Ptot= 59.78 mm ]
** CALIB NASHYD 0106 1 1.0 0.58 0.05 1.40 21.57 0.36 0.000
* [CN=74.0 ]
* [ N = 3.0:Tp 0.11]
* CHIC STORM 10.0
* [ Ptot= 59.78 mm ]
** CALIB STANDHYD 1001 1 10.0 0.35 0.02 1.33 20.41 0.34 0.000
* [I%=10.0:S%= 2.00]
* CHIC STORM 10.0
* [ Ptot= 59.78 mm ]
** CALIB STANDHYD 0105 1 10.0 1.70 0.19 1.33 26.54 0.44 0.000
* [I%=25.0:S%= 2.00]
* CHIC STORM 10.0
* [ Ptot= 59.78 mm ]
** CALIB STANDHYD 0100 1 10.0 0.90 0.11 1.33 26.71 0.45 0.000
* [I%=30.0:S%= 2.00]
* ADD [ 0100+ 1001] 0901 3 10.0 1.25 0.13 1.33 24.94 n/a 0.000
* ADD [ 0901+ 0105] 0901 1 10.0 2.95 0.32 1.33 25.86 n/a 0.000
* ADD [ 0901+ 0106] 0901 3 1.0 3.53 0.36 1.33 25.15 n/a 0.000
* ADD [ 0901+ 0202] 0901 1 1.0 9.98 0.37 1.33 19.53 n/a 0.000
** Reservoir
OUTFLOW: 0401 1 1.0 9.98 0.10 3.85 19.52 n/a 0.000
OVERFLOW: 0401 3 1.0 0.00 0.00 0.00 0.00 n/a 0.000
* ADD [ 0201+ 0401] 0902 3 1.0 66.22 0.71 3.03 13.62 n/a 0.000

```

```

=====
V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL
000 TTTTT TTTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
000 T T H H Y M M 000

```

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2021 Smart City Water Inc
All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\3b97ad95-e539
Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\3b97ad95-e539

DATE: 01-19-2022 TIME: 08:05:00

USER:

COMMENTS: _____

** SIMULATION : Run 05 - 50 Year 4 Hour Chica **

```

*****
W/E COMMAND HYD ID DT AREA Qpeak Tpeak R.V. R.C. Qbase
min ha cms hrs mm cms
-----
START @ 0.00 hrs
CHIC STORM 10.0
[ Ptot= 66.46 mm ]
** CALIB NASHYD 0201 1 14.0 56.24 0.76 2.80 15.53 0.23 0.000
* [CN=61.2 ]
* [ N = 3.0:Tp 1.16]
* CHIC STORM 10.0
* [ Ptot= 66.46 mm ]
** CALIB NASHYD 0202 1 10.0 6.45 0.13 2.50 19.97 0.30 0.000
* [CN=66.6 ]
* [ N = 3.0:Tp 0.91]
* CHIC STORM 10.0
* [ Ptot= 66.46 mm ]
** CALIB NASHYD 0106 1 1.0 0.58 0.06 1.40 25.83 0.39 0.000
* [CN=74.0 ]
* [ N = 3.0:Tp 0.11]
* CHIC STORM 10.0
* [ Ptot= 66.46 mm ]
** CALIB STANDHYD 1001 1 10.0 0.35 0.02 1.33 24.16 0.36 0.000
* [I%=10.0:S%= 2.00]
* CHIC STORM 10.0
* [ Ptot= 66.46 mm ]
** CALIB STANDHYD 0105 1 10.0 1.70 0.21 1.33 30.73 0.46 0.000
* [I%=25.0:S%= 2.00]
* CHIC STORM 10.0
* [ Ptot= 66.46 mm ]
** CALIB STANDHYD 0100 1 10.0 0.90 0.13 1.33 30.74 0.46 0.000
* [I%=30.0:S%= 2.00]
* ADD [ 0100+ 1001] 0901 3 10.0 1.25 0.15 1.33 28.90 n/a 0.000
* ADD [ 0901+ 0105] 0901 1 10.0 2.95 0.36 1.33 29.95 n/a 0.000
* ADD [ 0901+ 0106] 0901 3 1.0 3.53 0.41 1.33 29.27 n/a 0.000
* ADD [ 0901+ 0202] 0901 1 1.0 9.98 0.43 1.33 23.26 n/a 0.000
** Reservoir
OUTFLOW: 0401 1 1.0 9.98 0.11 4.02 23.25 n/a 0.000
OVERFLOW: 0401 3 1.0 0.00 0.00 0.00 0.00 n/a 0.000
* ADD [ 0201+ 0401] 0902 3 1.0 66.22 0.87 2.80 16.70 n/a 0.000

```

```

=====
V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL
000 TTTTT TTTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
000 T T H H Y M M 000

```

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2021 Smart City Water Inc
All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\155055ee-a743
Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\155055ee-a743

DATE: 01-19-2022 TIME: 08:05:00
 USER:

COMMENTS: _____

 ** SIMULATION : Run 06 - 100 Year 4 Hour Chic **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|---|--------------|----------------|--------------|--------------|--------------|---------------|------------|----------------|
| START @ 0.00 hrs | | | | | | | | |
| CHIC STORM [Ptot= 72.98 mm] | | 10.0 | | | | | | |
| ** CALIB NASHYD [CN=61.2 [N = 3.0:Tp 1.16] | 0201 | 1 14.0 | 56.24 | 0.92 | 2.80 | 18.64 | 0.26 | 0.000 |
| CHIC STORM [Ptot= 72.98 mm] | | 10.0 | | | | | | |
| ** CALIB NASHYD [CN=66.6 [N = 3.0:Tp 0.91] | 0202 | 1 10.0 | 6.45 | 0.16 | 2.50 | 23.62 | 0.32 | 0.000 |
| CHIC STORM [Ptot= 72.98 mm] | | 10.0 | | | | | | |
| ** CALIB NASHYD [CN=74.0 [N = 3.0:Tp 0.11] | 0106 | 1 1.0 | 0.58 | 0.08 | 1.40 | 30.19 | 0.41 | 0.000 |
| CHIC STORM [Ptot= 72.98 mm] | | 10.0 | | | | | | |
| ** CALIB STANDHYD [I%=10.0:S%= 2.00] | 1001 | 1 10.0 | 0.35 | 0.03 | 1.33 | 28.01 | 0.38 | 0.000 |
| CHIC STORM [Ptot= 72.98 mm] | | 10.0 | | | | | | |
| ** CALIB STANDHYD [I%=25.0:S%= 2.00] | 0105 | 1 10.0 | 1.70 | 0.24 | 1.33 | 34.97 | 0.48 | 0.000 |
| CHIC STORM [Ptot= 72.98 mm] | | 10.0 | | | | | | |
| ** CALIB STANDHYD [I%=30.0:S%= 2.00] | 0100 | 1 10.0 | 0.90 | 0.14 | 1.33 | 34.82 | 0.48 | 0.000 |
| ADD [0100+ 1001] | 0901 | 3 10.0 | 1.25 | 0.17 | 1.33 | 32.91 | n/a | 0.000 |
| ADD [0901+ 0105] | 0901 | 1 10.0 | 2.95 | 0.41 | 1.33 | 34.10 | n/a | 0.000 |
| ADD [0901+ 0106] | 0901 | 3 1.0 | 3.53 | 0.47 | 1.33 | 33.45 | n/a | 0.000 |
| ADD [0901+ 0202] | 0901 | 1 1.0 | 9.98 | 0.49 | 1.33 | 27.10 | n/a | 0.000 |
| ** Reservoir OUTFLOW: OVERFLOW: | 0401 0401 | 1 1.0 3 1.0 | 9.98 0.00 | 0.12 0.00 | 4.08 0.00 | 27.09 0.00 | n/a n/a | 0.000 0.000 |
| ADD [0201+ 0401] | 0902 | 3 1.0 | 66.22 | 1.03 | 2.80 | 19.91 | n/a | 0.000 |

V V I SSSSS U U A L (v 6.2.2008)
 V V I SS U U A A L
 V V I SS U U AAAAA L
 V V I SS U U A A L
 V V I SSSSS UUUU A A LLLLL
 OOO TTTT TTTT H H Y Y M M OOO TM
 O O T T H H Y Y M M O O
 O O T T H H Y Y M M O O

000 T T H H Y M M 000
 Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2021 Smart City Water Inc
 All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
 Output filename: C:\Users\LBuss\AppData\Local\Civica\XH5\18b1f826-8929-408e-9936-e2d934269cf1\2691caed-a310
 Summary filename: C:\Users\LBuss\AppData\Local\Civica\XH5\18b1f826-8929-408e-9936-e2d934269cf1\2691caed-a310

DATE: 01-19-2022 TIME: 08:05:00
 USER:

COMMENTS: _____

 ** SIMULATION : Run 07 - 2 Year 24 Hour SCS **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|---|--------------|----------------|--------------|--------------|---------------|---------------|------------|----------------|
| START @ 0.00 hrs | | | | | | | | |
| MASS STORM [Ptot= 56.53 mm] | | 15.0 | | | | | | |
| ** CALIB NASHYD [CN=61.2 [N = 3.0:Tp 1.16] | 0201 | 1 14.0 | 56.24 | 0.42 | 12.83 | 11.27 | 0.20 | 0.000 |
| MASS STORM [Ptot= 56.53 mm] | | 15.0 | | | | | | |
| ** CALIB NASHYD [CN=66.6 [N = 3.0:Tp 0.91] | 0202 | 1 10.0 | 6.45 | 0.08 | 12.67 | 14.82 | 0.26 | 0.000 |
| MASS STORM [Ptot= 56.53 mm] | | 15.0 | | | | | | |
| ** CALIB NASHYD [CN=74.0 [N = 3.0:Tp 0.11] | 0106 | 1 1.0 | 0.58 | 0.04 | 11.78 | 16.03 | 0.28 | 0.000 |
| MASS STORM [Ptot= 56.53 mm] | | 15.0 | | | | | | |
| ** CALIB STANDHYD [I%=10.0:S%= 2.00] | 1001 | 1 10.0 | 0.35 | 0.01 | 11.83 | 18.65 | 0.33 | 0.000 |
| MASS STORM [Ptot= 56.53 mm] | | 15.0 | | | | | | |
| ** CALIB STANDHYD [I%=25.0:S%= 2.00] | 0105 | 1 10.0 | 1.70 | 0.10 | 11.67 | 24.56 | 0.43 | 0.000 |
| MASS STORM [Ptot= 56.53 mm] | | 15.0 | | | | | | |
| ** CALIB STANDHYD [I%=30.0:S%= 2.00] | 0100 | 1 10.0 | 0.90 | 0.06 | 11.67 | 24.80 | 0.44 | 0.000 |
| ADD [0100+ 1001] | 0901 | 3 10.0 | 1.25 | 0.07 | 11.67 | 23.08 | n/a | 0.000 |
| ADD [0901+ 0105] | 0901 | 1 10.0 | 2.95 | 0.17 | 11.67 | 23.93 | n/a | 0.000 |
| ADD [0901+ 0106] | 0901 | 3 1.0 | 3.53 | 0.19 | 11.67 | 23.21 | n/a | 0.000 |
| ADD [0901+ 0202] | 0901 | 1 1.0 | 9.98 | 0.21 | 11.77 | 17.79 | n/a | 0.000 |
| ** Reservoir OUTFLOW: OVERFLOW: | 0401 0401 | 1 1.0 3 1.0 | 9.98 0.00 | 0.08 0.00 | 13.25 0.00 | 17.78 0.00 | n/a n/a | 0.000 0.000 |
| ADD [0201+ 0401] | 0902 | 3 1.0 | 66.22 | 0.50 | 12.83 | 12.26 | n/a | 0.000 |


```

* CALIB STANDHYD      0105 1 10.0   1.70  0.17 11.67  44.71 0.51  0.000
  [I%=25.0:S%= 2.00]
*
* MASS STORM
  [ Ptot= 87.24 mm ]      15.0
*
* CALIB STANDHYD      0100 1 10.0   0.90  0.10 11.67  44.16 0.51  0.000
  [I%=30.0:S%= 2.00]
*
* ADD [ 0100+ 1001]    0901 3 10.0   1.25  0.12 11.67  42.14 n/a  0.000
*
* ADD [ 0901+ 0105]    0901 1 10.0   2.95  0.29 11.67  43.62 n/a  0.000
*
* ADD [ 0901+ 0106]    0901 3 1.0    3.53  0.36 11.77  43.07 n/a  0.000
*
* ADD [ 0901+ 0202]    0901 1 1.0    9.98  0.42 11.78  36.07 n/a  0.000
*
** Reservoir
OUTFLOW:              0401 1 1.0    9.98  0.11 13.87  36.06 n/a  0.000
OVERFLOW:             0401 3 1.0    0.00  0.00 0.00    0.00 n/a  0.000
*
* ADD [ 0201+ 0401]    0902 3 1.0   66.22  1.13 12.83  27.67 n/a  0.000

```

```

[CN=74.0
 [ N = 3.0:Tp 0.11]
*
* MASS STORM
  [ Ptot=102.29 mm ]      15.0
*
* CALIB STANDHYD      1001 1 10.0   0.35  0.04 11.67  47.09 0.46  0.000
  [I%=10.0:S%= 2.00]
*
* MASS STORM
  [ Ptot=102.29 mm ]      15.0
*
* CALIB STANDHYD      0105 1 10.0   1.70  0.21 11.67  55.57 0.54  0.000
  [I%=25.0:S%= 2.00]
*
* MASS STORM
  [ Ptot=102.29 mm ]      15.0
*
* CALIB STANDHYD      0100 1 10.0   0.90  0.12 11.67  54.58 0.53  0.000
  [I%=30.0:S%= 2.00]
*
* ADD [ 0100+ 1001]    0901 3 10.0   1.25  0.16 11.67  52.48 n/a  0.000
*
* ADD [ 0901+ 0105]    0901 1 10.0   2.95  0.37 11.67  54.26 n/a  0.000
*
* ADD [ 0901+ 0106]    0901 3 1.0   3.53  0.46 11.77  53.83 n/a  0.000
*
* ADD [ 0901+ 0202]    0901 1 1.0   9.98  0.55 11.78  46.25 n/a  0.000
*
** Reservoir
OUTFLOW:              0401 1 1.0    9.98  0.13 14.05  46.24 n/a  0.000
OVERFLOW:             0401 3 1.0    0.00  0.00 0.00    0.00 n/a  0.000
*
* ADD [ 0201+ 0401]    0902 3 1.0   66.22  1.49 12.83  36.59 n/a  0.000

```

```

=====
V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
VV I SSSSS UUUU A A LLLLL
OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
OOO T T H H Y Y M M OOO

```

```

=====
V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
VV I SSSSS UUUU A A LLLLL
OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
OOO T T H H Y Y M M OOO

```

Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2021 Smart City Water Inc
 All rights reserved.

Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2021 Smart City Water Inc
 All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
 Output filename: C:\Users\LBuss\AppData\Local\Civica\5\18b1f826-8929-408e-9936-e2d934269cf1\5f8983f8-3f6b
 Summary filename: C:\Users\LBuss\AppData\Local\Civica\5\18b1f826-8929-408e-9936-e2d934269cf1\5f8983f8-3f6b

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
 Output filename: C:\Users\LBuss\AppData\Local\Civica\5\18b1f826-8929-408e-9936-e2d934269cf1\ee8abf5a-6e9a
 Summary filename: C:\Users\LBuss\AppData\Local\Civica\5\18b1f826-8929-408e-9936-e2d934269cf1\ee8abf5a-6e9a

DATE: 01-19-2022 TIME: 08:05:00

DATE: 01-19-2022 TIME: 08:05:00

USER:

USER:

COMMENTS: _____

COMMENTS: _____

 ** SIMULATION : Run 10 - 25 Year 24 Hour SCS **

 ** SIMULATION : Run 11 - 50 year 24 Hour SCS **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|---|--------|--------|---------|------------|------------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| MASS STORM [Ptot=102.29 mm] | | | | | | | | |
| ** CALIB NASHYD [CN=61.2 [N = 3.0:Tp 1.16] | 0201 | 1 14.0 | 56.24 | 1.37 12.83 | 34.87 0.34 | 0.000 | | |
| MASS STORM [Ptot=102.29 mm] | | | | | | | | |
| ** CALIB NASHYD [CN=66.6 [N = 3.0:Tp 0.91] | 0202 | 1 10.0 | 6.45 | 0.23 12.67 | 42.09 0.41 | 0.000 | | |
| MASS STORM [Ptot=102.29 mm] | | | | | | | | |
| ** CALIB NASHYD | 0106 | 1 1.0 | 0.58 | 0.10 11.78 | 43.36 0.42 | 0.000 | | |

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|---|--------|--------|---------|------------|------------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| MASS STORM [Ptot=113.66 mm] | | | | | | | | |
| ** CALIB NASHYD [CN=61.2 [N = 3.0:Tp 1.16] | 0201 | 1 14.0 | 56.24 | 1.66 12.83 | 41.91 0.37 | 0.000 | | |

```

*
* MASS STORM 15.0
* [ Ptot=113.66 mm ]
** CALIB NASHYD 0202 1 10.0 6.45 0.27 12.67 49.98 0.44 0.000
* [CN=66.6
* [ N = 3.0:Tp 0.91]
*
* MASS STORM 15.0
* [ Ptot=113.66 mm ]
** CALIB NASHYD 0106 1 1.0 0.58 0.12 11.78 51.06 0.45 0.000
* [CN=74.0
* [ N = 3.0:Tp 0.11]
*
* MASS STORM 15.0
* [ Ptot=113.66 mm ]
** CALIB STANDHYD 1001 1 10.0 0.35 0.05 11.67 55.14 0.49 0.000
* [I%=10.0:S%= 2.00]
*
* MASS STORM 15.0
* [ Ptot=113.66 mm ]
** CALIB STANDHYD 0105 1 10.0 1.70 0.24 11.83 64.11 0.56 0.000
* [I%=25.0:S%= 2.00]
*
* MASS STORM 15.0
* [ Ptot=113.66 mm ]
** CALIB STANDHYD 0100 1 10.0 0.90 0.14 11.67 62.77 0.55 0.000
* [I%=30.0:S%= 2.00]
*
* ADD [ 0100+ 1001] 0901 3 10.0 1.25 0.19 11.67 60.64 n/a 0.000
*
* ADD [ 0901+ 0105] 0901 1 10.0 2.95 0.43 11.67 62.64 n/a 0.000
*
* ADD [ 0901+ 0106] 0901 3 1.0 3.53 0.54 11.77 62.30 n/a 0.000
*
* ADD [ 0901+ 0202] 0901 1 1.0 9.98 0.64 11.80 54.34 n/a 0.000
** Reservoir
* OUTFLOW: 0401 1 1.0 9.98 0.14 14.15 54.33 n/a 0.000
* OVERFLOW: 0401 3 1.0 0.00 0.00 0.00 0.00 n/a 0.000
*
* ADD [ 0201+ 0401] 0902 3 1.0 66.22 1.79 12.83 43.78 n/a 0.000

```

```

=====
V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL
O O TTTTT TTTT H H Y Y M M O O TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
O O T T H H Y M M O O

```

Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2021 Smart City Water Inc
 All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
 Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\783637b2-51ff
 Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\783637b2-51ff

DATE: 01-19-2022 TIME: 08:05:00

USER:

COMMENTS: _____

 ** SIMULATION : Run 12 - 100 Year 24 Hour SCS **

```

W/E COMMAND HYD ID DT AREA Qpeak Tpeak R.V. R.C. Qbase
min cms hrs mm cms
-----
START @ 0.00 hrs
MASS STORM 15.0
* [ Ptot=124.82 mm ]
** CALIB NASHYD 0201 1 14.0 56.24 1.96 12.83 49.17 0.39 0.000
* [CN=61.2
* [ N = 3.0:Tp 1.16]
*
* MASS STORM 15.0
* [ Ptot=124.82 mm ]
** CALIB NASHYD 0202 1 10.0 6.45 0.32 12.50 58.04 0.46 0.000
* [CN=66.6
* [ N = 3.0:Tp 0.91]
*
* MASS STORM 15.0
* [ Ptot=124.82 mm ]
** CALIB NASHYD 0106 1 1.0 0.58 0.14 11.78 58.88 0.47 0.000
* [CN=74.0
* [ N = 3.0:Tp 0.11]
*
* MASS STORM 15.0
* [ Ptot=124.82 mm ]
** CALIB STANDHYD 1001 1 10.0 0.35 0.06 11.67 63.34 0.51 0.000
* [I%=10.0:S%= 2.00]
*
* MASS STORM 15.0
* [ Ptot=124.82 mm ]
** CALIB STANDHYD 0105 1 10.0 1.70 0.36 11.67 72.74 0.58 0.000
* [I%=25.0:S%= 2.00]
*
* MASS STORM 15.0
* [ Ptot=124.82 mm ]
** CALIB STANDHYD 0100 1 10.0 0.90 0.15 11.67 71.06 0.57 0.000
* [I%=30.0:S%= 2.00]
*
* ADD [ 0100+ 1001] 0901 3 10.0 1.25 0.21 11.67 68.90 n/a 0.000
*
* ADD [ 0901+ 0105] 0901 1 10.0 2.95 0.57 11.67 71.11 n/a 0.000
*
* ADD [ 0901+ 0106] 0901 3 1.0 3.53 0.67 11.67 70.87 n/a 0.000
*
* ADD [ 0901+ 0202] 0901 1 1.0 9.98 0.77 11.75 62.58 n/a 0.000
** Reservoir
* OUTFLOW: 0401 1 1.0 9.98 0.15 14.27 62.57 n/a 0.000
* OVERFLOW: 0401 3 1.0 0.00 0.00 0.00 0.00 n/a 0.000
*
* ADD [ 0201+ 0401] 0902 3 1.0 66.22 2.09 12.83 51.19 n/a 0.000

```

```

=====
V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL
O O TTTTT TTTT H H Y Y M M O O TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
O O T T H H Y M M O O

```

Developed and Distributed by Smart City Water Inc
 Copyright 2007 - 2021 Smart City Water Inc
 All rights reserved.

***** SUMMARY OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
 Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\9f1a3208-218a
 Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\9f1a3208-218a

DATE: 01-19-2022

TIME: 08:05:00

USER:

COMMENTS: _____

** SIMULATION : Run 13 - 25 mm **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|--|--------|--------|---------|-----------|-----------|---------|------|-----------|
| START @ 0.00 hrs | | | | | | | | |
| ----- | | | | | | | | |
| READ STORM | | 10.0 | | | | | | |
| [Ptot= 25.02 mm] | | | | | | | | |
| fname : C:\Users\LBuss\AppData\Local\Temp\a5ccea7c-cd06-4942-80d0-1abfc065ec5c\754080c7-048d-4c71-b842-e68 | | | | | | | | |
| remark: 25MMCHI | | | | | | | | |
| ** CALIB NASHYD | 0201 | 1 14.0 | 56.24 | 0.08 | 3.73 | 1.63 | 0.07 | 0.000 |
| [CN=61.2] | | | | | | | | |
| [N = 3.0:Tp 1.16] | | | | | | | | |
| READ STORM | | 10.0 | | | | | | |
| [Ptot= 25.02 mm] | | | | | | | | |
| fname : C:\Users\LBuss\AppData\Local\Temp\a5ccea7c-cd06-4942-80d0-1abfc065ec5c\754080c7-048d-4c71-b842-e68 | | | | | | | | |
| remark: 25MMCHI | | | | | | | | |
| ** CALIB NASHYD | 0202 | 1 10.0 | 6.45 | 0.02 | 3.00 | 2.71 | 0.11 | 0.000 |
| [CN=66.6] | | | | | | | | |
| [N = 3.0:Tp 0.91] | | | | | | | | |
| READ STORM | | 10.0 | | | | | | |
| [Ptot= 25.02 mm] | | | | | | | | |
| fname : C:\Users\LBuss\AppData\Local\Temp\a5ccea7c-cd06-4942-80d0-1abfc065ec5c\754080c7-048d-4c71-b842-e68 | | | | | | | | |
| remark: 25MMCHI | | | | | | | | |
| ** CALIB NASHYD | 0106 | 1 1.0 | 0.58 | 0.01 | 1.58 | 4.07 | 0.16 | 0.000 |
| [CN=74.0] | | | | | | | | |
| [N = 3.0:Tp 0.11] | | | | | | | | |
| READ STORM | | 10.0 | | | | | | |
| [Ptot= 25.02 mm] | | | | | | | | |
| fname : C:\Users\LBuss\AppData\Local\Temp\a5ccea7c-cd06-4942-80d0-1abfc065ec5c\754080c7-048d-4c71-b842-e68 | | | | | | | | |
| remark: 25MMCHI | | | | | | | | |
| * CALIB STANDHYD | 1001 | 1 10.0 | 0.35 | 0.00 | 1.50 | 4.89 | 0.20 | 0.000 |
| [I%=10.0:S%= 2.00] | | | | | | | | |
| READ STORM | | 10.0 | | | | | | |
| [Ptot= 25.02 mm] | | | | | | | | |
| fname : C:\Users\LBuss\AppData\Local\Temp\a5ccea7c-cd06-4942-80d0-1abfc065ec5c\754080c7-048d-4c71-b842-e68 | | | | | | | | |
| remark: 25MMCHI | | | | | | | | |
| * CALIB STANDHYD | 0105 | 1 10.0 | 1.70 | 0.05 | 1.50 | 8.04 | 0.32 | 0.000 |
| [I%=25.0:S%= 2.00] | | | | | | | | |
| READ STORM | | 10.0 | | | | | | |
| [Ptot= 25.02 mm] | | | | | | | | |
| fname : C:\Users\LBuss\AppData\Local\Temp\a5ccea7c-cd06-4942-80d0-1abfc065ec5c\754080c7-048d-4c71-b842-e68 | | | | | | | | |
| remark: 25MMCHI | | | | | | | | |
| * CALIB STANDHYD | 0100 | 1 10.0 | 0.90 | 0.03 | 1.50 | 8.61 | 0.34 | 0.000 |
| [I%=30.0:S%= 2.00] | | | | | | | | |
| ADD [0100+ 1001] | 0901 | 3 10.0 | 1.25 | 0.03 | 1.50 | 7.57 | n/a | 0.000 |
| ADD [0901+ 0105] | 0901 | 1 10.0 | 2.95 | 0.08 | 1.50 | 7.84 | n/a | 0.000 |
| ADD [0901+ 0106] | 0901 | 3 1.0 | 3.53 | 0.09 | 1.50 | 7.22 | n/a | 0.000 |
| ADD [0901+ 0202] | 0901 | 1 1.0 | 9.98 | 0.09 | 1.50 | 4.30 | n/a | 0.000 |
| ** Reservoir | | | | | | | | |
| OUTFLOW: | 0401 | 1 1.0 | 9.98 | 0.03 | 2.75 | 4.30 | n/a | 0.000 |
| OVERFLOW: | 0401 | 3 1.0 | 0.00 | 0.00 | 0.00 | 0.00 | n/a | 0.000 |
| ADD [0201+ 0401] | 0902 | 3 1.0 | 66.22 | 0.11 | 3.50 | 2.03 | n/a | 0.000 |

FINISH

 ** SIMULATION:Run 01 - 2 year 4 Hour Chicago **

```

RESERVOIR( 0401)
IN= 2---> OUT= 1
DT= 1.0 min
OVERFLOW IS OFF
  
```

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.1280 | 0.1640 |
| 0.0690 | 0.0180 | 0.1340 | 0.1870 |
| 0.0800 | 0.0370 | 0.1400 | 0.2110 |
| 0.0900 | 0.0570 | 0.1460 | 0.2360 |
| 0.0990 | 0.0770 | 0.1510 | 0.2610 |
| 0.1070 | 0.0980 | 0.1570 | 0.2870 |
| 0.1140 | 0.1190 | 0.1660 | 0.3350 |
| 0.1210 | 0.1410 | 0.0000 | 0.0000 |

| INFLOW : ID= 2 (0901) | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|------------------------|-----------|-------------|-------------|-----------|
| 9.980 | 9.980 | 0.170 | 1.33 | 7.07 |
| OUTFLOW: ID= 1 (0401) | 9.980 | 0.054 | 2.47 | 7.06 |

| PEAK FLOW | REDUCTION [Qout/Qin] (%) | TIME SHIFT OF PEAK FLOW (min) | MAXIMUM STORAGE USED (ha.m.) |
|-----------|--------------------------|-------------------------------|------------------------------|
| 31.70 | 68.00 | 0.0141 | |

 ** SIMULATION:Run 02 - 5 Year 4 Hour Chicago **

```

RESERVOIR( 0401)
IN= 2---> OUT= 1
DT= 1.0 min
OVERFLOW IS OFF
  
```

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.1280 | 0.1640 |
| 0.0690 | 0.0180 | 0.1340 | 0.1870 |
| 0.0800 | 0.0370 | 0.1400 | 0.2110 |
| 0.0900 | 0.0570 | 0.1460 | 0.2360 |
| 0.0990 | 0.0770 | 0.1510 | 0.2610 |
| 0.1070 | 0.0980 | 0.1570 | 0.2870 |
| 0.1140 | 0.1190 | 0.1660 | 0.3350 |
| 0.1210 | 0.1410 | 0.0000 | 0.0000 |

| INFLOW : ID= 2 (0901) | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|------------------------|-----------|-------------|-------------|-----------|
| 9.980 | 9.980 | 0.246 | 1.33 | 11.55 |
| OUTFLOW: ID= 1 (0401) | 9.980 | 0.076 | 3.12 | 11.54 |

| PEAK FLOW | REDUCTION [Qout/Qin] (%) | TIME SHIFT OF PEAK FLOW (min) | MAXIMUM STORAGE USED (ha.m.) |
|-----------|--------------------------|-------------------------------|------------------------------|
| 30.69 | 107.00 | 0.0293 | |

 ** SIMULATION:Run 03 - 10 Year 4 Hour Chicago **

```

RESERVOIR( 0401)
IN= 2---> OUT= 1
DT= 1.0 min
OVERFLOW IS OFF
  
```

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.1280 | 0.1640 |
| 0.0690 | 0.0180 | 0.1340 | 0.1870 |
| 0.0800 | 0.0370 | 0.1400 | 0.2110 |
| 0.0900 | 0.0570 | 0.1460 | 0.2360 |
| 0.0990 | 0.0770 | 0.1510 | 0.2610 |
| 0.1070 | 0.0980 | 0.1570 | 0.2870 |
| 0.1140 | 0.1190 | 0.1660 | 0.3350 |
| 0.1210 | 0.1410 | 0.0000 | 0.0000 |

| INFLOW : ID= 2 (0901) | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|------------------------|-----------|-------------|-------------|-----------|
| 9.980 | 9.980 | 0.301 | 1.33 | 14.94 |
| OUTFLOW: ID= 1 (0401) | 9.980 | 0.086 | 3.50 | 14.94 |

| PEAK FLOW | REDUCTION [Qout/Qin] (%) | TIME SHIFT OF PEAK FLOW (min) | MAXIMUM STORAGE USED (ha.m.) |
|-----------|--------------------------|-------------------------------|------------------------------|
| 28.42 | 130.00 | 0.0480 | |

 ** SIMULATION:Run 04 - 25 Hour 4 Hour Chicago **

```

RESERVOIR( 0401)
IN= 2---> OUT= 1
DT= 1.0 min
OVERFLOW IS OFF
  
```

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.1280 | 0.1640 |
| 0.0690 | 0.0180 | 0.1340 | 0.1870 |
| 0.0800 | 0.0370 | 0.1400 | 0.2110 |
| 0.0900 | 0.0570 | 0.1460 | 0.2360 |
| 0.0990 | 0.0770 | 0.1510 | 0.2610 |
| 0.1070 | 0.0980 | 0.1570 | 0.2870 |
| 0.1140 | 0.1190 | 0.1660 | 0.3350 |
| 0.1210 | 0.1410 | 0.0000 | 0.0000 |

| INFLOW : ID= 2 (0901) | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|------------------------|-----------|-------------|-------------|-----------|
| 9.980 | 9.980 | 0.371 | 1.33 | 19.53 |
| OUTFLOW: ID= 1 (0401) | 9.980 | 0.099 | 3.85 | 19.52 |

| PEAK FLOW | REDUCTION [Qout/Qin] (%) | TIME SHIFT OF PEAK FLOW (min) | MAXIMUM STORAGE USED (ha.m.) |
|-----------|--------------------------|-------------------------------|------------------------------|
| 26.59 | 151.00 | 0.0760 | |

 ** SIMULATION:Run 05 - 50 Year 4 Hour Chicago **

```

RESERVOIR( 0401)
IN= 2---> OUT= 1
DT= 1.0 min
OVERFLOW IS OFF
  
```

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.1280 | 0.1640 |
| 0.0690 | 0.0180 | 0.1340 | 0.1870 |
| 0.0800 | 0.0370 | 0.1400 | 0.2110 |
| 0.0900 | 0.0570 | 0.1460 | 0.2360 |
| 0.0990 | 0.0770 | 0.1510 | 0.2610 |
| 0.1070 | 0.0980 | 0.1570 | 0.2870 |
| 0.1140 | 0.1190 | 0.1660 | 0.3350 |
| 0.1210 | 0.1410 | 0.0000 | 0.0000 |

| INFLOW : ID= 2 (0901) | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|------------------------|-----------|-------------|-------------|-----------|
| 9.980 | 9.980 | 0.429 | 1.33 | 23.26 |
| OUTFLOW: ID= 1 (0401) | 9.980 | 0.108 | 4.02 | 23.25 |

| PEAK FLOW | REDUCTION [Qout/Qin] (%) | TIME SHIFT OF PEAK FLOW (min) | MAXIMUM STORAGE USED (ha.m.) |
|-----------|--------------------------|-------------------------------|------------------------------|
| 25.12 | 161.00 | 0.1006 | |

 ** SIMULATION:Run 06 - 100 Year 4 Hour Chicago **

```

RESERVOIR( 0401)
IN= 2---> OUT= 1
DT= 1.0 min
OVERFLOW IS OFF
  
```

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.1280 | 0.1640 |
| 0.0690 | 0.0180 | 0.1340 | 0.1870 |
| 0.0800 | 0.0370 | 0.1400 | 0.2110 |
| 0.0900 | 0.0570 | 0.1460 | 0.2360 |
| 0.0990 | 0.0770 | 0.1510 | 0.2610 |
| 0.1070 | 0.0980 | 0.1570 | 0.2870 |
| 0.1140 | 0.1190 | 0.1660 | 0.3350 |
| 0.1210 | 0.1410 | 0.0000 | 0.0000 |

| INFLOW : ID= 2 (0901) | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|------------------------|-----------|-------------|-------------|-----------|
| 9.980 | 9.980 | 0.487 | 1.33 | 27.10 |
| OUTFLOW: ID= 1 (0401) | 9.980 | 0.117 | 4.08 | 27.09 |

| PEAK FLOW | REDUCTION [Qout/Qin] (%) | TIME SHIFT OF PEAK FLOW (min) | MAXIMUM STORAGE USED (ha.m.) |
|-----------|--------------------------|-------------------------------|------------------------------|
| 23.94 | 165.00 | 0.1270 | |

 ** SIMULATION:Run 07 - 2 Year 24 Hour SCS **

```

RESERVOIR( 0401)
IN= 2---> OUT= 1
OVERFLOW IS OFF
  
```

DT= 1.0 min

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.1280 | 0.1640 |
| 0.0690 | 0.0180 | 0.1340 | 0.1870 |
| 0.0800 | 0.0370 | 0.1400 | 0.2110 |
| 0.0900 | 0.0570 | 0.1460 | 0.2360 |
| 0.0990 | 0.0770 | 0.1510 | 0.2610 |
| 0.1070 | 0.0980 | 0.1570 | 0.2870 |
| 0.1140 | 0.1190 | 0.1660 | 0.3350 |
| 0.1210 | 0.1410 | 0.0000 | 0.0000 |

| AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-----------|-------------|-------------|-----------|
| 9.980 | 0.212 | 11.77 | 17.79 |
| 9.980 | 0.081 | 13.25 | 17.78 |

| PEAK FLOW | REDUCTION [Qout/Qin] (%) |
|-------------------------|--------------------------|
| 38.24 | |
| TIME SHIFT OF PEAK FLOW | MAXIMUM STORAGE USED |
| (min)= 89.00 | (ha.m.)= 0.0392 |

SIMULATION:Run 08 - 5 Year 24 Hour SCS

RESERVOIR(0401)
 IN= 2--> OUT= 1
 DT= 1.0 min

OVERFLOW IS OFF

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.1280 | 0.1640 |
| 0.0690 | 0.0180 | 0.1340 | 0.1870 |
| 0.0800 | 0.0370 | 0.1400 | 0.2110 |
| 0.0900 | 0.0570 | 0.1460 | 0.2360 |
| 0.0990 | 0.0770 | 0.1510 | 0.2610 |
| 0.1070 | 0.0980 | 0.1570 | 0.2870 |
| 0.1140 | 0.1190 | 0.1660 | 0.3350 |
| 0.1210 | 0.1410 | 0.0000 | 0.0000 |

| AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-----------|-------------|-------------|-----------|
| 9.980 | 0.330 | 11.78 | 28.31 |
| 9.980 | 0.101 | 13.67 | 28.31 |

| PEAK FLOW | REDUCTION [Qout/Qin] (%) |
|-------------------------|--------------------------|
| 30.73 | |
| TIME SHIFT OF PEAK FLOW | MAXIMUM STORAGE USED |
| (min)=113.00 | (ha.m.)= 0.0835 |

SIMULATION:Run 09 - 10 Year 24 Hour SCS

RESERVOIR(0401)
 IN= 2--> OUT= 1
 DT= 1.0 min

OVERFLOW IS OFF

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.1280 | 0.1640 |
| 0.0690 | 0.0180 | 0.1340 | 0.1870 |
| 0.0800 | 0.0370 | 0.1400 | 0.2110 |
| 0.0900 | 0.0570 | 0.1460 | 0.2360 |
| 0.0990 | 0.0770 | 0.1510 | 0.2610 |
| 0.1070 | 0.0980 | 0.1570 | 0.2870 |
| 0.1140 | 0.1190 | 0.1660 | 0.3350 |
| 0.1210 | 0.1410 | 0.0000 | 0.0000 |

| AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-----------|-------------|-------------|-----------|
| 9.980 | 0.421 | 11.78 | 36.07 |
| 9.980 | 0.114 | 13.87 | 36.06 |

| PEAK FLOW | REDUCTION [Qout/Qin] (%) |
|-------------------------|--------------------------|
| 27.11 | |
| TIME SHIFT OF PEAK FLOW | MAXIMUM STORAGE USED |
| (min)=125.00 | (ha.m.)= 0.1197 |

SIMULATION:Run 10 - 25 Year 24 Hour SCS

RESERVOIR(0401)
 IN= 2--> OUT= 1
 DT= 1.0 min

OVERFLOW IS OFF

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.1280 | 0.1640 |

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0690 | 0.0180 | 0.1340 | 0.1870 |
| 0.0800 | 0.0370 | 0.1400 | 0.2110 |
| 0.0900 | 0.0570 | 0.1460 | 0.2360 |
| 0.0990 | 0.0770 | 0.1510 | 0.2610 |
| 0.1070 | 0.0980 | 0.1570 | 0.2870 |
| 0.1140 | 0.1190 | 0.1660 | 0.3350 |
| 0.1210 | 0.1410 | 0.0000 | 0.0000 |

| AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-----------|-------------|-------------|-----------|
| 9.980 | 0.546 | 11.78 | 46.25 |
| 9.980 | 0.130 | 14.05 | 46.24 |

| PEAK FLOW | REDUCTION [Qout/Qin] (%) |
|-------------------------|--------------------------|
| 23.74 | |
| TIME SHIFT OF PEAK FLOW | MAXIMUM STORAGE USED |
| (min)=136.00 | (ha.m.)= 0.1699 |

SIMULATION:Run 11 - 50 year 24 Hour SCS

RESERVOIR(0401)
 IN= 2--> OUT= 1
 DT= 1.0 min

OVERFLOW IS OFF

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.1280 | 0.1640 |
| 0.0690 | 0.0180 | 0.1340 | 0.1870 |
| 0.0800 | 0.0370 | 0.1400 | 0.2110 |
| 0.0900 | 0.0570 | 0.1460 | 0.2360 |
| 0.0990 | 0.0770 | 0.1510 | 0.2610 |
| 0.1070 | 0.0980 | 0.1570 | 0.2870 |
| 0.1140 | 0.1190 | 0.1660 | 0.3350 |
| 0.1210 | 0.1410 | 0.0000 | 0.0000 |

| AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-----------|-------------|-------------|-----------|
| 9.980 | 0.642 | 11.80 | 54.34 |
| 9.980 | 0.140 | 14.15 | 54.33 |

| PEAK FLOW | REDUCTION [Qout/Qin] (%) |
|-------------------------|--------------------------|
| 21.82 | |
| TIME SHIFT OF PEAK FLOW | MAXIMUM STORAGE USED |
| (min)=141.00 | (ha.m.)= 0.2118 |

SIMULATION:Run 12 - 100 Year 24 Hour SCS

RESERVOIR(0401)
 IN= 2--> OUT= 1
 DT= 1.0 min

OVERFLOW IS OFF

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.1280 | 0.1640 |
| 0.0690 | 0.0180 | 0.1340 | 0.1870 |
| 0.0800 | 0.0370 | 0.1400 | 0.2110 |
| 0.0900 | 0.0570 | 0.1460 | 0.2360 |
| 0.0990 | 0.0770 | 0.1510 | 0.2610 |
| 0.1070 | 0.0980 | 0.1570 | 0.2870 |
| 0.1140 | 0.1190 | 0.1660 | 0.3350 |
| 0.1210 | 0.1410 | 0.0000 | 0.0000 |

| AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-----------|-------------|-------------|-----------|
| 9.980 | 0.769 | 11.75 | 62.58 |
| 9.980 | 0.150 | 14.27 | 62.57 |

| PEAK FLOW | REDUCTION [Qout/Qin] (%) |
|-------------------------|--------------------------|
| 19.49 | |
| TIME SHIFT OF PEAK FLOW | MAXIMUM STORAGE USED |
| (min)=151.00 | (ha.m.)= 0.2555 |

SIMULATION:Run 13 - 25 mm

RESERVOIR(0401)
 IN= 2--> OUT= 1
 DT= 1.0 min

OVERFLOW IS OFF

| OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.) |
|---------------|-----------------|---------------|-----------------|
| 0.0000 | 0.0000 | 0.1280 | 0.1640 |
| 0.0690 | 0.0180 | 0.1340 | 0.1870 |
| 0.0800 | 0.0370 | 0.1400 | 0.2110 |
| 0.0900 | 0.0570 | 0.1460 | 0.2360 |

| | | | |
|--------|--------|--------|--------|
| 0.0990 | 0.0770 | 0.1510 | 0.2610 |
| 0.1070 | 0.0980 | 0.1570 | 0.2870 |
| 0.1140 | 0.1190 | 0.1660 | 0.3350 |
| 0.1210 | 0.1410 | 0.0000 | 0.0000 |

| | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|------------------------|--------------|----------------|----------------|--------------|
| INFLOW : ID= 2 (0901) | 9.980 | 0.087 | 1.50 | 4.30 |
| OUTFLOW: ID= 1 (0401) | 9.980 | 0.031 | 2.75 | 4.30 |

PEAK FLOW REDUCTION [Qout/Qin](%)= 36.01
 TIME SHIFT OF PEAK FLOW (min)= 75.00
 MAXIMUM STORAGE USED (ha.m.)= 0.0082

=====

V V I SSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
VV I SSSS UUUU A A LLLLL

000 TTTTT TTTTT H H Y Y M M 000 TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
000 T T H H Y Y M M 000

Developed and Distributed by Smart City Water Inc
Copyright 2007 - 2021 Smart City Water Inc
All rights reserved.

***** S U M M A R Y O U T P U T *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\aa6199a1-aad0
Summary filename: C:\Users\LBuss\AppData\Local\Civica\vh5\18b1f826-8929-408e-9936-e2d934269cf1\aa6199a1-aad0

DATE: 01-19-2022 TIME: 08:06:58

USER:

COMMENTS: _____

** SIMULATION : HAZEL.STM **

| W/E COMMAND | HYD ID | DT min | AREA ha | Qpeak cms | Tpeak hrs | R.V. mm | R.C. | Qbase cms |
|---|--------|--------|---------|-----------|-----------|---------|--------|------------|
| START @ 0.00 hrs | | | | | | | | |
| READ STORM [Ptot=212.00 mm] fname : C:\Users\LBuss\AppData\Local\Temp\cf3225d1-d683-447b-9033-5525d80b52d4\b3895df3-8984-4913-9a29-d81 remark: HAZEL.STM | | | | | 12.0 | | | |
| ** CALIB NASHYD [CN=78.0] [N = 3.0:Tp 1.16] | 0201 | 1 | 14.0 | 56.24 | 5.11 | 11.20 | 149.83 | 0.71 0.000 |
| READ STORM [Ptot=212.00 mm] fname : C:\Users\LBuss\AppData\Local\Temp\cf3225d1-d683-447b-9033-5525d80b52d4\b3895df3-8984-4913-9a29-d81 remark: HAZEL.STM | | | | | 12.0 | | | |
| ** CALIB NASHYD [CN=82.0] [N = 3.0:Tp 0.91] | 0202 | 1 | 10.0 | 6.45 | 0.66 | 11.00 | 163.02 | 0.77 0.000 |
| READ STORM [Ptot=212.00 mm] fname : C:\Users\LBuss\AppData\Local\Temp\cf3225d1-d683-447b-9033-5525d80b52d4\b3895df3-8984-4913-9a29-d81 remark: HAZEL.STM | | | | | 12.0 | | | |
| ** CALIB NASHYD [CN=87.0] [N = 3.0:Tp 0.07] | 0106 | 1 | 1.0 | 0.58 | 0.08 | 10.00 | 176.04 | 0.83 0.000 |
| READ STORM [Ptot=212.00 mm] fname : C:\Users\LBuss\AppData\Local\Temp\cf3225d1-d683-447b-9033-5525d80b52d4\b3895df3-8984-4913-9a29-d81 remark: HAZEL.STM | | | | | 12.0 | | | |
| * CALIB STANDHYD [I%=10.0:S%= 2.00] | 1001 | 1 | 10.0 | 0.35 | 0.05 | 10.00 | 167.00 | 0.79 0.000 |
| READ STORM [Ptot=212.00 mm] fname : C:\Users\LBuss\AppData\Local\Temp\cf3225d1-d683-447b-9033-5525d80b52d4\b3895df3-8984-4913-9a29-d81 remark: HAZEL.STM | | | | | 12.0 | | | |
| * CALIB STANDHYD | 0105 | 1 | 10.0 | 1.70 | 0.23 | 10.00 | 173.51 | 0.82 0.000 |

```

* [I%=25.0:S%= 2.00]
* READ STORM 12.0
* [ Ptot=212.00 mm ]
* fname : C:\Users\LBuss\AppData\Local\Temp\cf3225d1-d683-447b-9033-5525d80b52d4\b3895df3-8984-4913-9a29-d81
* remark: HAZEL.STM
* * CALIB STANDHYD 0100 1 10.0 0.90 0.12 10.00 169.33 0.80 0.000
* [I%=30.0:S%= 2.00]
* ADD [ 0100+ 1001] 0901 3 10.0 1.25 0.17 10.00 168.68 n/a 0.000
* ADD [ 0901+ 0105] 0901 1 10.0 2.95 0.40 10.00 171.46 n/a 0.000
* ADD [ 0901+ 0106] 0901 3 1.0 3.53 0.48 10.00 172.21 n/a 0.000
* ADD [ 0901+ 0202] 0901 1 1.0 9.98 1.02 11.00 166.27 n/a 0.000
* ADD [ 0201+ 0901] 0902 3 1.0 66.22 6.03 11.00 152.31 n/a 0.000
* FINISH
=====

```

Water Quality Control

Water Quality Treatment Train Calculation

Site Area

Drainage Area to Controls= 4.57 ha

Imperviousness of Drainage Area to Controls= 20%

| Device | | Target Total Suspended Solids (TSS) Removal |
|---------------------|-------------------|---|
| Primary Treatment | Enhanced Ditches | 40% |
| Secondary Treatment | Existing Dry Pond | 60% |
| Tertiary Treatment | Veg. Filter Strip | 35% |

$$TSS\ Removal = (1 - ((1 - R_P) * (1 - R_S) * (1 - R_T)))$$

where:

R_P = % TSS Removal provided by Primary Treatment Strategy

R_S = % TSS Removal provided by Secondary Treatment Strategy

R_T = % TSS Removal provided by Tertiary Treatment Strategy

TSS Removal Provided By Controls = 84% %

Water Quality Treatment Train Calculation

Site Area

Drainage Area to Controls= 1.68 ha

Imperviousness of Drainage Area to Controls= 20%

| Device | | Target Total Suspended Solids (TSS) Removal |
|---------------------|-------------------------|---|
| Primary Treatment | Enhanced Ditches | 40% |
| Secondary Treatment | Enhanced Lot-Line Swale | 40% |

$$TSS\ Removal = (1 - ((1 - R_p) * (1 - R_s)))$$

where:

R_p = % TSS Removal provided by Primary Treatment Strategy

R_s = % TSS Removal provided by Secondary Treatment Strategy

TSS Removal Provided By Controls = 64% %

Note: Site drainage area of 1.68 ha subject to water quality controls excludes rooftop area as this area is generally considered to be clean, and, therefore, not subject to water quality controls.

Appendix D: Phosphorus Budget Calculations



| | | | |
|---------|------------------------------|----------|------------|
| PROJECT | Melville Court Development | FILE | 420427 |
| | | DATE | 2021-12-24 |
| SUBJECT | Phosphorus Budget Assessment | DESIGNED | LB |
| | | CHECKED | NHF |

Phosphorus Loading

| LAND USE CATEGORY | Existing Phosphorus Loading Rate (kg/ha/year) | Future Phosphorus Loading Rate (kg/ha/year) | Existing | | Proposed | |
|----------------------------------|---|---|--------------------|---------------------------------------|-------------|---------------------------|
| | | | Existing Area (ha) | Existing Phosphorus Loading (kg/year) | Area (ha) | Phosphorus Load (kg/year) |
| Cropland | 0.19 | 0.19 | 0.00 | 0.00 | 0.00 | 0.00 |
| Hay-Pasture | 0.07 | 0.07 | 5.42 | 0.38 | 0.00 | 0.00 |
| Turf-Sod | 0.12 | 0.12 | 0.00 | 0.00 | 0.33 | 0.04 |
| High Intensity Development - C/I | 1.82 | 1.82 | 0.00 | 0.00 | 0.00 | 0.00 |
| High Intensity Development - R | 1.32 | 1.32 | 0.00 | 0.00 | 0.00 | 0.00 |
| Low Intensity Development | 0.13 | 0.13 | 0.00 | 0.00 | 6.25 | 0.81 |
| Quarry | 0.08 | 0.08 | 0.00 | 0.00 | 0.00 | 0.00 |
| Unpaved Road | 0.83 | 0.83 | 0.00 | 0.00 | 0.00 | 0.00 |
| Forest | 0.05 | 0.05 | 1.16 | 0.06 | 0.00 | 0.00 |
| Transition | 0.06 | 0.06 | 0.00 | 0.00 | 0.00 | 0.00 |
| Wetland | 0.05 | 0.05 | 0.00 | 0.00 | 0.00 | 0.00 |
| Open Water | 0.26 | 0.26 | 0.00 | 0.00 | 0.00 | 0.00 |
| Total | | | 6.58 | 0.44 | 6.58 | 0.85 |

Controls

| Land Use Area | Control Removal Efficiency (%) | | | | | Proposed | | |
|---|--------------------------------|------------|------------------------|------------|----------|-----------|----------------------------|------|
| | Infiltration Trenches | Enh. Swale | Vegetated Filter Strip | Enh. Ditch | Dry Pond | Area (ha) | Phosphorous Load (kg/year) | |
| Low Intensity Development (50% of Rooftop Area) | 0.60 | | 20% | 25% | 10% | 0.23 | 0.01 | |
| Low Intensity Development (to Outlet #1) | | | 20% | 25% | 10% | 4.34 | 0.30 | |
| Low Intensity Development (to Outlet #2) | | 25% | | 25% | | 1.68 | 0.12 | |
| Turf-Sod | | | 20% | | | 0.33 | 0.03 | |
| Total Phosphorus (No Controls) | | | | | | | | 0.85 |
| Total Phosphorus (With Controls) | | | | | | | | 0.47 |

Summary

| | | |
|--|------|---------|
| Existing Phosphorus Load | 0.44 | kg/year |
| Post Development Phosphorus Load (no controls) | 0.85 | kg/year |
| Increase | 0.41 | kg/year |
| Post Development Phosphorus Load (with controls) | 0.47 | kg/year |

Phosphorus Offsetting Policy

| | | |
|--|---------------------|---------|
| Post Development Phosphorus Load (with controls) | 0.47 | kg/year |
| Offset Calculation (2.5 * P deficit * \$35,000) | \$ 40,740.51 | |

Appendix E: Utilities Correspondence

John-Lui Marra

From: David Smith <David.Smith@enbridge.com>
Sent: December 16, 2021 7:55 AM
To: John-Lui Marra
Cc: Matt Brace
Subject: RE: 420427 - Melville Court Development - Utility Inquiry

Follow Up Flag: Follow up
Flag Status: Completed

CAUTION: This email originated from outside of Tatham Engineering. Do not click on links or open attachments unless you know the sender and have verified the sender's email address and know the content is safe.

Good morning John.

We currently have main on the south side of Melville and I think we are going to want to run a new main on the north side to avoid 14 long services. Also there is no main in front of lots 15 and 16.

I will send this to planning and have them determine if gas is available. With the holiday season close to upon us I probably will not have anything back until mid January.

I hope that works.

Thanks.

David Smith

Customer Connections Representative

ENBRIDGE

TEL: 705-739-5254 | CELL: 705-220-5997 | FAX: 705-739-5200

10 Churchill Dr. Barrie ON L4N 8Z5

enbridge.com

Safety. Integrity. Respect. Inclusion.

From: John-Lui Marra <jlmarra@tathameng.com>
Sent: Wednesday, December 15, 2021 4:24 PM
To: David Smith <David.Smith@enbridge.com>
Cc: Matt Brace <mbrace@tathameng.com>
Subject: [External] 420427 - Melville Court Development - Utility Inquiry

CAUTION: EXTERNAL EMAIL

This email originated from outside Enbridge and could be a phish. Criminals can pretend to be anyone. Do not interact with the email unless you are 100% certain it is legitimate. Report any suspicious emails.

Good afternoon David,

I work with Nick Millington at Tatham Engineering who passed along your contact information.

We are working on behalf of Doncor Developments Inc. in support of official plan amendment, zoning by-law amendment, and draft plan of subdivision applications for a development project located on the north side of Melville Court in Oro-Medonte, near Highway 11 and Line 5 North. The project proposes to develop existing agricultural lands by constructing 16 new residences (site plan attached). Currently, only the south side of Melville Court has existing residential units.

We are looking for preliminary comment regarding feasibility of gas servicing for the site to allow us to include commentary regarding such in our Functional Servicing Report.

If you could kindly review and provide comment at your earliest convenience it would be greatly appreciated.

Thanks,

John-Lui Marra, B.Eng.
Engineering Intern

Tatham Engineering Limited

41 King Street, Unit 4 | Barrie | Ontario | L4N 6B5

T 705-733-9037 x2184 | C 647-404-2070 | jlmarra@tathameng.com | tathameng.com



Enhancing our communities

This email may contain confidential and/or privileged information for the sole use of the intended recipient. Any review or distribution by others is strictly prohibited. If you have received this email in error, please contact the sender and delete all copies.

Tatham Engineering Limited's agreement to transfer digital documents electronically or otherwise is made under the following conditions:

1. Electronic documents made available by Tatham Engineering Limited are supplied for the recipient's use only under authorization from the current owner and with the consent of Tatham Engineering Limited. It is the responsibility of the recipient to determine the accuracy, completeness and the appropriateness of the information provided.

2. It is agreed that only those hard copy documents bearing the professional seal and signature of the Tatham Engineering Limited project engineer will govern the work of the project. In the event of any dispute concerning an electronic document, the appropriately dated hard copy will be the document used by Tatham Engineering Limited to govern and resolve the dispute.

John-Lui Marra

To: David Smith
Subject: RE: 420427 - Melville Court Development - Utility Inquiry

From: David Smith <David.Smith@enbridge.com>
Sent: January 19, 2022 9:54 AM
To: John-Lui Marra <jlmarra@tathameng.com>; Area Planning 50 <AreaPlanning50@enbridge.com>
Cc: Matt Brace <mbrace@tathameng.com>
Subject: RE: 420427 - Melville Court Development - Utility Inquiry

Hi John.

We had a little miscommunication here on our end. See my email from earlier. I am submitting this for gas availability to ensure gas is available. As mentioned in my earlier email we will have to install a main on the north side of Melville Court to avoid 14 long services. Costs for the entire site will vary depending on whether the portion along the north side of Melville is installed joint trench with hydro. Is this a possibility? I leave that question with you. First we need to ensure that gas is available off our existing plant.

I will get back to you with that shortly.

Thanks.

David Smith

Customer Connections Representative

ENBRIDGE

TEL: 705-739-5254 | CELL: 705-220-5997 | FAX: 705-739-5200
10 Churchill Dr. Barrie ON L4N 8Z5

enbridge.com

Safety. Integrity. Respect. Inclusion.

From: John-Lui Marra <jlmarra@tathameng.com>
Sent: Wednesday, January 19, 2022 9:48 AM
To: Area Planning 50 <AreaPlanning50@enbridge.com>
Cc: Matt Brace <mbrace@tathameng.com>; David Smith <David.Smith@enbridge.com>
Subject: [External] RE: 420427 - Melville Court Development - Utility Inquiry

CAUTION: EXTERNAL EMAIL

This email originated from outside Enbridge and could be a phish. Criminals can pretend to be anyone. Do not interact with the email unless you are 100% certain it is legitimate. Report any suspicious emails.

Good morning Jennifer,

I appreciate the detailed response. Can you please provide a high-level cost estimate for the main extension to Lots 15 and 16? It would be for the owner's reference.

Thanks,

John-Lui Marra, B.Eng., EIT
Engineering Intern

Tatham Engineering Limited

41 King Street, Unit 4 | Barrie | Ontario | L4N 6B5

T 705-733-9037 x2184 | C 647-404-2070 | jlmarra@tathameng.com | tathameng.com



Enhancing our communities

This email may contain confidential and/or privileged information for the sole use of the intended recipient. Any review or distribution by others is strictly prohibited. If you have received this email in error, please contact the sender and delete all copies.

From: Area Planning 50 <AreaPlanning50@enbridge.com>

Sent: January 19, 2022 9:04 AM

To: John-Lui Marra <jlmarra@tathameng.com>

Cc: Matt Brace <mbrace@tathameng.com>; David Smith <David.Smith@enbridge.com>

Subject: RE: 420427 - Melville Court Development - Utility Inquiry

CAUTION: This email originated from outside of Tatham Engineering. Do not click on links or open attachments unless you know the sender and have verified the sender's email address and know the content is safe.

Hi John,

It looks there is already gas main along Melville Court, so with the exception of Lot 15 and 16, all that is required is the gas services to the individual homes. I've attached the Builder Service Order Form, which can be used if there is one builder only within the site. If this isn't the case, please let me know. Please also include the registered plan if at all possible, showing both lot number and civic numbers.

The timeframe for gas services is typically about 6-8 weeks for an exclusive trench gas service, if there are no issues. It looks like the east end of the street does fall within the Lake Simcoe Region Conservation Authority's regulated area, so Enbridge would need to get a Conservation permit for any services that fall within this regulated area. This adds approximately 6-8 weeks onto the timeframe.

The current costs for gas services that are coming off existing gas main are as follows:

Enbridge will provide and install at no cost one service line per civic address to new customers provided that:

1. The distance between the Owner's property line and the front wall of house/building is 20 meters or less
2. The distance between the front wall of house/building and the selected meter location is 2 meters or less

Service and meter installation in excess of these distances will result in additional charges of \$32 per meter (plus applicable taxes). In-field measurements or in-field changes will result in additional charges.

For Lots 15 and 16, it does not look like there is gas main coverage on Line 5 North. As these two lots don't have frontage onto Melville Court, we also couldn't run the services off the main on that street either. It looks like a main extension would be required north on Line 5 N. Main extensions for only a couple of homes would incur a significant contribution. The timeframe for main extensions are quite a bit longer as well, typically around 8-12 months. If you'd like we can get a high level estimate for the main extension to reach these two homes.

Sincerely,

Jennifer Wingrove-Jones

Customer Connections Coordinator

ENBRIDGE

TEL: 705-739-5237 | FAX: 705-739-5200
10 Churchill Dr, Barrie, Ontario L4N 8Z5

enbridge.com

Safety. Integrity. Respect. Inclusion.

From: John-Lui Marra <jlmarra@tathameng.com>
Sent: December 15, 2021 4:21 PM
To: Area Planning 50 <AreaPlanning50@enbridge.com>
Cc: Matt Brace <mbrace@tathameng.com>
Subject: 420427 - Melville Court Development - Utility Inquiry

Good afternoon,

We are working on behalf of Doncor Developments Inc. in support of official plan amendment, zoning by-law amendment, and draft plan of subdivision applications for a development project located on the north side of Melville Court in Oro-Medonte, near Highway 11 and Line 5 North. The project proposes to develop existing agricultural lands by constructing 16 new residences (site plan attached). Currently, only the south side of Melville Court has existing residential units.

We are looking for preliminary comment regarding feasibility of gas servicing for the site to allow us to include commentary regarding such in our Functional Servicing Report.

If you could kindly review and provide comment at your earliest convenience it would be greatly appreciated.

Thanks,

John-Lui Marra, B.Eng.

Engineering Intern

Tatham Engineering Limited

41 King Street, Unit 4 | Barrie | Ontario | L4N 6B5

T 705-733-9037 x2184 | C 647-404-2070 | jlmarra@tathameng.com | tathameng.com



Enhancing our communities

This email may contain confidential and/or privileged information for the sole use of the intended recipient. Any review or distribution by others is strictly prohibited. If you have received this email in error, please contact the sender and delete all copies.

Tatham Engineering Limited's agreement to transfer digital documents electronically or otherwise is made under the following conditions:

1. Electronic documents made available by Tatham Engineering Limited are supplied for the recipient's use only under authorization from the current owner and with the consent of Tatham Engineering Limited. It is the responsibility of the recipient to determine the accuracy, completeness and the appropriateness of the information provided.
2. It is agreed that only those hard copy documents bearing the professional seal and signature of the Tatham Engineering Limited project engineer will govern the work of the project. In the event of any dispute concerning an electronic document, the appropriately dated hard copy will be the document used by Tatham Engineering Limited to govern and resolve the dispute.

John-Lui Marra

From: Damstedt, Shannon <shannon.micks@bell.ca>
Sent: December 16, 2021 12:29 PM
To: John-Lui Marra
Cc: Matt Brace
Subject: RE: 420427 - Melville Court Development - Utility Inquiry
Attachments: RFI.xls

CAUTION: This email originated from outside of Tatham Engineering. Do not click on links or open attachments unless you know the sender and have verified the sender's email address and know the content is safe.

Good Afternoon,

Please fill out the attached form and return it at your earliest convenience and then I can forward that to governance and provide you with the plan for Bell.

We have copper cable feeding the homes on the South side at the very least I think we would provide copper to the new homes.

Thank you

Shannon



Shannon Damstedt
Network Planning & Provisioning
Network Provisioning Manager
136 Bayfield St. FL2
Barrie, ON, L4M 3B1

C:(705)627-0597
email: shannon.micks@bell.ca

From: O'Brien, Carol A <carol.obrien@bell.ca>
Sent: December-16-21 8:08 AM
To: John-Lui Marra <jlmarra@tathameng.com>
Cc: Matt Brace <mbrace@tathameng.com>; Damstedt, Shannon <shannon.micks@bell.ca>
Subject: RE: 420427 - Melville Court Development - Utility Inquiry

Good morning

This proposed development would fall under the territory that my peer Shannon Damstedt would manage. I have cc'd her on this email.

Carol



Carol O'Brien
Network Provisioning Manager
136 Bayfield st, Barrie ON
office: 705-722-2405
cell: 705-627-0259

From: John-Lui Marra <jlmarra@tathameng.com>
Sent: December-15-21 4:18 PM
To: O'Brien, Carol A <carol.obrien@bell.ca>
Cc: Matt Brace <mbrace@tathameng.com>
Subject: [EXT]420427 - Melville Court Development - Utility Inquiry

Good afternoon Carol,

I work with Nick Millington at Tatham Engineering who passed along your contact information.

We are working on behalf of Doncor Developments Inc. in support of official plan amendment, zoning by-law amendment, and draft plan of subdivision applications for a development project located on the north side of Melville Court in Oro-Medonte, near Highway 11 and Line 5 North. The project proposes to develop existing agricultural lands by constructing 16 new residences (site plan attached). Currently, only the south side of Melville Court has existing residential units.

We are looking for preliminary comment regarding feasibility of Bell servicing for the site to allow us to include commentary regarding such in our Functional Servicing Report.

If you could kindly review and provide comment at your earliest convenience it would be greatly appreciated.

Thanks,

John-Lui Marra, B.Eng.
Engineering Intern

Tatham Engineering Limited
41 King Street, Unit 4 | Barrie | Ontario | L4N 6B5
T 705-733-9037 x2184 | C 647-404-2070 | jlmarra@tathameng.com | tathameng.com



This email may contain confidential and/or privileged information for the sole use of the intended recipient. Any review or distribution by others is strictly prohibited. If you have received this email in error, please contact the sender and delete all copies.

Tatham Engineering Limited's agreement to transfer digital documents electronically or otherwise is made under the following conditions:

1. Electronic documents made available by Tatham Engineering Limited are supplied for the recipient's use only under authorization from the current owner and with the consent of Tatham Engineering Limited. It is the responsibility of the recipient to determine the accuracy, completeness and the appropriateness of the information provided.
2. It is agreed that only those hard copy documents bearing the professional seal and signature of the Tatham Engineering Limited project engineer will govern the work of the project. In the event of any dispute concerning an electronic document, the appropriately dated hard copy will be the document used by Tatham Engineering Limited to govern and resolve the dispute.

External Email: Please use caution when opening links and attachments / Courriel externe: Soyez prudent avec les liens et documents joints

John-Lui Marra

From: Xinyi Wang <Xinyi.Wang@rci.rogers.com>
Sent: January 4, 2022 3:21 PM
To: John-Lui Marra
Cc: Matt Brace
Subject: RE: 420427 - Melville Court Development - Utility Inquiry

Hi John,

Subdivision currently requires new fibre build and new node for servicing. Rogers only has existing fibre support for commercial units in this area.

What is the proposed timeline for this subdivision?

Regards,

Xinyi Wang
System Planner Outside Plant Engineering
xinyi.wang@rci.rogers.com



From: John-Lui Marra <jlmarra@tathameng.com>
Sent: December 22, 2021 11:21 AM
To: Xinyi Wang <Xinyi.Wang@rci.rogers.com>
Cc: Matt Brace <mbrace@tathameng.com>; Graham McPherson <Graham.McPherson@rci.rogers.com>
Subject: RE: 420427 - Melville Court Development - Utility Inquiry

Good morning Xinyi,

I am following up on the below emails. Attached is the site plan of the proposed development. Please advise at your earliest convenience.

Thanks,

John-Lui Marra, B.Eng.
Engineering Intern

Tatham Engineering Limited
41 King Street, Unit 4 | Barrie | Ontario | L4N 6B5
T 705-733-9037 x2184 | C 647-404-2070 | jlmarra@tathameng.com | tathameng.com



This email may contain confidential and/or privileged information for the sole use of the intended recipient. Any review or distribution by others is strictly prohibited. If you have received this email in error, please contact the sender and delete all copies.

Tatham Engineering Limited's agreement to transfer digital documents electronically or otherwise is made under the following conditions:

1. Electronic documents made available by Tatham Engineering Limited are supplied for the recipient's use only under authorization from the current owner and with the consent of Tatham Engineering Limited. It is the responsibility of the recipient to determine the accuracy, completeness and the appropriateness of the information provided.

2. It is agreed that only those hard copy documents bearing the professional seal and signature of the Tatham Engineering Limited project engineer will govern the work of the project. In the event of any dispute concerning an electronic document, the appropriately dated hard copy will be the document used by Tatham Engineering Limited to govern and resolve the dispute.

From: Graham McPherson <Graham.McPherson@rci.rogers.com>

Sent: December 15, 2021 5:03 PM

To: John-Lui Marra <jlmarra@tathameng.com>; Xinyi Wang <Xinyi.Wang@rci.rogers.com>

Cc: Matt Brace <mbrace@tathameng.com>

Subject: RE: 420427 - Melville Court Development - Utility Inquiry

CAUTION: This email originated from outside of Tatham Engineering. Do not click on links or open attachments unless you know the sender and have verified the sender's email address and know the content is safe.

Hi Xinyi, please see the attached request.

Thanks,

Graham McPherson
System Planner

Outside Plant Engineering - York
244 Newkirk Road
Richmond Hill, ON L4C 3S5

graham.mcperson@rci.rogers.com

o (705) 812-4528
m (705) 896-9608

From: John-Lui Marra <jlmarra@tathameng.com>
Sent: December 15, 2021 4:14 PM
To: Graham McPherson <Graham.McPherson@rci.rogers.com>
Cc: Matt Brace <mbrace@tathameng.com>
Subject: 420427 - Melville Court Development - Utility Inquiry

Good afternoon Graham,

I work with Nick Millington at Tatham Engineering who passed along your contact information.

We are working on behalf of Doncor Developments Inc. in support of official plan amendment, zoning by-law amendment, and draft plan of subdivision applications for a development project located on the north side of Melville Court in Oro-Medonte, near Highway 11 and Line 5 North. The project proposes to develop existing agricultural lands by constructing 16 new residences (site plan attached). Currently, only the south side of Melville Court has existing residential units.

We are looking for preliminary comment regarding feasibility of Rogers servicing for the site to allow us to include commentary regarding such in our Functional Servicing Report.

If you could kindly review and provide comment at your earliest convenience it would be greatly appreciated.

Thanks,

John-Lui Marra, B.Eng.
Engineering Intern

Tatham Engineering Limited

41 King Street, Unit 4 | Barrie | Ontario | L4N 6B5

T 705-733-9037 x2184 | C 647-404-2070 | jlmarra@tathameng.com | tathameng.com



This email may contain confidential and/or privileged information for the sole use of the intended recipient. Any review or distribution by others is strictly prohibited. If you have received this email in error, please contact the sender and delete all copies.

Tatham Engineering Limited's agreement to transfer digital documents electronically or otherwise is made under the following conditions:

1. Electronic documents made available by Tatham Engineering Limited are supplied for the recipient's use only under authorization from the current owner and with the consent of Tatham Engineering Limited. It is the responsibility of the recipient to determine the accuracy, completeness and the appropriateness of the information provided.

2. It is agreed that only those hard copy documents bearing the professional seal and signature of the Tatham Engineering Limited project engineer will govern the work of the project. In the event of any dispute concerning an electronic document, the appropriately dated hard copy will be the document used by Tatham Engineering Limited to govern and resolve the dispute.

This communication is confidential. We only send and receive email on the basis of the terms set out at www.rogers.com/web/content/emailnotice

Ce message est confidentiel. Notre transmission et réception de courriels se fait strictement suivant les modalités énoncées dans l'avis publié à www.rogers.com/aviscourriel

This communication is confidential. We only send and receive email on the basis of the terms set out at www.rogers.com/web/content/emailnotice

Ce message est confidentiel. Notre transmission et réception de courriels se fait strictement suivant les modalités énoncées dans l'avis publié à www.rogers.com/aviscourriel

Matt Brace

From: John-Lui Marra
Sent: December 22, 2021 11:20 AM
To: BRACKLEY Kevin; SZYMCZAK Sarah
Cc: Matt Brace
Subject: RE: 420427 - Melville Court Development - Utility Inquiry
Attachments: Site plan - Melville Court, Oro.pdf

Good morning Kevin and Sarah,

I am following up on the below email. Attached is the site plan of the proposed development. Please advise at your earliest convenience.

Thanks,

John-Lui Marra, B.Eng.
Engineering Intern

Tatham Engineering Limited
41 King Street, Unit 4 | Barrie | Ontario | L4N 6B5
T 705-733-9037 x2184 | C 647-404-2070 | jmarra@tathameng.com | tathameng.com



Enhancing our communities

This email may contain confidential and/or privileged information for the sole use of the intended recipient. Any review or distribution by others is strictly prohibited. If you have received this email in error, please contact the sender and delete all copies.

Tatham Engineering Limited's agreement to transfer digital documents electronically or otherwise is made under the following conditions:

1. Electronic documents made available by Tatham Engineering Limited are supplied for the recipient's use only under authorization from the current owner and with the consent of Tatham Engineering Limited. It is the responsibility of the recipient to determine the accuracy, completeness and the appropriateness of the information provided.
2. It is agreed that only those hard copy documents bearing the professional seal and signature of the Tatham Engineering Limited project engineer will govern the work of the project. In the event of any dispute concerning an electronic document, the appropriately dated hard copy will be the document used by Tatham Engineering Limited to govern and resolve the dispute.

From: John-Lui Marra
Sent: December 15, 2021 4:32 PM

To: BRACKLEY Kevin <Kevin.BRACKLEY@HydroOne.com>; SZYMCZAK Sarah <Sarah.Szymczak@HydroOne.com>

Cc: Matt Brace <mbrace@tathameng.com>

Subject: 420427 - Melville Court Development - Utility Inquiry

Good afternoon Kevin and Sarah,

I work with Nick Millington at Tatham Engineering who passed along your contact information.

We are working on behalf of Doncor Developments Inc. in support of official plan amendment, zoning by-law amendment, and draft plan of subdivision applications for a development project located on the north side of Melville Court in Oro-Medonte, near Highway 11 and Line 5 North. The project proposes to develop existing agricultural lands by constructing 16 new residences (site plan attached). Currently, only the south side of Melville Court has existing residential units.

We are looking for preliminary comment regarding feasibility of hydro servicing for the site to allow us to include commentary regarding such in our Functional Servicing Report.

If you could kindly review and provide comment at your earliest convenience it would be greatly appreciated.

Thanks,

John-Lui Marra, B.Eng.

Engineering Intern

Tatham Engineering Limited

41 King Street, Unit 4 | Barrie | Ontario | L4N 6B5

T 705-733-9037 x2184 | C 647-404-2070 | jlmarra@tathameng.com | tathameng.com



Enhancing our communities

This email may contain confidential and/or privileged information for the sole use of the intended recipient. Any review or distribution by others is strictly prohibited. If you have received this email in error, please contact the sender and delete all copies.

Tatham Engineering Limited's agreement to transfer digital documents electronically or otherwise is made under the following conditions:

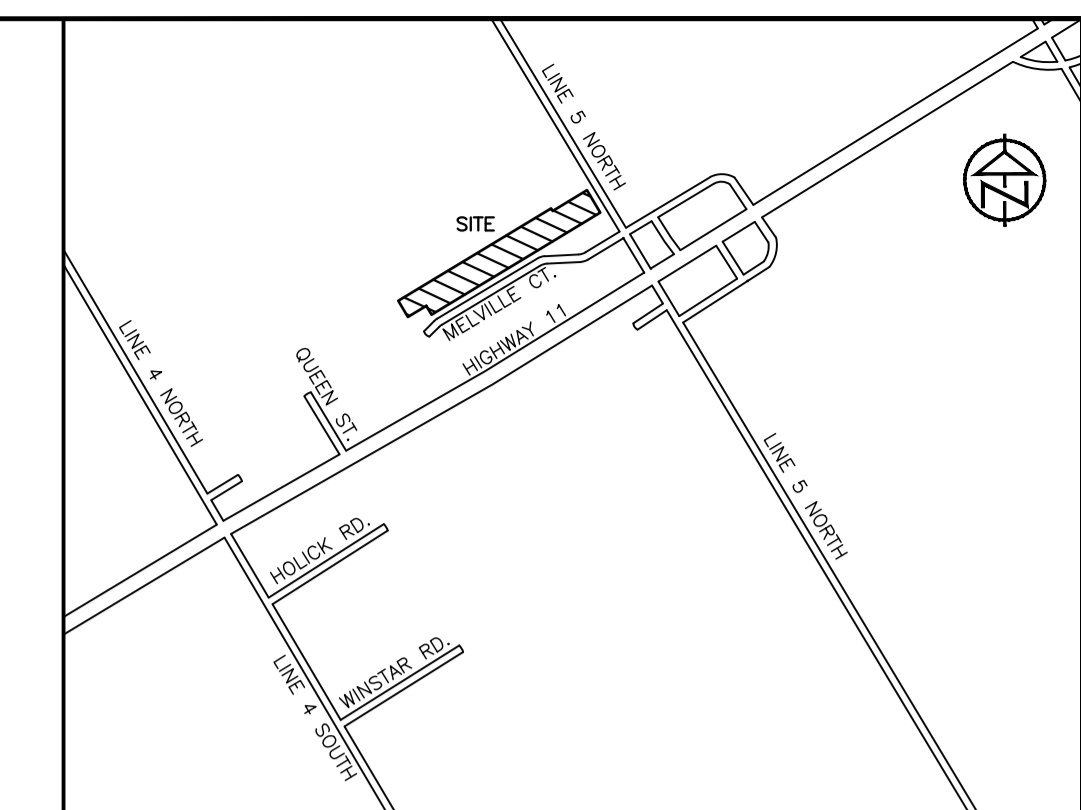
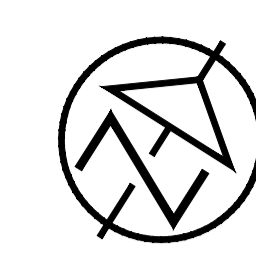
1. Electronic documents made available by Tatham Engineering Limited are supplied for the recipient's use only under authorization from the current owner and with the consent of Tatham Engineering Limited. It is the responsibility of the recipient to determine the accuracy, completeness and the appropriateness of the information provided.

2. It is agreed that only those hard copy documents bearing the professional seal and signature of the Tatham Engineering Limited project engineer will govern the work of the project. In the event of any dispute concerning an electronic document, the appropriately dated hard copy will be the document used by Tatham Engineering Limited to govern and resolve the dispute.

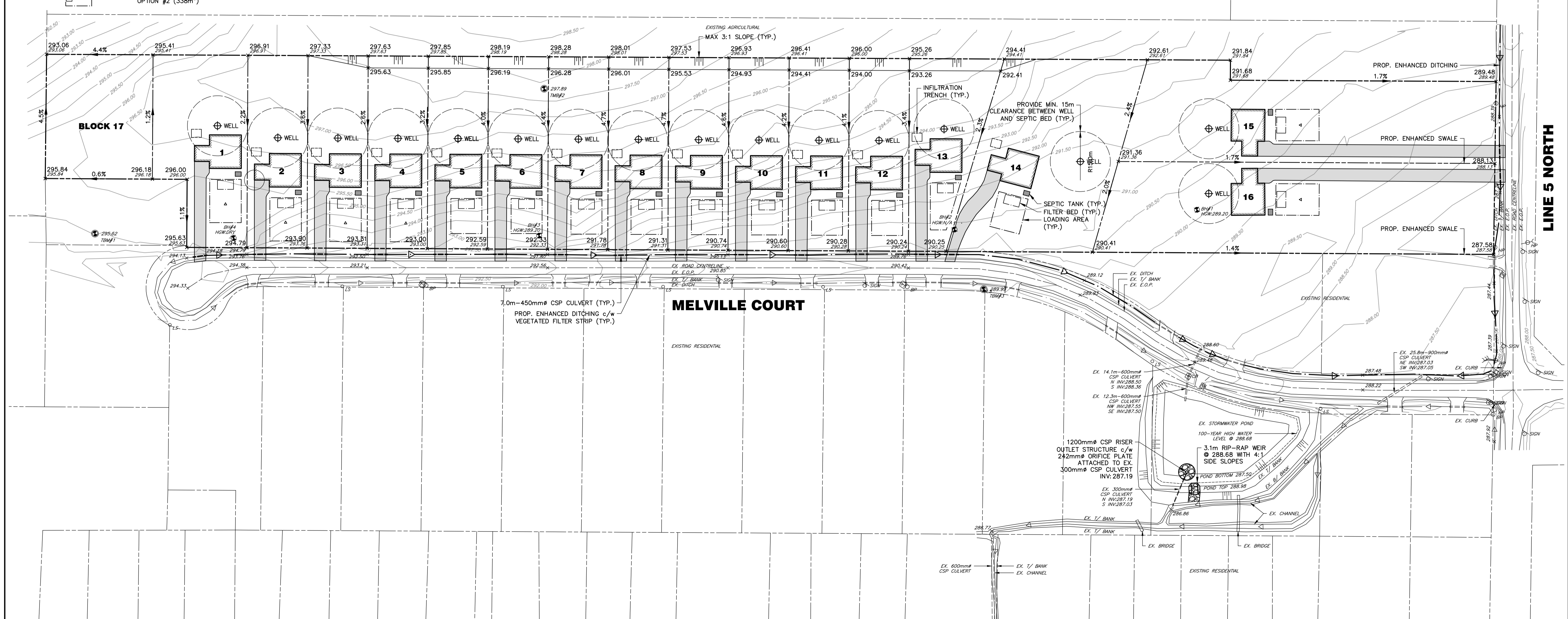
Appendix F: Drawings

LEGEND

- EXISTING PROPERTY LINE
- EXISTING ROAD CENTRELINE
- EXISTING EDGE OF PAVEMENT (E.O.P.)
- EXISTING ELEVATION CONTOURS (0.50m INTERVALS)
- EXISTING DITCH CENTRELINE
- EXISTING CULVERT
- PROPOSED PROPERTY LINE
- PROPOSED LOT LINE
- PROPOSED HOUSE
- PROPOSED DITCH CENTRELINE
- PROPOSED CULVERT
- PROPOSED SEPTIC BED LIMIT
- EXISTING SPOT ELEVATION (m)
- EXISTING CATCH BASIN
- EXISTING MAILBOX
- EXISTING HYDRO TRANSFORMER
- EXISTING HYDRO POLE
- EXISTING BELL POLE
- EXISTING LIGHT STANDARD
- EXISTING ROAD SIGN
- EXISTING BOREHOLE: HIGH GROUNDWATER LEVEL (m)
- TEMPORARY BENCHMARK
- PROPOSED SPOT ELEVATION (m)
- PROPOSED SLOPE
- PROPOSED WELL
- PROPOSED 6000L SEPTIC TANK
- PROPOSED DRIVEWAY
- PROPOSED INFILTRATION TRENCH (20.5m³)
- PROPOSED SEPTIC SYSTEM OPTION #1 (270m³)
- PROPOSED SEPTIC SYSTEM OPTION #2 (338m³)



KEY PLAN
NTS

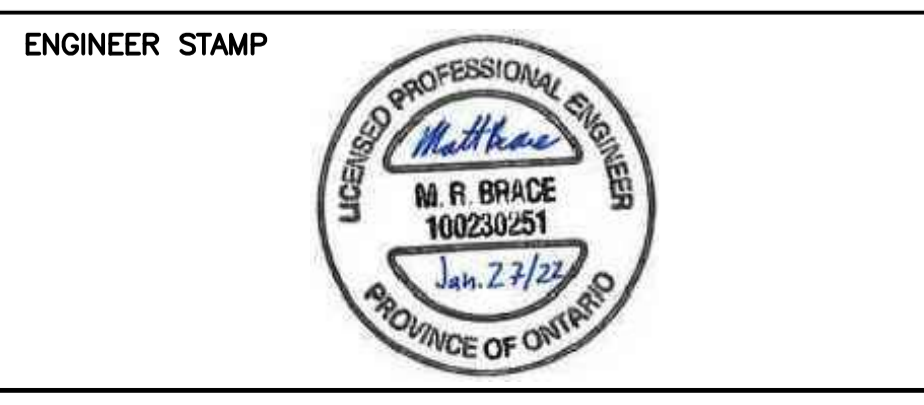


DISCLAIMER AND COPYRIGHT
CONTRACTOR MUST VERIFY ALL DIMENSIONS AND BE RESPONSIBLE FOR SAME. ANY DISCREPANCIES MUST BE REPORTED TO THE ENGINEER BEFORE COMMENCING WORK. DRAWINGS ARE NOT TO BE SCALED.
TATHAM ENGINEERING LIMITED CLAIMS COPYRIGHT TO THIS DRAWING WHICH MAY NOT BE USED FOR ANY PURPOSE OTHER THAN THAT PROVIDED IN THE CONTRACT BETWEEN THE OWNER/CLIENT AND THE ENGINEER WITHOUT THE EXPRESS CONSENT OF TATHAM ENGINEERING LIMITED.

BENCHMARKS
TBM#1 - ELEVATION 295.52m
TOP OF IRON BAR, NORTH WEST OF SUBJECT SITE, LOCATED SOUTH OF PROPOSED BLOCK 17 AND WEST OF PROPOSED LOT 1.
TBM#2 - ELEVATION 297.89m
TOP OF IRON BAR, WITHIN SUBJECT SITE, LOCATED WITHIN PROPOSED LOT 6.
TBM#3 - ELEVATION 289.99m
TOP OF IRON BAR, SOUTH OF SUBJECT SITE, LOCATED SOUTH OF PROPOSED LOT 14, SOUTH SIDE OF ROAD.

NOTES
DRAFT PLAN PREPARED BY INNOVATIVE PLANNING SOLUTIONS; DATED NOVEMBER 25, 2021.
LEGAL SURVEY COMPLETED BY DEARDEN & STANTON LTD.; DATED JUNE 8, 2004.
TOPOGRAPHIC SURVEY FOR SWM POND AND SOUTH SIDE OF MELVILLE COURT COMPLETED BY C.C. TATHAM & ASSOCIATES LTD.; DATED JULY 2015.
TOPOGRAPHIC SURVEY FOR ROAD AND NORTH SIDE OF MELVILLE COURT COMPLETED BY JOETOPO; DATED OCTOBER 6, 2021.

| No. | REVISION DESCRIPTION | DATE | ENGINEER STAMP |
|-----|------------------------|-----------|----------------|
| 1. | FSR - FIRST SUBMISSION | JAN 27/22 | |



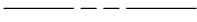

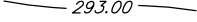

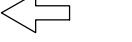

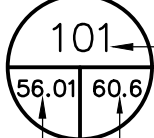
MELVILLE COURT DEVELOPMENT
TOWNSHIP OF ORO-MEDONTE

PRELIMINARY SITE SERVICING AND GRADING PLAN

TATHAM ENGINEERING

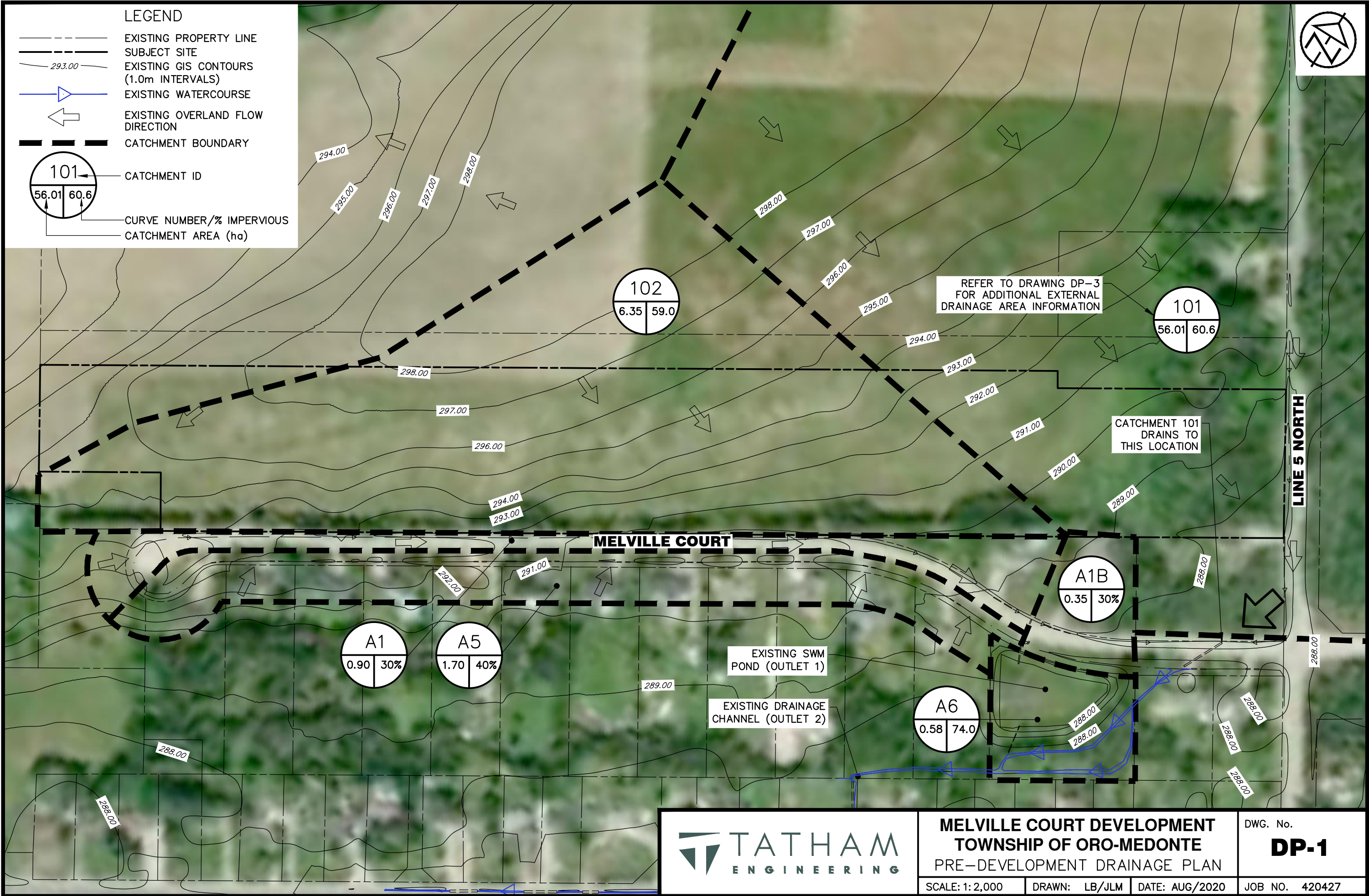
| | | |
|------------|----------------|--------------|
| DESIGN: MB | FILE: 420427 | DWG: |
| DRAWN: JLM | DATE: SEP 2021 | SSG-1 |
| CHECK: NM | SCALE: 1:1,000 | |

LEGEND

-  EXISTING PROPERTY LINE
 -  SUBJECT SITE
 -  EXISTING GIS CONTOURS (1.0m INTERVALS)
 -  EXISTING WATERCOURSE
 -  EXISTING OVERLAND FLOW DIRECTION
 -  CATCHMENT BOUNDARY
- 

CATCHMENT ID

CURVE NUMBER/% IMPERVIOUS CATCHMENT AREA (ha)

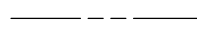

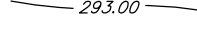

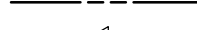




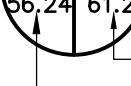


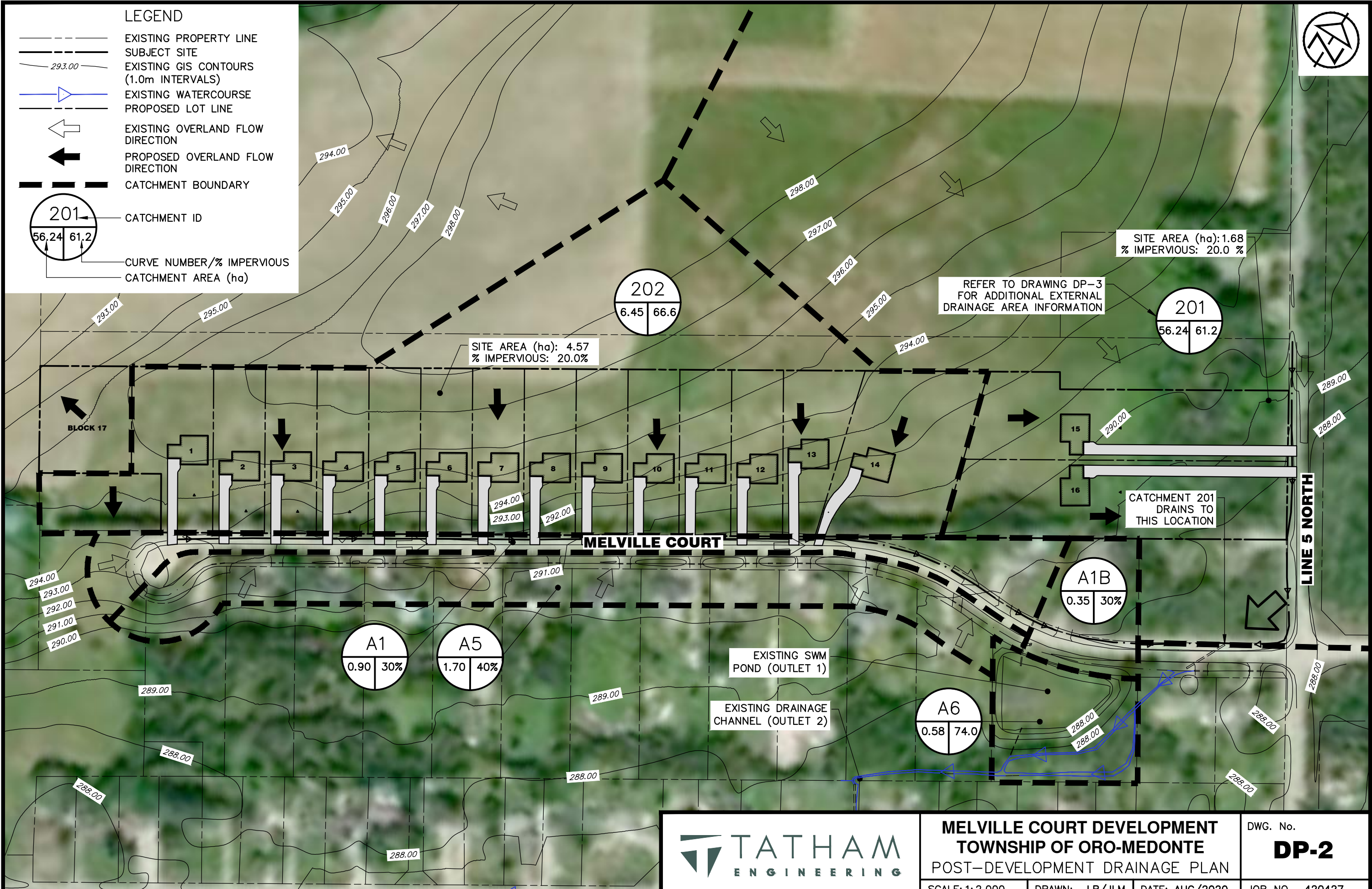
MELVILLE COURT DEVELOPMENT
TOWNSHIP OF ORO-MEDONTE
 PRE-DEVELOPMENT DRAINAGE PLAN

DWG. No.
DP-1

SCALE: 1: 2,000 | DRAWN: LB/JLM | DATE: AUG/2020 | JOB NO. 420427

LEGEND

-  EXISTING PROPERTY LINE
-  SUBJECT SITE
-  EXISTING GIS CONTOURS (1.0m INTERVALS)
-  EXISTING WATERCOURSE
-  PROPOSED LOT LINE
-  EXISTING OVERLAND FLOW DIRECTION
-  PROPOSED OVERLAND FLOW DIRECTION
-  CATCHMENT BOUNDARY
-  CATCHMENT ID
-  CURVE NUMBER/% IMPERVIOUS CATCHMENT AREA (ha)



SITE AREA (ha): 4.57
% IMPERVIOUS: 20.0%

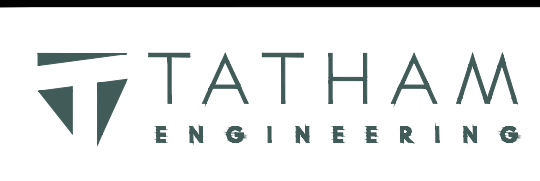
SITE AREA (ha): 1.68
% IMPERVIOUS: 20.0%

REFER TO DRAWING DP-3
FOR ADDITIONAL EXTERNAL
DRAINAGE AREA INFORMATION

CATCHMENT 201
DRAINS TO
THIS LOCATION

EXISTING SWM
POND (OUTLET 1)

EXISTING DRAINAGE
CHANNEL (OUTLET 2)



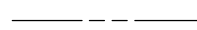
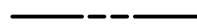
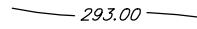


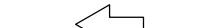
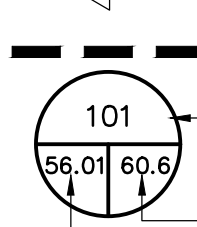
**MELVILLE COURT DEVELOPMENT
TOWNSHIP OF ORO-MEDONTE**
POST-DEVELOPMENT DRAINAGE PLAN

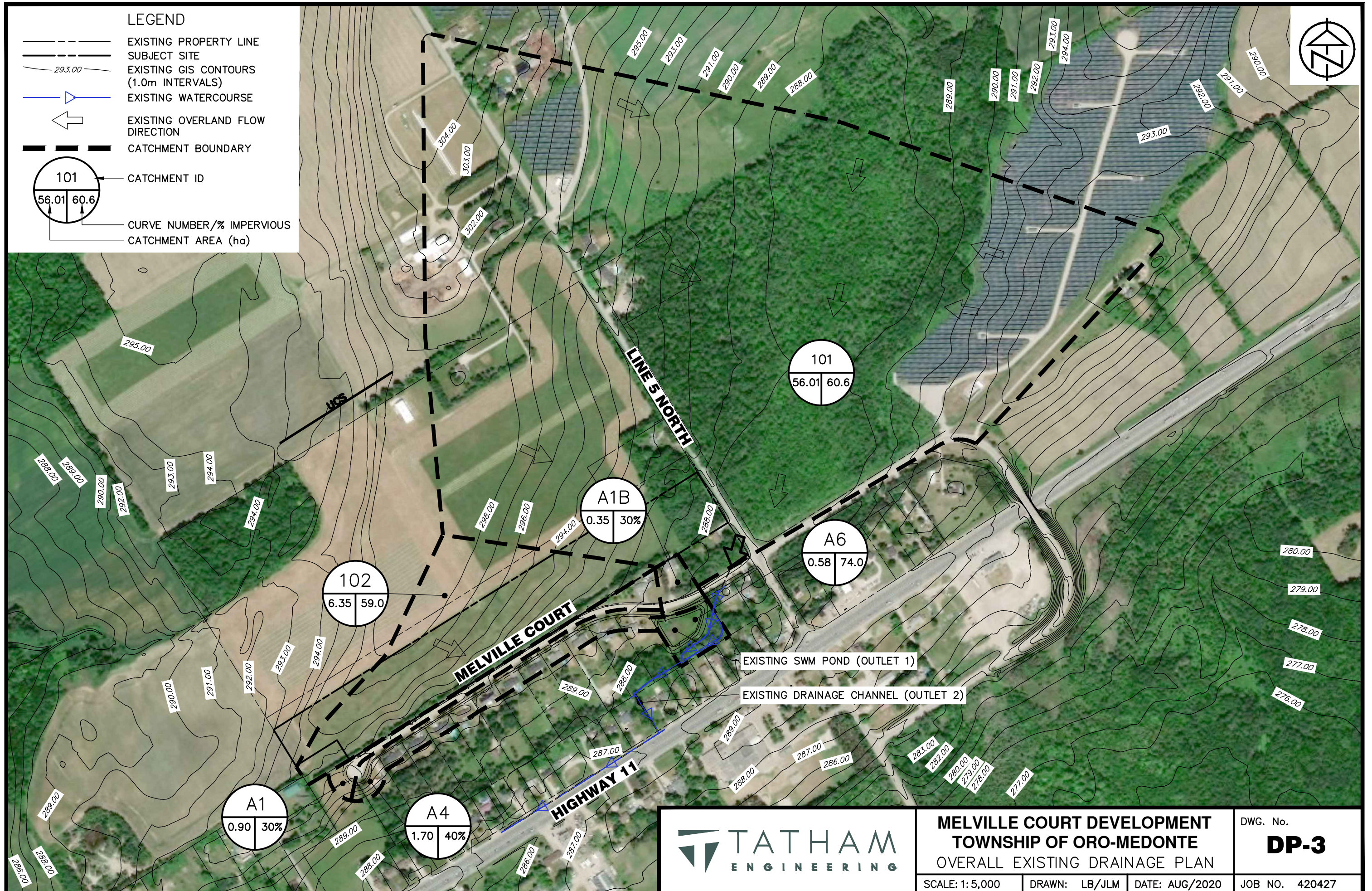
SCALE: 1: 2,000 DRAWN: LB/JLM DATE: AUG/2020

DWG. No.
DP-2

JOB NO. 420427

LEGEND

-  EXISTING PROPERTY LINE
-  SUBJECT SITE
-  EXISTING GIS CONTOURS (1.0m INTERVALS)
-  EXISTING WATERCOURSE
-  EXISTING OVERLAND FLOW DIRECTION
-  CATCHMENT BOUNDARY
-  CATCHMENT ID
CURVE NUMBER/% IMPERVIOUS
CATCHMENT AREA (ha)



**MELVILLE COURT DEVELOPMENT
TOWNSHIP OF ORO-MEDONTE
OVERALL EXISTING DRAINAGE PLAN**

SCALE: 1: 5,000 DRAWN: LB/JLM DATE: AUG/2020

DWG. No.
DP-3

JOB NO. 420427