

C.C. Tatham & Associates Ltd. Consulting Engineers

BURL'S CREEK EVENT GROUNDS Township of Oro-Medonte

Traffic Impact Study

prepared by:

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1 Introduction

C.C. Tatham & Associates was retained by Burl's Creek Event Grounds to address the traffic impacts associated with the proposed Event Grounds development, located in the Township of Oro-Medonte. The location of the development site and the immediate area road network is illustrated in Figure 1.

The purpose of this study is to address the requirements of the Township of Oro-Medonte, the County of Simcoe and the Ministry of Transportation of Ontario (MTO) with respect to the potential transportation impacts of the development on the respective road systems (Township, County and Provincial). In particular, the following will be discussed:

- the operations of the road system through the study area prior to the proposed development;
- an estimation of the growth in the traffic volumes not otherwise attributed to the development (ie. from overall growth in the area and/or other developments);
- an estimation of the number of new trips the proposed Event Grounds development is likely to generate (in consideration of various levels of events);
- the operations of the study area road system upon completion of the development; and
- the resulting impacts and need for mitigating measures (if required) to ensure acceptable overall road operations.

Chapter 2 of this report addresses the existing conditions, detailing the road system and corresponding traffic operations. Chapter 3 addresses future conditions, prior to the completion of the proposed development, and will address the expected growth in the traffic levels and the resulting operating conditions. Chapters 4 and 5 address the proposed development, the ensuing vehicle trips that it will generate, and the associated impacts on the road system. Lastly, Chapter 6 summarizes the report and the key findings.

Burl's Creek has proposed a number of events to occur over the 2015 season, ranging from locally based events (eg. minor soccer and farmers market), to regionally based events (festivals and automotive flea market) to provincially (and beyond) based events (Wayhome Arts & Music Festival and Boots & Hearts Music Festival). In consideration of the event durations and significant traffic volumes associated with the noted major music festivals, a separate traffic study has been prepared by a special events transportation consultant to address corresponding traffic operations, routings and protocols. The intent of this study is therefore to address the remaining local and regional based events and the associated traffic impacts of such (if any).

2 Existing Conditions

This chapter will describe the road network, traffic volumes and operations for the existing conditions.

2.1 Study Area Road Network

The road network to be addressed by this study consists of: Highway 11; Simcoe Road 20 (Ridge Road); Line 7; Line 8 and Line 9. Photographs of the road system are provided in Figure 2.

2.1.1 Road Sections

Highway 11 is a provincial highway under the jurisdiction of the MTO. While through the immediate study area, the highway is oriented south-west to north-east and vice versa; in consideration of the larger area and general orientation, Highway 11 is referenced as north-south. The highway has a posted speed of 90 km/h and provides 2 travel lanes per direction with a median (box beam guiderail) separation. Although a provincial highway, there are numerous commercial access points and municipal road intersections along the highway. Given the divided nature of the highway, the access points and intersections are limited to right-in/right-out movements with the exception of those at Line 3, Line 5, Line 7, Line 9 and Simcoe Road 20. At these locations, full access to Highway 11 North and Highway 11 South is provided through means of an overpass and connections to the highway on either side. The capacity of Highway 11 has been determined following MTO procedures for maximum service flow for a multi-lane highway, considering factors such as base capacity (2000 vehicles per hour per lane) and adjustment factors for lane width and/or lateral clearance restrictions, presence of heavy vehicles, development environment and driver population. Based on the MTO methodology, a maximum service flow of 3270 vehicles has been determined per direction, which equates to 1635 vehicles per hour per lane for the 2-lane highway.

Simcoe Road 20 (known locally as Ridge Road) is classified as a secondary arterial in the *County of Simcoe Transportation Master Plan*, and thus under the jurisdiction of the County. As per the Master Plan, County roads are facilities where traffic movement is the primary consideration while land access is a secondary function. In this respect, County roads are intended to serve higher traffic volumes and all vehicle types. Simcoe Road 20 provides 1 lane per direction, with a posted speed limit of 60 km/h, reduced to 50 km/h through the hamlets of Oro-Station (at Line 7) and Hawkestone (at Line 11). Typically, County roads are assumed to have capacities of 900 to 1100 vehicles per hour per lane. In consideration of the "secondary arterial" designation and the reduced speed limit (County roads are typically posted 80 km/h), a reduced capacity of 800 vehicles per hour per lane has been assumed (which is consistent with assumptions as per the *County of Simcoe Transportation Master Plan*).

Line 7, Line 8 and Line 9 are all Township of Oro-Medonte roads, providing 1 lane per direction with an assumed capacity of 600 vehicles per hour per lane (in consideration of their role as collector roads). As with Simcoe Road 20, these roads have reduced speed limits of 60 km/h through the study area.

2.1.2 Key Intersections

The key intersections are those of the noted road sections, which are detailed below and illustrated in Figure 3.

As previously noted, the intersections of Line 7 and Line 9 with Highway 11 provide full moves access to/from the highway via a right-in/right-out intersection (as the highway is median separated) and an overpass with 2 stop controlled intersections with the Township road. In all cases, single lane approaches are provided (ie. no dedicated turn lanes exist). The intersection of Line 8 with Highway 11 is restricted to right-in/right-out movements only, both of which are yield controlled and single lanes. At each intersection with the highway, there are acceleration and deceleration lanes to accommodate the right-in and right-out movements.

The intersection of Line 7 with Simcoe Road 20 is a 4-way stop controlled intersection, with each leg providing a single approach lane. This intersection is located in Oro Station.

The intersection of Line 8 with Simcoe Road 20 is a 3-legged intersection (Line 8 terminates at the County road) with stop control on Line 8. All approaches are limited to a single lane.

Lastly, the intersection of Line 9 with Simcoe Road 20 is a 4-legged intersection stop controlled on Line 9, with single lanes on each approach.

2.2 Existing Traffic Volumes

2.2.1 Link Volumes

Traffic volumes on the study area road sections were requested of the MTO, Simcoe County and Township; a summary of which is provided in Table 1. The corresponding daily traffic volumes are presented in Table 2 for spring (May), summer (July-August) and fall (October) conditions, whereas additional details are provided in Appendix A. No recent count data was available for Line 8 or Line 9.

Table 1: Av	ailable Tra	ffic Data
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Location	Source	Date	Туре
Line 5 to Line 6 (8.23km north of Simcoe Road 93)	МТО	July 2014	24 hour counts by hour
Line 3 to Line 7		May 2014	
Line 7 to Line 11	County	August 2014	24 hour counts by hour
Line 11 to Hwy 11		October 2014	
south of overpass	Township	May 2014	24 hour
	Line 5 to Line 6 (8.23km north of Simcoe Road 93) Line 3 to Line 7 Line 7 to Line 11 Line 11 to Hwy 11	Line 5 to Line 6 (8.23km north of MTO Simcoe Road 93) Line 3 to Line 7 Line 7 to Line 11 County Line 11 to Hwy 11	Line 5 to Line 6 (8.23km north of Simcoe Road 93)MTOJuly 2014Line 3 to Line 7May 2014Line 7 to Line 11CountyMay 2014 August 2014 October 2014Line 11 to Hwy 11

Location	Spring	Summer	Fall
Line 5 to Line 6	-	40,000 to 58,000	-
Line 3 to Line 7	1500 to 1700	1350 to 1500	1350 to 1500
Line 7 to Line 11	1000 to 1150	1000 to 1150	900 to 1000
Line 11 to Hwy 11	1950 to 2050	1700 to 1850	1800 to 1950
south of overpass	4100	-	-
	Line 5 to Line 6 Line 3 to Line 7 Line 7 to Line 11 Line 11 to Hwy 11	Line 5 to Line 6 - Line 3 to Line 7 1500 to 1700 Line 7 to Line 11 1000 to 1150 Line 11 to Hwy 11 1950 to 2050	Line 5 to Line 6 - 40,000 to 58,000 Line 3 to Line 7 1500 to 1700 1350 to 1500 Line 7 to Line 11 1000 to 1150 1000 to 1150 Line 11 to Hwy 11 1950 to 2050 1700 to 1850

Table 2: 2014 Daily Traffic Volumes

2.2.2 Intersection Volumes

Further to the link volumes provided by the road authorities, additional traffic counts were completed at a number of the key intersections to provide an indication of peak hour turning volumes. Traffic counts were completed on Thursday April 2, 2015 from 7AM to 10AM and 4PM to 7PM at the following intersections (details of which are provided in Appendix A):

- Line 7 & Highway 11 overpass (both intersections);
- Line 8 & Highway 11 (right-in/right/out);
- Line 9 & Highway 11 overpass (both intersections); and
- Line 7 & Simcoe Road 20.

A Thursday count is considered representative of typical weekday operations, whereas April volumes are typically considered representative of average conditions throughout the year (as is October). While counts were not completed at the Line 7 and Line 9 right-in/right-outs with Highway 11, the traffic volumes can be established from the counts at the adjacent overpass intersections in that there are generally no significant traffic generators or attractors between the intersections. The exception is the access to the Tim Hortons/gas station on Line 7 between Highway 11 and the west overpass intersection (this access was included in the traffic count and thus associated volumes established).

2.2.3 Seasonal Variations

The traffic data for Simcoe Road 20 was reviewed for seasonal variations, given that the counts were completed in the spring, summer and fall. The corresponding daily volumes were relatively consistent suggesting no significant increases in summer volumes on Simcoe Road 20. For example, for the road section from Line 3 to Line 7, the average daily volumes observed in the spring, summer and fall were 1634, 1429, 1424. For the section from Line 7 to Line 11, the spring, summer and fall volumes were 1088, 1046 and 977 respectively.

On Highway 11, a review of published MTO average and summer daily traffic volumes for the year 2010 (the most current published data available) suggests summer volumes are approximately 12% greater than average volumes (43,200 vs 39,000). Given the significance of Highway 11 in serving areas to the north (eg. "cottage country"), such an increase as compared to Simcoe Road 20 is expected (ie. similar increases on the local road system, which primarily serve the local residences, are not expected).

2.2.4 Daily Variations

The available data was also reviewed with respect to daily variations (namely weekday vs weekend). Counts for Simcoe Road 20 were typically conducted Tuesday to Thursday with little variation, and thus are considered typical weekday. Counts were not completed over the weekend and thus there is no basis for comparison along Simcoe Road 20.

The Highway 11 counts were conducted for a continuous week, with the daily variations outlined in Table 3. Based on the summer 2014 traffic data observed between Line 5 and Line 6, the following increases were observed:

- Friday peak hour volumes are approximately 5% greater than the average, with a more pronounced increase during the northbound direction (which corresponds to travel to cottage country);
- Saturday peak hour volumes are approximately 30% greater than the average, with a greater increase in the AM northbound direction (which corresponds to travel to cottage country); and
- Sunday peak hour volumes are approximately 15% greater, with greater increases in the AM and PM southbound direction (which corresponds to travel from cottage country).

Date	AM Pea	ık Hour	PM Pea	ik Hour
Dale	NB	SB	NB	SB
Average Weekday	1500 vph	1210 vph	1210 vph 1985 vph 140	
Friday	2515 vph	1245 vph	2790 vph	1505 vph
	67% higher	3% higher	41% higher	7% higher
Saturday	2490 vph	1565 vph	2450 vph	1620 vph
	66% higher	29% higher	23% higher	16% higher
Sunday	1735 vph	2190 vph	1695 vph	2580 vph
uph uphialas par	16% higher	81% higher	15% lower	84% higher

Table 3: 2014 Daily Traffic Variations on Highway 11

vph - vehicles per hour

2.2.5 Peak Hour Volumes

The peak hour volumes reflective of weekday AM and PM peak hour conditions (as per the April 2, 2015 traffic count), are illustrated in Figure 4. Traffic volumes on Highway 11 correspond to the noted average weekday conditions from the MTO 2014 count, increased by 1% per annum to reflect 2015 conditions (further discussion of the noted growth rate is provided in Section 3.2). As traffic counts were not completed at the intersections of Line 8 and Line 9 with Simcoe Road 20, they have been estimated based on the available data. The volumes on Line 7 and Line 9 just east of Highway 11 are comparable (as per the intersection counts at the east Highway 11 overpass intersections) and thus comparable volumes at Simcoe Road 20 have also been assumed (ie. the volumes at Line 9/Simcoe Road 20 have been estimated based on the Line 7/Simcoe Road 20 intersection volumes). Volumes at the Line 8/Simcoe Road 20 intersection were estimated based on the volumes on Line 8 as determined from the traffic count at the Highway 11 right-in/right-out. In most cases, the traffic volumes are not considered significant and thus if the assumptions are understated, the corresponding margin of error will not be critical.

In consideration of the event schedules planned for Burl's Creek, consideration has also been given to Friday, Saturday and Sunday traffic levels. Given the broad range of days and intervals considered, traffic counts corresponding to each were not completed. Rather, the volumes for the Friday, Saturday and Sunday periods have been estimated in consideration of the daily variations observed on Highway 11 (not considering the increases corresponding to the peak cottage country demands). Namely:

- Friday volumes are estimated as 5% greater than the weekday volumes (as per the traffic counts);
- Saturday volumes are estimated as 30% greater; and
- Sunday volumes are estimated as 15% greater.

Given the local nature of the area road system in serving the local development, such increases are not likely. Furthermore, the commuter peaks, as evident in the weekday data, would not be expected to be as significant over the weekends (travel over the weekends tends to be more distributed throughout the day, and not as pronounced during the early morning and late afternoon). Notwithstanding, the noted factors have been assumed to employ a conservative approach.

Traffic volumes on Highway 11 have been established for each of the noted days and peak periods based on the actual 2014 Highway 11 traffic counts, adjusted to reflect 2015 conditions.

The corresponding 2015 volumes are noted in Figure 5, Figure 6 and Figure 7 for the Friday, Saturday and Sunday peak hour periods.

2.3 Existing Traffic Operations

2.3.1 Intersection Operations

The assessment of existing conditions provides the baseline from which the future traffic volumes and operations (both with and without the subject development) can be assessed. The capacity, and hence operations, of a road system is effectively dictated by its intersections and thus the operational analysis considers the operations of the key intersections previously noted. The analysis is based on the 2015 Saturday traffic volumes, the existing intersection configuration and controls, and procedures outlined in the *2010 Highway Capacity Manual*¹ (using Synchro v.8 software). As the Saturday traffic volumes at the key intersections are greatest, their use in the assessment will represent the most conservative operational assessment. While it is acknowledged that travel patterns on Highway 11 vary between the Saturday and Sunday peak hours (on Saturday, the northbound volumes are greatest, whereas on Sunday the southbound volumes are greatest), these volumes have no bearing on the operations of the key intersections.

A summary of the 2015 Saturday peak hour traffic volumes analyses is provided in Table 4. Results of the operational assessment are provided in the form of average delay (measured in seconds), level of service (LOS) and volume to capacity (v/c) for the critical stop controlled movements. Level of service A corresponds to the best operating condition with minimal delays whereas level of service F corresponds to poor operations resulting from high intersection delays. A v/c ratio of less than 1.0 indicates the intersection movement/approach is operating at less than capacity while v/c of 1.0 indicates capacity has been reached. Detailed operations worksheets for the existing traffic conditions are included in Appendix B.

As indicated, all of the key study area intersections are currently providing excellent levels of service (level of service A or B) with minimal delays. In this respect, no improvements to the existing road system are considered necessary to support the existing peak hour traffic volumes. As the traffic operations under the peak Saturday conditions are considered acceptable, it can be inferred that operations under the weekday, Friday and Sunday conditions will also be acceptable recognizing that the intersection volumes will be less for these time periods.

¹ *Highway Capacity Manual.* Transportation Research Board, Washington DC, 2010.

Intersection and Movement		Control	AM Peak Hour			PM Peak Hour		
		Control	delay	LOS	v/c	delay	LOS	v/c
Line 7 & Hwy 11 Overpass W	SB	stop	1	А	0.01	4	А	0.02
Line 7 & Hwy 11 Overpass E	SB	stop	6	А	0.06	4	А	0.07
	EB		8	А	?	8	А	0.09
Line 7 &	WB	4-way stop	8	А	?	8	А	0.08
Simcoe Road 20	NB		9	А	?	8	А	0.11
	SB		8	А	?	8	А	0.20
Line 8 & Simcoe Road 20	SB	stop	8.9	А	0.02	9	А	0.02
Line 9 & Hwy 11 Overpass W	SB	stop	0	А	0.03	0	А	0.02
Line 9 & Hwy 11 Overpass E	SB	stop	0	А	0.03	0	А	0.06
Line 9 &	NB	stop	11	В	0.25	11	В	0.12
Simcoe Road 20	SB	stop	9	А	0.06	11	В	0.17

Table 4: Intersection Operations - 2015 Saturday

2.3.2 Road Section Operations

Further to the operations of the key intersections, the operations of the key road sections has also been investigated, based on the projected volumes and the following lane capacities (as previously presented):

- 1635 vehicles per hour per lane (vphpl) for Highway 11 (3270 vehicles per hour given 2 lanes per direction);
- 800 vphpl for Simcoe Road 20; and
- 600 vphpl for Line 7, Line 8 and Line 9.

A summary of the corresponding traffic volumes, road capacities and resulting volume to capacity (v/c) ratios is provided in Table 5. A v/c ratio of 1.0 or greater suggests that the road section is operating at or above capacity; v/c of less than 1.0 indicates that there is excess capacity available.

Road Section	& Period		ak Hour Imes		PM Peak Hour AM Peak Volumes Volume/Capac			PM Peak ty Volume/Capacity		
		NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	
	Friday	2,540	1,260	2,820	1,525	0.78	0.39	0.86	0.47	
Lliaburov 11	Saturday	2,520	1,585	2,475	1,640	0.77	0.48	0.76	0.50	
Highway 11	Sunday	1,755	2,215	1,715	2,605	0.54	0.68	0.52	0.80	
	Weekday	1,520	1,225	2,005	1,420	0.46	0.37	0.61	0.43	
	Friday	35	55	55	45	0.04	0.07	0.07	0.06	
Simcoe Road 20	Saturday	40	65	70	55	0.05	0.08	0.09	0.07	
(Line 3 to Line 7)	Sunday	35	55	60	50	0.04	0.07	0.08	0.06	
	Weekday	35	50	55	40	0.04	0.06	0.07	0.05	
	Friday	30	50	60	45	0.04	0.06	0.08	0.06	
Simcoe Road 20	Saturday	30	60	65	60	0.04	0.08	0.08	0.08	
(Line 7 to Line 11)	Sunday	30	50	60	55	0.04	0.06	0.08	0.07	
	Weekday	25	45	50	45	0.03	0.06	0.06	0.06	
	Friday	60	175	120	85	0.10	0.29	0.20	0.14	
Lino 7	Saturday	70	215	145	105	0.12	0.36	0.24	0.18	
Line 7	Sunday	65	190	130	90	0.11	0.32	0.22	0.15	
	Weekday	55	165	110	80	0.09	0.28	0.18	0.13	
	Friday	10	5	10	15	0.02	0.01	0.02	0.03	
Line 0	Saturday	10	10	10	15	0.02	0.02	0.02	0.03	
Line 8	Sunday	10	5	10	15	0.02	0.01	0.02	0.03	
	Weekday	10	5	5	15	0.02	0.01	0.01	0.03	
	Friday	30	140	90	60	0.05	0.23	0.15	0.10	
Line 0	Saturday	40	170	110	70	0.07	0.28	0.18	0.12	
Line 9	Sunday	30	155	100	65	0.05	0.26	0.17	0.11	
	Weekday	30	135	85	55	0.05	0.23	0.14	0.09	

Table 5: Road Section Operations - 2015

As noted in the table, the maximum v/c ratios across the time periods considered are as follows (excess capacity is denoted in brackets):

- Highway 11 0.86 Friday PM northbound (excess capacity for 450 vehicles remaining);
- Simcoe Road 20 0.09 Saturday PM northbound (730 vehicles);
- Line 7 0.36 Saturday AM westbound (385 vehicles);
- Line 8 0.03 all days (585 vehicles); and
- Line 9 0.28 Saturday AM westbound (430 vehicles).

In consideration of the above, the existing road system has sufficient capacity to accommodate existing travel demands.

3 Future Background Conditions

This chapter will describe the road network and background traffic volumes (ie. without Burl's Creek Event Grounds) expected for the years 2020 and 2025, in order to consider the longer-term impacts (5 and 10 years beyond opening).

3.1 Road Network

No planned improvements to the study area road network (apart from routine maintenance) have been identified and thus the road network as discussed in Section 2.1 will be maintained through all future horizons.

3.2 Background Traffic Volumes

Future background traffic volumes expected for the 2020 and 2025 horizon years have been determined based on the existing traffic volumes, historical and projected growth, and additional increases in volumes due to other development within the immediate area (apart from the subject development).

3.2.1 Background Growth

Historic traffic volumes on Highway 11 and Simcoe Road 20 through the study area were reviewed for the 10-year period 2000 through to 2010 for Highway 11 (the most current published MTO data) and for the period 2002 to 2014 for Simcoe Road 20 (corresponding data is provided in Appendix A); 10 year planning horizons have been employed to reflect more recent growth trends and changes in travel patterns and behaviours. The following annual growth rates were realized:

- 1.5% for average annual daily traffic and 0.1% for summer average daily traffic on Highway 11 (slightly higher growth rates of 2% and 1.1% were realized for the period 2000 to 2009); and
- 0.5 to 1.9% for daily traffic on Simcoe Road 20 (depending on road section).

To consider growth on the Township roads, population statistics have been reviewed. In 2011, the Township had a census population of 20,078, which represent an increase of 0.2% from the 2006 level of 20,031. On an annual basis, this translates to negligible growth over the same period. As per the *Simcoe County Transportation Master Plan*, the 2031 population is projected to be 28,100 which suggests an annual growth of 1.7% from 2011 to 2031 (which will largely be accommodated in the more developed, populated areas as opposed to the rural areas adjacent to the site).

In consideration of the historic growth in the area relating to both traffic (specifically the summer average daily traffic volumes in that the summer months are the subject of this study) and population,

and future growth projections for the Township, the annual growth rates as noted in Table 6 have been assumed. The resulting overall growth through to the 2020 and 2025 horizon years are also noted.

Road	Annual Growth	Gro	wth in Horizon Peri	od ¹
KUdu	Rate	2014 to 2015	2015 to 2020	2015 to 2025
Highway 11	1%	1%	5%	10%
Simcoe Road 20	2%	-	10%	22%
Line 7	1%	-	5%	10%
Line 8	1%	-	5%	10%
Line 9	1%	-	5%	10%

Table 6: Background Growth

note: compound growth has been assumed

3.2.2 Background Development

No other significant developments in the immediate area were identified by the Township for consideration in the background traffic volumes.

3.2.3 Traffic Volumes

The corresponding 2020 and 2025 background traffic volumes, as derived from the 2015 traffic counts and applying the noted adjustment and growth factors, are provided in Figure 8 through Figure 15.

3.3 Background Traffic Operations

3.3.1 Intersection Operations

The key intersections were again analysed given the projected background volumes, the results of which are provided in Table 7 for the critical 2025 Saturday peak hours. As the 2025 Saturday traffic volumes through the key intersections are greatest, this represents the worst case scenario. Provided the intersections operate with acceptable levels of service under 2025 Saturday operations, so too will they under 2020 conditions. Corresponding detailed worksheets are provided in Appendix C.

As indicated, despite the increase in background traffic volumes assumed, all of the study area key intersections will continue to provide excellent levels of service (level of service B or better) with minimal delays. In this regard, the existing road and intersection configurations are considered appropriate to serve the future background traffic volumes.

Intersection and Movement		Control	AM Peak Hour			PM Peak Hour		
		Control	delay	LOS	v/c	delay	LOS	v/c
Line 7 & Hwy 11 Overpass W	SB	stop	12	В	0.38	11	В	0.24
Line 7 & Hwy 11 Overpass E	SB	stop	11	В	0.17	13	В	0.18
	EB		8	А	0.08	8	А	0.07
Line 7 &	WB	4-way stop	8	А	0.11	8	А	0.11
Simcoe Road 20	NB		9	А	0.29	8	А	0.13
	SB		8	А	0.08	9	А	0.23
Line 8 & Simcoe Road 20	SB	stop	9	А	0.04	9	А	0.04
Line 9 & Hwy 11 Overpass W	SB	stop	10	А	0.19	9	А	0.11
Line 9 & Hwy 11 Overpass E	SB	stop	10	В	0.06	10	А	0.10
Line 9 &	NB	stop	12	В	0.31	11	В	0.16
Simcoe Road 20	SB	stop	10	В	0.08	12	В	0.21

Table 7: Intersection Operations - 2025 Saturday

3.3.2 Road Section Operations

The resulting 2020 and 2025 background traffic volumes are presented in Table 8 and Table 9 respectively. As with the 2015 conditions, the volume to capacity ratios (ie. the extent to which the available road capacity will be consumed) has been determined. In all cases, the projected traffic volumes remain less than the corresponding traffic volumes (ie. v/c < 1.0), thus suggesting despite the assumed annual growth, there will remain excess capacity to accommodate additional traffic. Of particular interest are the County and Township roads, which will operate at less than 12% and 45% of their respective capacities.

Road Section	& Period		ak Hour Imes		ak Hour Imes		Peak Capacity		Peak Capacity
		NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB
	Friday	2,685	1,335	2,980	1,610	0.82	0.41	0.91	0.49
Highway 11	Saturday	2,660	1,675	2,615	1,730	0.81	0.51	0.80	0.53
Highway 11	Sunday	1,855	2,340	1,815	2,745	0.57	0.72	0.56	0.84
	Weekday	1,695	1,295	2,240	1,580	0.52	0.40	0.69	0.48
	Friday	50	70	70	60	0.06	0.09	0.09	0.08
Simcoe Road 20	Saturday	55	80	85	70	0.07	0.10	0.11	0.09
(Line 3 to Line 7)	Sunday	50	70	75	65	0.06	0.09	0.09	0.08
	Weekday	50	65	70	55	0.06	0.08	0.09	0.07
	Friday	45	65	75	60	0.06	0.08	0.09	0.08
Simcoe Road 20	Saturday	45	75	80	75	0.06	0.09	0.10	0.09
(Line 7 to Line 11)	Sunday	45	65	75	70	0.06	0.08	0.09	0.09
	Weekday	40	60	65	60	0.05	0.08	0.08	0.08
	Friday	70	195	140	100	0.12	0.33	0.23	0.17
Line 7	Saturday	80	245	165	120	0.13	0.41	0.28	0.20
Line 7	Sunday	75	215	150	105	0.13	0.36	0.25	0.18
	Weekday	65	185	125	95	0.11	0.31	0.21	0.16
	Friday	15	10	15	20	0.03	0.02	0.03	0.03
	Saturday	15	15	15	20	0.03	0.03	0.03	0.03
Line 8	Sunday	15	10	15	20	0.03	0.02	0.03	0.03
	Weekday	15	10	10	20	0.03	0.02	0.02	0.03
	Friday	40	160	105	70	0.07	0.27	0.18	0.12
	Saturday	50	195	125	85	0.08	0.33	0.21	0.14
Line 9	Sunday	40	175	115	75	0.07	0.29	0.19	0.13
	Weekday	40	150	100	65	0.07	0.25	0.17	0.11

Table 8: Road Section Operations - 2020 Volumes

Road Section	& Period		ak Hour Imes		ik Hour Imes		Peak Capacity		Peak Capacity
Road Coolion		NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB
Highway 11	Friday	2,820	1,410	3,145	1,700	0.86	0.43	0.96	0.52
	Saturday	2,805	1,770	2,760	1,825	0.86	0.54	0.84	0.56
	Sunday	1,955	2,465	1,920	2,890	0.60	0.75	0.59	0.88
	Weekday	1,695	1,370	2,240	1,580	0.52	0.42	0.69	0.48
	Friday	50	75	75	65	0.06	0.09	0.09	0.08
Simcoe Road 20	Saturday	55	90	95	75	0.07	0.11	0.12	0.09
(Line 3 to Line 7)	Sunday	50	75	85	70	0.06	0.09	0.11	0.09
	Weekday	50	65	75	60	0.06	0.08	0.09	0.08
	Friday	45	70	80	65	0.06	0.09	0.10	0.08
Simcoe Road 20	Saturday	45	80	85	80	0.06	0.10	0.11	0.10
(Line 7 to Line 11)	Sunday	45	70	80	75	0.06	0.09	0.10	0.09
	Weekday	40	60	70	65	0.05	0.08	0.09	0.08
	Friday	75	215	155	110	0.13	0.36	0.26	0.18
line 7	Saturday	90	270	180	130	0.15	0.45	0.30	0.22
Line 7	Sunday	85	235	165	115	0.14	0.39	0.28	0.19
	Weekday	70	205	135	105	0.12	0.34	0.23	0.18
	Friday	15	10	15	20	0.03	0.02	0.03	0.03
	Saturday	15	15	15	20	0.03	0.03	0.03	0.03
Line 8	Sunday	15	10	15	20	0.03	0.02	0.03	0.03
	Weekday	15	10	10	20	0.03	0.02	0.02	0.03
	Friday	40	175	115	75	0.07	0.29	0.19	0.13
Line O	Saturday	50	215	135	90	0.08	0.36	0.23	0.15
Line 9	Sunday	40	190	125	80	0.07	0.32	0.21	0.13
	Weekday	40	170	110	70	0.07	0.28	0.18	0.12

Table 9: Road Section Operations - 2025 Volumes

4 Burl's Creek Event Grounds

This chapter will provide additional details with respect to the proposed development, including its location, the projected site generated traffic volumes and the assignment of such to the adjacent road network.

4.1 **Proposed Use**

The Burl's Creek Event Grounds is planned to be a premier event venue within Simcoe County. A range of events are to be hosted through the year including, but not limited to:

- music events;
- flea markets;
- camping and arts festivals;
- Oro-Medonte minor soccer; and
- a farmers market.

A site plan is provided in Figure 16.

4.2 Location & Access

As illustrated in Figure 1, Burl's Creek Events Ground is situated between Line 7 and Line 9, on the east side (water side) of Highway 11.

As per the site plan of Figure 16, access to the site is provided via each of Line 7, Line 8 (several access points) and Line 9. In all cases, the access points will provide for a single entry lane and single exit lane. Access points on Line 7 and Line 8 currently exist and served previous site uses; the access on Line 9 is proposed. The actual use of the access points will be dictated by the on-site event and location of activities within the site. For the larger musical events, control gates will be implemented immediately within the site boundaries at each of the Line 7, Line 8 and Line 9 access points - the single lane entry will flare to 18 control gates at Line 7, 9 at Line 8 (for VIP and RV access only) and 16 at Line 9.

The distances between the centre of Highway 11 and the centre of the access points are as follows:

- 830 metres for the Line 7 access point;
- 115, 465 and 1040 metres for the Line 8 access points;
- 1400 metres for the Line 9 access point.

4.3 Scheduled Events

A calendar of events planned for the year 2015 is provided in Appendix D, spanning from May to mid-October. For purposes of this assessment, 3 levels of events have been established as described below.

4.3.1 Level 1 Events

Level 1 events are more local in nature drawing from the Township of Oro-Medonte and immediately surrounding areas. They are expected to have shorter durations and/or fewer participants, and thus will generate fewer vehicle trips. These events include the following, with the noted number of occurrences scheduled in 2015:

- Huronia Fur & Feathers (1 Sunday morning event);
- Farmers Market (15 Friday events); and
- Oro-Medonte Minor Soccer (13 Wednesday evening events).

4.3.2 Level 2 Events

Level 2 events are intended to draw from a more significant regional market, resulting in greater attendance and associated traffic levels. Events considered as Level 2 include:

- Tough Mudder parking (1 Saturday event);
- Contemporary Music & Camping Festival (1 Saturday to Sunday event); and
- Automotive Flea Market (2 Thursday to Sunday events).

4.3.3 Level 3 Events

Level 3 events are the highest level of events to be staged at Burl's Creek. These events have broad market draw (provincial and beyond) and include:

- Wayhome Arts & Music Festival (1 Thursday to Sunday event); and
- Boots and Hearts Music Festival (1 Wednesday to Sunday event).
- 4.4 Event Traffic Trip Generation

Typically, estimates of site generated traffic volumes are based on the proposed land use, size of development and trip generation rates published in the ITE *Trip Generation Manual*. However, given the uniqueness of the proposed undertaking, applicable trip rates are not available. Rather trip generation estimates have been established in consideration of the specific events and operating

details associated with each, as provided by Burl's Creek Events Ground and/or as determined from the event organizers. Specific details pertaining to the derivation of the site traffic volumes for each event are detailed in the following sections.

4.4.1 Level 1 Events

Huronia Fur & Feathers

The Huronia Fur and Feathers event is oriented towards the buying, selling and/or trading of poultry, game birds, pigeons, waterfowl, rabbits, cavies, etc., as hosted by the Huronia Fur and Feather Breeders Association. Assumptions regarding the event operations are:

- auto occupancy of 1.5 people per vehicle to reflect the specialized nature of the event and propensity for single attendees; and
- on average, 20% of the daily trips will occur during each event hour (5 hour duration); assume 40% (2x average) of daily trips in the peak hour with equal inbound and outbound trips.

Schedule	Event Attendance	Vehicle Occupancy	Event	Peak Hour Vehicle Trips		
			Vehicles	Time	In	Out
Sunday (x1) 7AM to 12PM	500	1.5 per vehicle	333	mid-morning	133	133

Farmers Market

The Farmers Market is a multi-stall market at which local farmers/producers sell products and merchandise directly to the general public, such as fresh fruit and vegetables, meat products, and other associated merchandise. Assumptions regarding the event operations are:

- vendors will arrive at the start of the market and depart at the end of the market, but market patrons will arrive/depart throughout the event;
- auto occupancy of 1.5 people per vehicle to reflect the specialized nature of the event and propensity for single attendees (which is considered conservative in that many families attend farmers markets); and
- on average, 11% of the daily trips will occur during each event hour (9 hour duration); assume 25% (2x average) of daily trips in the peak hour with equal inbound and outbound trips

Schedule	Event	Vehicle	Event	Peak Hour Vehicle Trips		
	Attendance	Occupancy	Vehicles	Time	In	Out
Friday (x15) 12PM to 9PM	500	1.5 per vehicle	333	mid-event	83	83

Oro-Medonte Minor Soccer

Assumptions regarding the soccer matches are:

- as per the Oro-Medonte Minor Soccer website, athletes will train from 6:15PM to 6:40PM with a game to follow;
- younger athletes will end at 7:30PM or 7:40PM, whereas the older athletes will end at 8:00PM;
- auto occupancy of 1.2 athletes per vehicle to reflect multiple athlete families recognizing that all athletes will play at the same time; and
- assume all athletes arrive during the initial event hour and depart during the final event hour with 10% of athletes being dropped-off and picked-up

Schedule	Event	Vehicle Occupancy	Event Vehicles	Peak Hour Vehicle Trips			
	Attendance				Time	In	Out
Wednesday	600 atblatas	1.2 athletes	400	start:	5:15 to 6:15PM	400	40
(x13) 6PM to 8PM	600 athletes	per vehicle	400 -	end:	7:30 to 8:30PM	40	400

4.4.2 Level 2 Events

Tough Mudder

Tough Mudder is an endurance event series in which participants attempt a military-style obstacle course. The event will be hosted at Mount St. Louis Moonstone, but Burl's Creek Events Ground will serve as the event parking facility with shuttle services provided to Mount St. Louis Moonstone. In considering the associated traffic volumes, the following have been assumed:

- 2000 vehicles to be parked;
- arrivals to the parking area are expected to occur between 5AM and 1PM (8 hours); departures are to occur between 11AM and 7PM (8 hours); and
- on average, 12.5% of the daily trips will occur during each arrival or departure hour (8 hour durations); assume 25% (2x average) of daily trips in the peak hour for peak direction

Schedule	Event	Vehicle	Event	Peak Hour Vehicle Trips			
Schedule	Attendance	Occupancy	Vehicles		Time	In	Out
) ()	2000 cars	nla	2000 per _ day	arrive:	mid-morning	500	250
	per day	n/a		depart:	mid-afternoon	250	500

Contemporary Music & Camping Festival

Assumptions regarding the event operations are:

- weekend event with 50% of participants to arrive on the Saturday and 50% to arrive on the Sunday;
- auto occupancy of 2.5 people per vehicle;
- 25% of daily participants are assumed to arrive during the 1 hour period prior to the start of the event with the remaining arriving during the course of the event; and
- 50% of participants are assumed to depart during the 1 hour period following the end of the event with the remaining 50% to camp overnight and depart the next morning (over the course of the morning).

Schedule	Event	Vehicle Occupancy	Event Vehicles	Peak Hour Vehicle Trips			
	Attendance				Time	In	Out
Sat to Sun (x1)	8000	2.5 per	3200 per	Sat:	11AM to 12PM	400	-
12PM to 11PM	8000	vehicle	day	Sun:	11PM to 12AM	-	1600

With respect to the auto occupancy, data has been compiled by Burl's Creek from 12 similar events as noted below, including the 2014 Boots & Hearts festival.

- Bonnaroo (2009 to 2013) 2.56 persons per vehicle
- Life is Good (2010) 2.98
- Electric Forest (2013) 2.48
- Rothbury (2008-2009) 2.52
- Jimmy Buffet (2010) 2.73
- All Good (2011) 2.43
- Boots & Hearts (2014) 2.66

The above provide a range of auto occupancies from 2.43 to 2.98 people per vehicle with an average of 2.62. For this assessment, the occupancy of 2.5 has been assumed to provide a conservative assessment.

Barrie Automotive Flea Market

The Barrie Automotive Flea Market is an event oriented to collectibles, classic cars and automotive parts. There are also automobile auctions, shows and other associated events. Assumptions regarding the event operations are:

- auto occupancy of 2.5 persons per car to reflect the greater number of attendees, and varied nature and target markets for the event;
- vendors will arrive at the start of the flea market and depart at the end of the flea market, but patrons will arrive/depart throughout the event;
- some vendors and/or patrons will camp overnight on site; and
- on average, 8% of the daily trips will occur during each event hour (12 hour duration per day); assume 15% (2x average) of daily trips in the peak hour with equal inbound and outbound trips.

Schedule	Event	Vehicle	Event	Peak Hour Vehicle Trips		
Schedule	Attendance	Occupancy	Vehicles	Time	In	Out
Thurs to Sun (x2) 7AM to 7PM	10,000 per day	2.5 per vehicle	4000 per day	mid-event	600	600

4.4.3 Level 3 Events

The Level 3 events are major music festivals, namely Wayhome Arts & Music Festival and Boots & Hearts Music Festival.

Wayhome Arts & Music Festival

Assumptions regarding the event operations are:

- auto occupancy of 2.5 people per vehicle as per the previous music festival discussion (refer to the Level 2 event Contemporary Music & Camping Festival);
- vehicle arrivals and departures as per past events, corresponding data for which is provided in Figure 17;
- arrivals are to begin on Thursday with peak arrivals late morning/early afternoon Friday (10:00AM to 1:00PM as per past events); and
- 50% of participants are assumed to depart at the conclusion of the event (likely over several hours), with the remaining 50% to camp overnight and depart the next morning (over the course of the morning).

Schedule	Event	Vehicle	Event	Peak Hour Vehicle Trips			
Schedule	Attendance	Occupancy	Vehicles		Time	In	Out
Thurs to Sun (x1)	40,000	2.5 per vehicle	16,000 -	arrive:	late morn/early aft Friday	1200	-
	40,000			depart:	Sun night	-	2400

Boots & Hearts Music Festival

Assumptions regarding the event operations, as considered in the above, are:

- auto occupancy of 2.5 people per vehicle as per the previous music festival discussion (refer to the Level 2 event Contemporary Music & Camping Festival);
- vehicle arrivals and departures as per past events, corresponding data for which is provided in Figure 18;
- arrivals are to begin on Wednesday with peak arrivals mid-day Friday (as per past events); and
- 50% of participants are assumed to depart at the conclusion of the event (likely over several hours), with the remaining 50% to camp overnight and depart the next morning (over the course of the morning).

Schedule	Event	Vehicle	Event	Peak Hour Vehicle Trips			
Schedule	Attendance	Occupancy	Vehicles		Time	In	Out
Wed to Sun (x1)	40,000	2.5 per vehicle	16,000 -	arrive:	late morn/early aft Friday	900	-
	40,000			depart:	Sun night	-	2400

4.5 Event Traffic - Trip Distribution & Assignment

This section will detail the distribution of trips to surrounding areas and assignment of such to the road system.

4.5.1 Level 1 Events

For the Farmers Market and Oro-Medonte Minor Soccer, the visitors to Burl's Creek Event Grounds are expected to be from the immediately local area (ie. within the Township primarily). A slightly larger market area (encompassing the Cities and Orillia and Barrie, and other neighbouring Towns) is expected for the Huronia Fur and Feathers.

The following trip distributions have been assumed (based on the location of the Burl's Creek Event Grounds in relation to the anticipated markets and means of access to such):

		Farmers Market Minor Soccer	Huronia Fur and Feathers
•	to/from Highway 11 south	30%	50%
•	to/from Highway 11 north	30%	30%
•	to/from Lines 7, 8 & 9 west	30%	10%
·	to/from Simcoe Road 20 south	5%	5%
·	to/from Simcoe Road 20 north	5%	5%

4.5.2 Level 2 Events

The assumed distribution for the Level 2 events is as follows:

		Contemporary Music & Camping Festival Automotive Flea Market	Tough Mudder
•	to/from Highway 11 south	50%	70%
•	to/from Highway 11 north	30%	20%
•	to/from Lines 7, 8 & 9 west	10%	10%
•	to/from Simcoe Road 20 south	5%	0%
·	to/from Simcoe Road 20 north	5%	0%

As evident, it is assumed that the Tough Mudder will have a greater draw to areas to the south (eg. York Region and the GTA) and less of a draw from the local areas.

4.5.3 Level 3 Events

As previously noted, Level 3 events are considered major events and will generate significant traffic volumes. Given the limited occurrence of such, and the traffic volumes to be accommodated, separate traffic control plans have been established for the Wayhome Arts & Music Festival and the Boots & Hearts Music Festival, the intent of which is to establish transportation strategies to best accommodate the demands and minimize impacts through temporary traffic control measures. In this regard, these events have not been considered further in this assessment.

4.5.4 Traffic Volumes

The resulting assignments of the event related traffic volumes to the area road system, following the previously noted distributions, are presented in Figure 19 through Figure 24. The assignment of the site trips to the site access points recognizes the location of each event within the event grounds as per information provided by Burl's Creek Event Grounds (the information is noted on the event calendar and corresponding zone map in Appendix D). For those events with defined inbound and outbound peaks (ie. patrons will arrive at the start of the event and leave at the end), volumes have been illustrated separately.

5 Transportation Impacts

This chapter will address the resulting impacts of the proposed Burl's Creek Event Grounds development on the adjacent road system.

5.1 Traffic Volumes with Burl's Creek Traffic

The site generated traffic volumes for each individual event have been combined with the future 2015, 2020 and 2025 background traffic volumes for the corresponding day (eg. Friday, Saturday, Sunday or weekday) and time period (AM or PM) to realize the future total traffic volumes. For events over multiple days, volumes for the most critical day have been established. The AM or PM peak period was selected to best correspond to the peak hour of the event - the AM peak was used for events that peak mid-morning, whereas the PM peak was used for events that peak mid-afternoon or evening. In most cases, this represents a conservative approach in that the daily peaks (particularly on weekdays) correspond more closely to the start and end of the work day, volumes outside of these periods (during which the events are expected to peak) are generally less.

The corresponding traffic volumes are provided in Figure 25 through Figure 42; derivations of the future total traffic volumes (ie. background traffic volumes + site traffic volumes = total volumes) are provided for each event in Appendix E. Only data corresponding to the peak hour deemed most applicable to the event operations has been illustrated (eg. the Huronia Fur & Feathers event will occur during the morning, and thus only the AM peak hour has been considered as the event will not generate any significant traffic during the PM peak hour, if any at all).

5.2 Traffic Operations with Burl's Creek Traffic

5.2.1 Intersection Operations

The key intersections were again analyzed to establish potential impacts associated with the Burl's Creek Event Grounds. In consideration of the number of events and horizon years, the analysis has focussed on the 2025 horizon, recognizing that the associated volumes are greatest. Provided the 2025 traffic operations are acceptable, so too will the 2015 and 2020 operations be acceptable. The intermediate horizon years of 2015 and 2020 were only considered if significant operational impacts have been identified and further details as to related timings was considered necessary.

The results of the assessments are summarized in Table 10 through Table 15 for each specific event and considering the associated day and time of the event peak hours (eg. weekday evening vs Saturday morning). The corresponding detailed worksheets are provided in Appendix F.

Intersection and Movement		Control	AM Peak Hour			PM Peak Hour		
		CONTINU	delay	LOS	v/c	delay	LOS	v/c
Line 7 & Hwy 11 Overpass W	WB	stop	14	В	0.48	-	-	-
Line 7 & Hwy 11 Overpass E	WB	stop	16	С	0.34	-	-	-
	EB		8	А	0.08	-	-	-
Line 7 &	WB	4-way	8	А	0.11	-	-	-
Simcoe Road 20	NB	stop	9	А	0.24	-	-	-
	SB		8	А	0.10	-	-	-
Line 8 & Simcoe Road 20	SB	stop	9	А	0.04	-	-	-
Line 9 & Hwy 11 Overpass W	EB	stop	9	А	0.17	-	-	-
Line 9 & Hwy 11 Overpass E	EB	stop	10	В	0.05	-	-	-
Line 9 &	NB	stop	12	В	0.28	-	-	-
Simcoe Road 20	SB	stop	11	В	0.08	-	-	-
Line 7 & Site Access	WB	stop	5	В	0.19	-	-	-
Line 8 & Site Access	EB	stop	0	А	0	-	-	-
	WB	stop	0	А	0	-	-	-
Line 9 & Site Access	EB	stop	0	А	0	-	-	-

Table 10: Intersection Operations - 2025 Huronia Fur & Feathers (Sunday mid-morning)

Under the Huronia Fur and Feathers event, all intersection operations are considered excellent (levels of service A or B) with minimal delays. As such, no road system improvements are considered necessary to support this event, as based on the noted traffic operations.

Intersection and Movement		Control	AM Peak Hour			PM Peak Hour			
		Control	delay	LOS	v/c	delay	LOS	v/c	
Line 7 & Hwy 11 Overpass W	WB	stop	-	-	-	10	В	0.20	
Line 7 & Hwy 11 Overpass E	WB	stop	-	-	-	11	В	0.27	
	EB		-	-	-	8	А	0.11	
Line 7 &	WB	4-way	-	-	-	8	А	0.10	
Simcoe Road 20	NB	stop	-	-	-	8	А	0.12	
	SB		-	-	-	9	А	0.19	
Line 8 & Simcoe Road 20	SB	stop	-	-	-	9	А	0.05	
Line 9 & Hwy 11 Overpass W	EB	stop	-	-	-	9	А	0.15	
Line 9 & Hwy 11 Overpass E	EB	stop	-	-	-	10	А	0.08	
Line 9 &	NB	stop	-	-	-	11	В	0.14	
Simcoe Road 20	SB	stop	-	-	-	11	В	0.18	
Line 7 & Site Access	WB	stop	-	-	-	0	А	0.19	
Line 8 & Site Access	EB	stop	-	-	-	10	А	0.10	
	WB	stop	-	-	-	0	А	0	
Line 9 & Site Access	EB	stop	-	-	-	0	A	А	

Table 11: Intersection Operations - 2025 Farmers Market (Friday Evening)

Under the Farmers Market event, all intersection operations are considered excellent (levels of service A or B) with minimal delays. As such, no road system improvements are considered necessary to support this event, as based on the noted traffic operations.

Intersection and Movement		Control	Inbound Peak Hour			Outbound Peak Hour		
		CONTROL	delay	LOS	v/c	delay	LOS	v/c
Line 7 & Hwy 11 Overpass W	WB	stop	16	С	0.37	14	В	0.56
Line 7 & Hwy 11 Overpass E	WB	stop	60	F	0.93	20	С	0.32
	EB		8	А	0.14	8	А	0.11
Line 7 &	WB	4-way	8	А	0.12	8	А	0.10
Simcoe Road 20	NB	stop	8	А	0.11	8	А	0.11
	SB		9	А	0.20	9	А	0.24
Line 8 & Simcoe Road 20	SB	stop	1	А	0.01	1	А	0.01
Line 9 & Hwy 11 Overpass W	EB	stop	9	А	0.09	9	А	0.09
Line 9 & Hwy 11 Overpass E	EB	stop	9	А	0.08	9.4	А	0.08
Line 9 &	NB	stop	11	В	0.10	11	В	0.10
Simcoe Road 20	SB	stop	11	В	0.15	11	В	0.15
Line 7 & Site Access	WB	stop	12	В	0.10	15	В	0.59
Line 8 & Site Access	EB	stop	0	А	0.10	0	А	0.10
	WB	stop	0	А	0	0	А	0
Line 9 & Site Access	EB	stop	0	А	0	0	А	0

Table 12: Intersection Operations - 2025 Minor Soccer (Wednesday evening)

For the Minor Soccer events, some higher delays are incurred, resulting in slightly poorer levels of service. Notwithstanding, all intersection operations are considered acceptable (in that they are less than the level of service F, which denotes the intersection or movement capacity has been fully consumed), with the exception of the intersection of Line 7 with the east Highway 11 overpass. At this intersection, the average delay is 60 seconds for the westbound movement, thus resulting in a level of service F. However, such a delay is not considered unbearable. Furthermore, as these conditions will be experienced only for the peak inbound direction of soccer related traffic, which is limited to a 1 hour period, no road improvements are considered necessary at this intersection.

Intersection and Movement		Control	AM Peak Hour			PM Peak Hour		
			delay	LOS	v/c	delay	LOS	v/c
Line 7 & Hwy 11 Overpass W	WB	stop	14	В	0.42	11	В	0.24
Line 7 & Hwy 11 Overpass E	WB	stop	12	В	0.34	12	В	0.24
	EB		8	А	0.07	8	А	0.12
Line 7 &	WB	4-way	8	А	0.10	8	А	0.10
Simcoe Road 20	NB	stop	9	А	0.27	8	А	0.14
	SB		8	А	0.08	9	А	0.23
Line 8 & Simcoe Road 20	SB	stop	9	А	0.04	1	А	0.01
Line 9 & Hwy 11 Overpass W	EB	stop	11	В	0.39	13	В	0.54
Line 9 & Hwy 11 Overpass E	EB	stop	12	В	0.07	17	С	0.21
Line 9 &	NB	stop	12	В	0.29	11	В	0.15
Simcoe Road 20	SB	stop	10	В	0.08	11	В	0.20
Line 7 & Site Access	WB	stop	0	А	0	0	А	0
Line 8 &	EB	stop	0	А	0	0	А	0
Site Access	WB	stop	10	А	0.26	12	В	0.51
Line 9 & Site Access	EB	stop	0	А	0	0	A	0

Table 13: Intersection Operations - 2025 Tough Mudder (Saturday)

During the Tough Mudder peak hour operations, all intersections will provide acceptable operations (level of service C or better) and thus no improvements are considered necessary.

Intersection and Movement		Control	AM Peak Hour PM Peak Hour					our
		Control	delay	LOS	v/c	delay	LOS	v/c
Line 7 & Hwy 11 Overpass W	WB	stop	-	-	-	12	В	0.30
Line 7 & Hwy 11 Overpass E	WB	stop	-	-	-	12	В	0.36
	EB		-	-	-	9	А	0.16
Line 7 &	WB	4-way	-	-	-	8	А	0.11
Simcoe Road 20	NB	stop	-	-	-	8	А	0.15
	SB		-	-	-	9	А	0.24
Line 8 & Simcoe Road 20	SB	stop	-	-	-	9.5	А	0.04
Line 9 & Hwy 11 Overpass W	EB	stop	-	-	-	9	А	0.11
Line 9 & Hwy 11 Overpass E	EB	stop	-	-	-	10	А	0.10
Line 9 &	NB	stop	-	-	-	11	В	0.16
Simcoe Road 20	SB	stop	-	-	-	12	В	0.21
Line 7 & Site Access	WB	stop	-	-	-	0	А	0
Line 8 &	EB	stop	-	-	-	0	А	0
Site Access	WB	stop	-	-	-	0	А	0.51
Line 9 & Site Access	EB	stop	-	-	-	0	А	0

Table 14: Intersection Operations - 2025 Contemporary Music & Camping Festival (Sat mid-day)

Under the Contemporary Music and Camping Festival, all intersection operations are considered excellent (levels of service A or B) with minimal delays. As such, no road system improvements are considered necessary to support this event, as based on the noted traffic operations.

Intersection and Mov	iomont —	Control	AM Peak Hour			PM Peak Hour		
Intersection and Mov	/ement	Control	delay	LOS	v/c	delay	LOS	v/c
Line 7 & Hwy 11 Overpass W	WB	stop	17	С	0.58	-	-	-
Line 7 & Hwy 11 Overpass E	WB	stop	30	D	0.66	-	-	-
	EB		9	А	0.18	-	-	-
Line 7 &	WB	4-way	9	А	0.14	-	-	-
Simcoe Road 20	NB	stop	9	А	0.15	-	-	-
	SB		10	А	0.29	-	-	-
Line 8 & Simcoe Road 20	SB	stop	1	А	0.01	-	-	-
Line 9 & Hwy 11 Overpass W	EB	stop	11	В	0.34	-	-	-
Line 9 & Hwy 11 Overpass E	EB	stop	11	В	0.24	-	-	-
Line 9 &	NB	stop	12	В	0.18	-	-	-
Simcoe Road 20	SB	stop	12	В	0.33	-	-	-
Line 7 & Site Access	WB	stop	13	В	0.43	-	-	_
Line 8 &	EB	stop	0	А	0	-	-	-
Site Access	WB	stop	0	А	0.51	-	-	-
Line 9 & Site Access	EB	stop	0	А	0	-	-	-

Table 15: Intersection Operations - 2025 Barrie Automotive Flea Market (Sat mid-day)

All intersection operating conditions are considered acceptable under this event (level of service D or better) and thus no improvements are warranted to address poor traffic operations.

5.2.2 Queue Operations

Further to the intersection operations, the presence of traffic queues has been reviewed for each stop controlled movement and also the left turns on the major roads (which operate under free flow conditions and are thus only obstructed by opposing traffic). The corresponding 95th percentile queues are noted in Table 16 for the 2025 Saturday operations (the 95th percentile queue represents the queue length that would only be exceeded 5% of the time).

Table 16: Queue Operations - 2025 with Burl's Creek	

			95 th Percentile Queues (metres)						
Intersection and Mov	Intersection and Movement		Fur & Fathers	Farmer Market	Minor Soccer	Tough Mudder	Music & Camping	Automotive Flea Market	
Line 7 &	WB	stop	20	6	13	7	9	28	
Hwy 11 Overpass W	SB	left	1	1	4	1	2	1	
Line 7 &	WB	stop	11	8	79	7	13	35	
Hwy 11 Overpass E	SB	left	2	2	2	2	2	3	
Line 7 & Simcoe Road 20	EB		1	1	1	1	1	1	
	WB	4-way stop	1	1	1	1	1	1	
	NB		1	1	1	1	1	1	
	SB		1	1	1	1	1	1	
Line 8 &	SB	stop	1	1	1	1	1	1	
Simcoe Road 20	EB	left	1	1	1	1	1	1	
Line 9 &	EB	stop	5	4	2	26	3	12	
Hwy 11 Overpass W	NB	left	1	1	1	1	1	3	
Line 9 &	EB	stop	1	2	2	6	3	7	
Hwy 11 Overpass E	NB	left	2	1	1	2	1	0	
	NB	stop	1	1	1	1	1	1	
Line 9 &	SB	stop	1	1	1	1	2	1	
Simcoe Road 20	EB	left	9	4	3	4	11	5	
	WB	left	2	5	4	6	12	11	

		95 th Percentile Queues (metres)							
Intersection and Movement		Control	Fur & Fathers	Farmer Market	Minor Soccer	Tough Mudder	Music & Camping	Automotive Flea Market	
Line 7 &	WB	stop	5	0	2	0	0	17	
Site Access	SB	left	0	0	12	0	0	5	
	EB	stop	0	3	0	0	0	0	
Line 8 &	WB	stop	0	0	0	23	0	0	
Site Access	NB	left	0	1	0	0	1	0	
	SB	left	0	0	0	5	0	5	
Line 9 &	EB	stop	0	0	0	0	0	0	
Site Access	NB	left	0	0	0	0	0	0	

As noted, the 95th percentile queues are typically in the order of 1 vehicle or less (1 vehicle = 7.6 metre queue length). Queues at the Line 7 and Line 9 overpass intersections are somewhat greater (up to 10 cars or 79 metres under the Minor Soccer event), but remain within reason. At the site access points, any queueing can be accommodated readily within the site. At no point are traffic queues on Line 7 or Line 9 projected to extend to Highway 11 thus interfering with highway operations.

5.2.3 Right-In/Right-Out Operations

There are 2 right-in/right-out intersections at each of the intersections of Line 7, Line 8 and Line 9 with Highway 11. In all cases, the right-in to the highway is provided with a free flow movement, with an acceleration lane provided on the highway. This allows motorists to enter the highway and attain an appropriate travel speed prior to merging with the outside lane of traffic. Similarly, each right-out movement has a separate deceleration and a free-flow exit manoeuvre from the highway, thereby minimizing (if not negating) potential impacts to Highway 11. In consideration of these operations, the right-in/right-out intersections were not specifically considered (as each movement operates under free flow there are no delays and hence a level of service A results).

Traffic simulations were also reviewed for the Tough Mudder event, which results in the greatest volume of traffic entering and exiting Highway 11 at a single right-in/right-out location (the corresponding projected volumes are estimated at 515 to 520 vehicles per hour, which equates to less than 10 vehicles per minute). Based on the simulation results, no impacts to the highway operations are expected.

5.2.4 Link Operations

The resulting 2015, 2020 and 2025 total volume to capacity ratios are presented in the tables of Appendix E for each event being considered, for those days and time periods to which the event contributes additional traffic. Summaries of the peak hour v/c ratios for each event are provided in Table 17, Table 18 and Table 19 for the 2015, 2020 and 2025 horizon years. Where an event has multiple peaks, only the most critical figures are noted (ie. the greatest v/c ratios). When a v/c ratio exceeds 0.90, the road section is nearing capacity whereas when the v/c exceeds 1.0, the road section volumes surpass the capacity.

Road Section &		I	Level 1 Events	;	Level 2 Events			
Direction	Iα	Fur & Feathers	Farmers Market	Minor Soccer	Tough Mudder	Music & Camping	Auto Flea Market	
Highway 11	NB	0.56	0.89	0.66	0.92	0.82	0.86	
	SB	0.70	0.48	0.48	0.62	0.83	0.89	
Simcoe Road 20	NB	0.05	0.08	0.10	-	0.11	0.13	
	SB	0.08	0.06	0.09	-	0.10	0.12	
Line 7	NB	0.31	-	0.93	-	-	0.62	
	SB	0.52	-	0.88	-	-	0.81	
Line 8	EB	-	0.14	-	0.85	0.62	0.39	
	WB	-	0.15	-	0.86	0.09	0.48	
Line 9	EB	-	0.23	-	0.52	-	0.48	
	WB	-	-	-	-	-	-	

Table 17: Volume to Capacity Ratios - 2015 Volumes with Burl's Creek
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Highway 11 Operations

In considering operations on Highway 11, the various events will contribute an additional 30 to 500 vehicles per hour per direction on select sections of Highway 11. Combined with the existing and future traffic volumes on the highway, the future total traffic volumes (ie. with consideration for Burl's Creek) on Highway 11 will approach capacity (ie. v/c > 0.90) or slightly exceed its capacity (v/c of 1.01) during the associated peak hours of the noted events.

Road Section & Direction			_evel 1 Events	;		Level 2 Events			
		Fur & Feathers	Farmers Market	Minor Soccer	Tough Mudder	Music & Camping	Auto Flea Market		
Highway 11	NB	0.59	0.93	0.73	0.97	0.86	0.91		
	SB	0.74	0.51	0.53	0.65	0.88	0.93		
Simcoe Road 20	NB	0.07	0.10	0.12	-	0.13	0.14		
	SB	0.10	0.08	0.11	-	0.12	0.14		
Line 7	NB	0.33	-	0.96	-	-	0.65		
Line 7	SB	0.56	-	0.91	-	-	0.86		
Line 0	EB	-	0.15	-	0.86	0.63	0.40		
Line 8	WB	-	0.16	-	0.87	0.10	0.48		
Line O	EB	-	0.26	-	0.54		0.51		
Line 9	WB	-	-	-	-	-	-		

Table 18: Volume to Capacity Ratios - 2020 Volumes with Burl's Creek

Table 19: Volume to Capacity Ratios - 2025 Volumes with Burl's Creek

Road Section &			_evel 1 Events	;	l	_evel 2 Events	S
Direction	ΙQ	Fur & Feathers	Farmers Market	Minor Soccer	Tough Mudder	Music & Camping	Auto Flea Market
Highway 11	NB	0.62	0.98	0.73	1.01	0.91	0.95
	SB	0.77	0.54	0.53	0.68	0.92	0.98
Simcoe Road 20	NB	0.07	0.11	0.13	-	0.14	0.16
	SB	0.10	0.09	0.11	-	0.13	0.15
Line 7	NB	0.34	-	0.98	-	-	0.68
	SB	0.59	-	0.93	-	-	0.90
Line 8	EB	-	0.15	-	0.86	0.63	0.40
LINE O	WB	-	0.16	-	0.87	0.10	0.48
Line 9	EB	-	0.28	-	0.56	-	0.53
	WB	-	-	-	-	-	-

The greatest increase will result from the Tough Mudder event (only 1 occurrence) with arrivals primarily occurring the morning hours and departures during the afternoon hours on the corresponding Saturday. The associated v/c ratios range from 0.92 to 1.01 in the Highway 11 northbound direction of the AM peak hour, which is conservative in that it assumes the peak hours for the Tough Mudder will correspond to the peak hours of the highway. It is further noted that only the section of Highway 11 between Line 7 and Line 8 will realize an increase of 500 vehicles during the arrival peak hour, whereas only the section from Line 8 to Line 9 will realize the same increase during the departure peak hour. Outside of these road sections, the additional volumes are projected in the order of 100 to 350 vehicles depending on direction of travel. This increase results from the need for travelers to utilize the downstream intersections at Line 9 and Line 7 to change travel direction for access to/from Line 8. This also assumes that all vehicles travelling to/from the site will utilize Line 8 to access the highway. Any redistribution of traffic to Line 7 or Line 9 will lessen the incremental increases in traffic volumes through the noted sections of Highway 11. Given the short duration of the increased traffic volumes, the operating conditions are considered acceptable, and thus no improvements are warranted. It is noted that consideration could be given to providing access to the parking area via Line 7 or Line 9 as appropriate, to provide a more distributed arrival pattern during the Tough Mudder event.

The Automotive Flea Market is expected to contribute 300 vehicles per hour per direction on Highway 11 to/from areas to the south, resulting in a v/c ratio of 0.89 to 0.98 in the vicinity of Line 7. Burl's Creek will operate parking/access off Line 7 and Line 8 during this event, thus distributing the traffic between these 2 roads accordingly, and alleviating demands on the highway system beyond Line 7. Again, given the short nature of this event (2 occurences per year), no improvements to the highway system are considered necessary.

Simcoe Road 20 Operations

The existing traffic volumes on Simcoe Road 20 equate to approximately 9% or less of its available road capacity (ie. volumes of 70 vehicles per hour or less vs a capacity of 800 vehicles per hour per lane). While Simcoe Road 20 provides an alternative and parallel travel route to Highway 11, it does not extend beyond Line 11 to the north. To the south, Simcoe Road 20 turns into Shanty Bay Road in the north end of the City of Barrie. While Simcoe Road 20 provides a connection to the City, the majority of site visitors are expected to access the site via Highway 11. Increases of 20 to 30 vehicles per hour per direction are projected (as per Figure 19 through Figure 24), which can be readily accommodated on the road (25x the amount could in fact be accommodated).

Township Road Operations

The only noted issue on the Township roads is that of Line 7 associated with the Minor Soccer events, during which time traffic volumes from Highway 11 to the site access are projected to be 93 to 98% of capacity. During this event (13 Wednesday evening occurrences), all 600 athletes are assumed to arrive during the same hour in that all of the soccer sessions will commence at 6:15 (which corresponds to the commuter PM peak hour). Based on the traffic projections and assignment of

traffic volumes to the area roads, peak hour peak direction volumes during this arrival hour will approach 600 vehicles per hour (which is the assumed planning capacity of Line 7). It is assumed that the primary access route will consist of Line 7 from Highway 11 to the site, although there are other possible routes via Line 9 and Simcoe Road 20 that would reduce demands on Line 7. The concluding times for the soccer events will be staggered somewhat due to different match durations for different age groups. A similar approach could also be adopted to stagger the start times by age group, thus alleviating some of the peak hour demands. Given that the road will continue to operate below capacity, that other alternative routes are possible and that changes to the actual event operations can also be considered (eg. staggering start and end times, hosting the soccer matches over several nights or providing access via Line 8), no improvements to the road system are considered necessary to support the soccer events.

The more significant increases in traffic volumes on the local roads associated with the remaining events are as follows:

- 250 to 500 vehicles per direction on Line 8 and 200 per direction on Line 9 during Tough Mudder;
- up to 360 vehicles per direction on Line 8 during the Contemporary Music and Camping Festival; and
- 180 to 270 vehicles per direction on Lines 7, 8 and 9 during the Barrie Automotive Flea Market.

Notwithstanding the noted increases, the resulting total volumes will remain below the assumed road capacities (particularly on Line 8 on which existing volumes are minimal) and thus are acceptable.

5.3 Turn Lane Requirements

The need for turn lanes at the site access points has been considered based on the nature of the event (occurrences per year and duration) and corresponding traffic volumes (both existing and future). As several of the events will occur over a single day, and may only happen once or twice per year, it is not considered necessary to implement permanent infrastructure improvements.

5.3.1 Right Turn Lanes

As per the MTO *Geometric Design Guidelines for Ontario Highways*, right turn lanes are recommended when the turning volume exceeds 60 vehicles per hour and has the potential to impact through movements due to turning vehicles. In considering the projected volumes for each event (as per Figure 19 through Figure 24), the noted threshold for a right turn lane is exceeded during the following events:

- Farmers Market 75 EB right turns at the Line 8 access (Friday evening);
- Contemporary Music & Camping Festival 360 EB right turns at the Line 8 access (Saturday/ Sunday mid-day); and

 Barrie Automotive Flea Market - 120 WB right turns at the Line 8 access (Saturday/Sunday midday).

However, as Line 8 has a limited service area (it extends only from Highway 11 to Simcoe Road 20), existing volumes on this road section are minimal (less than 20 vehicles per during the peak hours). In this regard, any impacts to through traffic, resulting from the right turning traffic, will be negligible. In this regard, right turn lanes are not considered necessary.

5.3.2 Left Turn Lanes

As per the MTO *Geometric Design Guidelines for Ontario Highways*, the need for left turn lanes is based on left turn traffic volumes, through volumes and the design speed. As with right turn lanes, left turn lanes are recommended when the volume of turning traffic unduly impacts the through traffic. Increased left turn movements are anticipated for the following events:

- Huronia Fur & Feathers 120 EB left turns at the Line 7 access (Sunday mid-morning);
- Minor Soccer 450 EB left turns at the Line 7 access (Wednesday evening);
- Tough Mudder 250 to 500 EB left turns at the Line 8 access (Saturday mid-morning and midafternoon); and
- Barrie Automotive Flea Market 225 EB left turns at the Line 7 and Line 8 access (Saturday/Sunday mid-day).

Apart from Minor Soccer, the occurrences of the remaining events is limited (2 Sunday mornings for Fur & Feathers, 1 Saturday for Tough Mudder and 4 days for the Flea Market x 2 events) and thus left turn lanes to support these events are not considered necessary (particularly on Line 8 given the limited volume of other traffic on the road during the noted peak hours).

The Minor Soccer events are expected on 13 Wednesday evenings over the months of May, June, July and August. Based on the MTO left turn lane warrants, an EB left turn lane is warranted at the Line 7 access (the provision of which would also serve the remaining events using the same access). In considering a 70 km/h design speed (10 km/h over the speed limit), the left turn lane should consist of a 25 metre storage length, a 40 metre parallel lane and a 115 metre taper. Based on a review of Simcoe County mapping, Line 7 has a 26 metre right-of-way in the area of the site access (original 20 metres + 6 metre widening along the north side). As the road is centred in the original 20 metre right-of-way, it is offset to the south within the 26 metre right-of-way. In consideration of this, and given the line of large, mature trees along the south side of Line 7, the left turn lane should be implemented on the north side of the road where it can be more readily implemented with lesser impacts.

5.4 Monitoring of Traffic Operations

It is acknowledged that the traffic volumes for the basis of the operational and turn lane assessments have been established from limited available existing data and projections for the corresponding events. In this regard, it is recommended that site operations be monitored over the course of the initial year of operations to confirm traffic volumes along the area roads and to/from the site, from which the need for and exact timing of, the road improvements can be established.

As many of the events will have limited duration and limited occurrences, supporting permanent infrastructure may not be required. While some impacts may occur (eg. congestion and increased delays to the motoring public) such are tolerable given the limited nature. Furthermore, for particular events, temporary traffic control measures could be considered as warranted by operations (eg. traffic control persons at the site access points).

6 Summary

This study has reviewed the transportation impacts associated with the proposed Burl's Creek Event Grounds, located east of Highway 11 between Line 7 and Line 9 in Oro-Medonte.

Existing Traffic Operations

Based on 2014/15 traffic volumes on the study area roads, all relevant road sections (including those on Highway 11) are operating below their respective planning capacities. There are no known operational issues at any of the corresponding study area intersections given the limited volumes - single lane approaches and stop control are considered appropriate.

Future Traffic Operations

A 2015 horizon (opening year), 2020 horizon (5-year beyond opening) and 2025 horizon (10-year beyond opening) were considered for this study.

An annual growth rate of 1% was applied to the existing traffic volumes on Highway 11 and the local Township roads, and 2% on Simcoe Road 20 to estimate future background traffic volumes for the horizon years. These were based on historic growth in summer traffic conditions and in considering future growth potential in the area.

Under future background conditions (ie. without Burl's Creek Event Grounds traffic) the study area roads will continue to operate below their capacity levels. Highway 11 will operate at 86% or less of its capacity during the Saturday, Sunday and weekday peak hours, and 96% or less during the more critical Friday peak hour (which reflects the increase in NB traffic heading to "cottage country"). Simcoe Road 20 and the Township roads will operate at less than 12% and 36% of their respective capacities.

Burl's Creek Event Grounds

The Burl's Creek Event Grounds has 8 distinct events currently planned for the 2015 season, including soccer, music festivals, satellite parking, famer and flea markets, and camping. Attendance at the site events is expected to range from 500 people per day to 40,000 people per day. In considering auto occupancy levels of 1.5 to 2.5 persons per vehicles (depending on event) 333 to 16,000 vehicles are expected per day. Based on event durations and arrival/departure times, and not considering the major music festivals (which have been addressed in a separate traffic control report), the site is expected to generate 167 to 1200 vehicle trips per hour (total of inbound and outbound trips).

Existing site access will be largely maintained, providing access via Line 7 and Line 8 (with multiple access points along the latter road). Access is also proposed vial Line 9 for the major music festivals.

Future Traffic Operations with Burl's Creek

In considering the future total traffic volumes (eg. background traffic + event traffic) associated with each event (with the exception of the major music festivals), the area intersections will continue to operate below their capacities with a few exceptions.

- The key study area intersections will typically operate at level of service A or B, with decreased levels of service C or D occurring at select intersections under select events. All of these operations are considered acceptable.
- Under the Minor Soccer events, the intersection of Line 7 and the Highway 11 overpass on the east side of the highway will experience a level of service F under 2025 conditions resulting from increased delays for those at the stop sign. However, the corresponding delay of 60 seconds is not considered unbearable, and will only occur over a short arrival period (all athletes will arrive over the course of 1 hour). The 95th percentile queue for the southbound movements on the overpass is estimated at 79 metres under the 2025 conditions, which correlates to approximately 10 vehicles, and can be accommodated on the overpass.

Similarly, the key road sections will typically operate below their respective capacities, following consideration for the Burl's Creek Event Grounds site traffic.

- Highway 11 is projected to operate at 48% to 101% of capacity over the 2015 to 2025 horizon years (the maximum under the future background traffic levels was 96% with the incremental increase associated with site operations). As such, Highway 11, the major road providing service to the area can accommodate the projected event traffic (operations at capacity will be limited in duration and occurrence).
- Simcoe Road 20 is expected to operate at 5% to 16% during the same periods.
- On Line 7, the projected volumes will approach the planning capacity of 600 vehicles per hour per lane on the section from Highway 11 to the site access during the Minor Soccer events. Peak hour peak directional volumes are projected in the order of 555 to 585 vehicles per hour. However, this is not considered critical given the relatively short section of road in question and the rural and remote nature of the area, and limited duration (soccer occurs over 2 hours). Notwithstanding, consideration could be given to staggering the start time of the soccer matches to better accommodate arrival traffic (as it is, all 600 participants start at 6:15PM). There are also alternative routes to/from the soccer pitches that users will divert to should operations become problematic (the majority of traffic was assumed to access the site via Line 7 and Highway 11). Additional means of access to/from the site could also be provided for these events (eg. utilizing site access of Line 7 and Line 8).
- On Line 8, traffic volumes associated with the Tough Mudder will amount to in excess of 500 vehicles per hour through the section from Highway 11 to the site access (approximately 86% of capacity). However, this road has minimal background traffic volumes (ie. most of the noted volumes will be site related) and thus minimal impacts to other road users will result. This event is limited to a single day also.

In considering MTO warrants for left and right turn lanes, the associated event traffic volumes will exceed the thresholds for EB left turns at the Line 7 access; and EB left and WB right turns at the Line 8 access points.

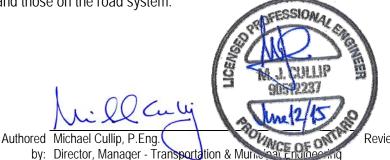
- Notwithstanding, given the limited nature of the events and traffic otherwise on Line 8, no additional turn lanes are considered necessary. While the associated turn volumes may exceed the threshold limits, impacts to other road users will be limited (ie. it will primarily be site traffic on the road).
- At the Line 7 access, an EB left turn lane is recommended in consideration of the projected traffic volumes and more regular use of the Line 7 access for site events. Given the location of the road within the existing right-of-way, and presence of large mature trees on the south side of the road, the left turn lane is recommended to be constructed on the north side of the road. Prior to implementation, the need for the turn lane should be confirmed based on actual event and Line 7 traffic volumes during the course of the event (as suggested in the monitoring program).

Monitoring

The future traffic volumes projected under each event scenario have been established based on limited traffic volumes through the area and in consideration of anticipated event attendance and travel patterns. As the development proceeds, and events occur, it is recommended that traffic volumes and conditions on the area roads and at the site access points be counted and monitored during the initial year of operations to establish actual traffic levels and corresponding peak durations.

Based on the first year of operations, the need for and timing of, external road improvements can be confirmed (for implementation in the following year).

Temporary traffic control measures can also be considered as warranted (eg. traffic control persons at the site access points) during peak events to provide additional guidance and control to site visitors and those on the road system.

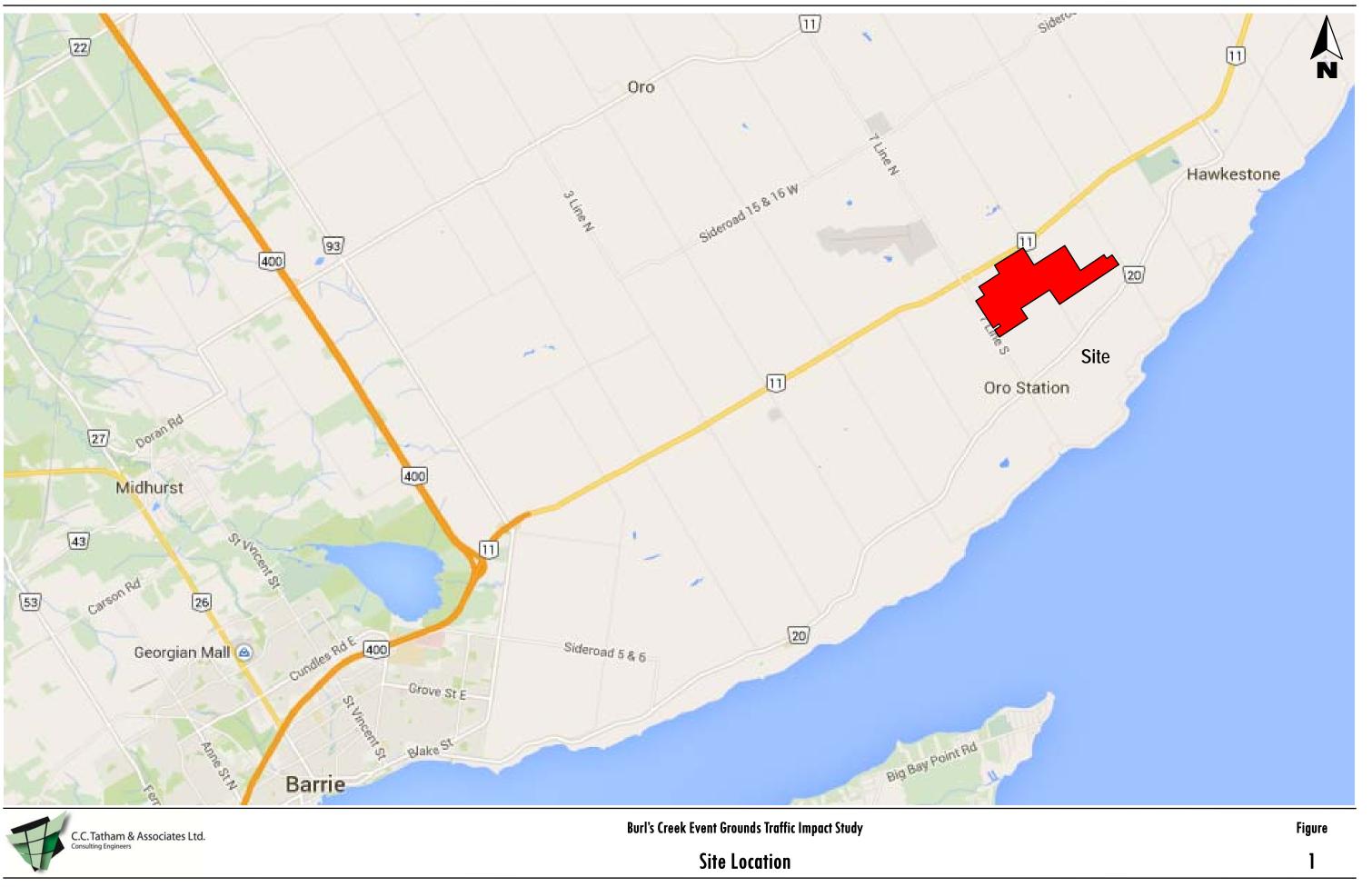


Reviewed David Perks, PTP by: Transportation Planner

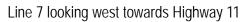
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Highway 11 NB looking towards Line 8 right-in/right-out



Line 7 looking west towards Highway 11 overpass intersection



Line 7 looking west at Burl's Creek access



Line 9 looking west towards Highway 11





Line 7 looking east towards Simcoe Road 20

Burl's Creek Event Grounds Traffic Impact Study

Area Road Network





Simcoe Road 20 looking south at Line 7



Simcoe Road 20 looking south at Line 8



Figure

2

Simcoe Road 20 looking south at Line 9





C.C. Tatham & Associates Ltd. Consulting Engineers Burl's Creek Event Grounds Traffic Impact Study

Key Intersections

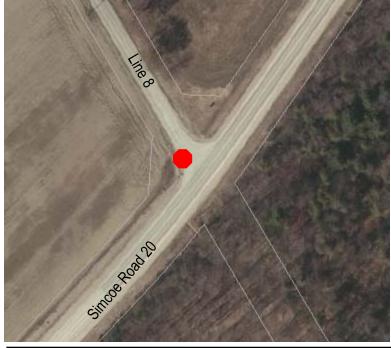




Figure **3a**

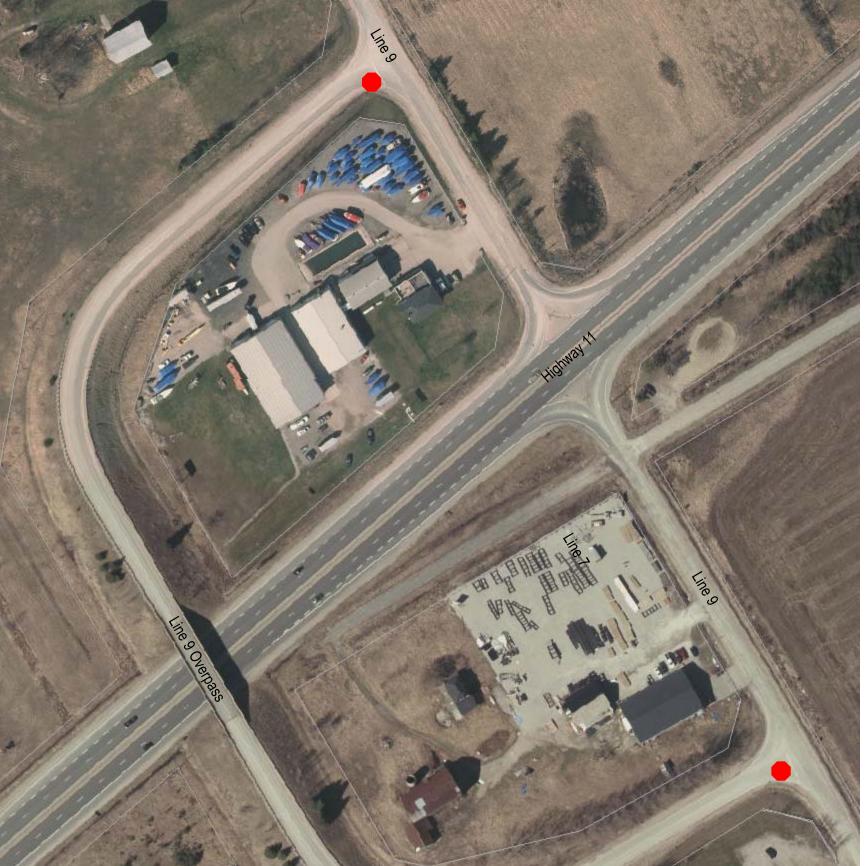










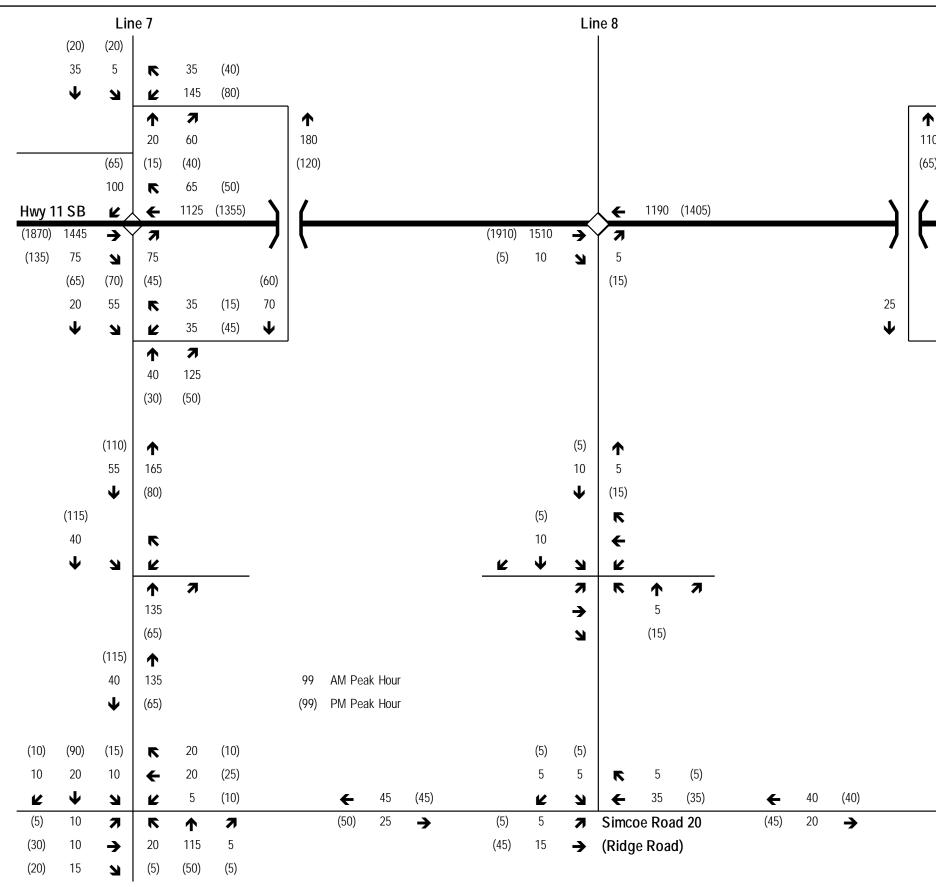


Burl's Creek Event Grounds Traffic Impact Study

Key Intersections



Figure **3b**





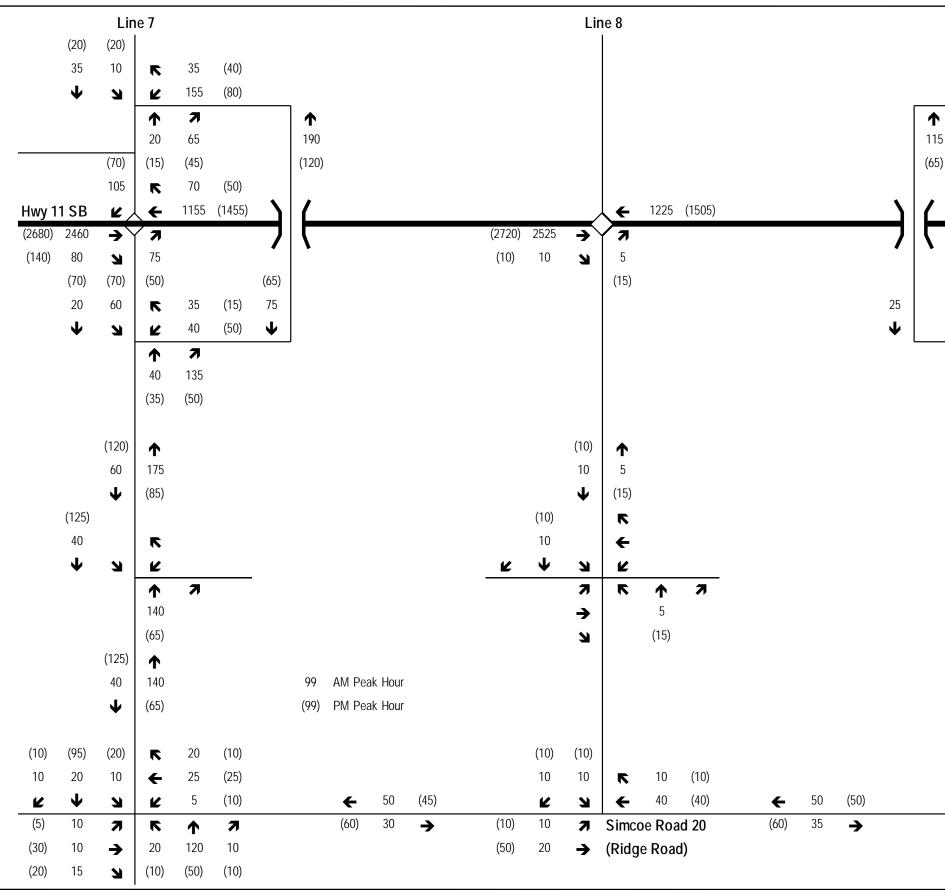
Burl's Creek Event Grounds Traffic Impact Study

2015 Traffic Volumes - Weekday Peak Hours

			Lin	ne 9			
		(10)	(10)				
		5	25				
		K	$\mathbf{\Lambda}$				
	(15)	15	7	7	↑		
0	(50)	95	Ы	15	• 5		
5)			(60)	(40)	(10)		
			120	Л	20	(40)	
			Ľ,	+	1070	(1345)	
	(1850)	1485	→	7	Hwy	11 NB	
	(75)	30	R	50			
		(25)	(50)	(30)			
		15	15				
		Ľ	$\mathbf{\Psi}$				
	(15)	10	7	R	♠		
	(35)	15	R	95	40		
				(40)	(15)		
			(85)	♠			
			30	135			
			$\mathbf{\Lambda}$	(55)			
			(85)				
			30				
		Ľ	•	_			
			7	R	↑		
			R		135		
					(55)		
	(5)	(70)	(5)	R	15	(5)	
	5	15	5	←	20	(20)	
	Ľ	$\mathbf{\Psi}$	Ы	Ľ	5	(10)	
	(5)	5	7	R	↑	7	
	(25)	5	→	15	115	5	
	(15)	10	R	(5)	(45)	(5)	

N

Figure



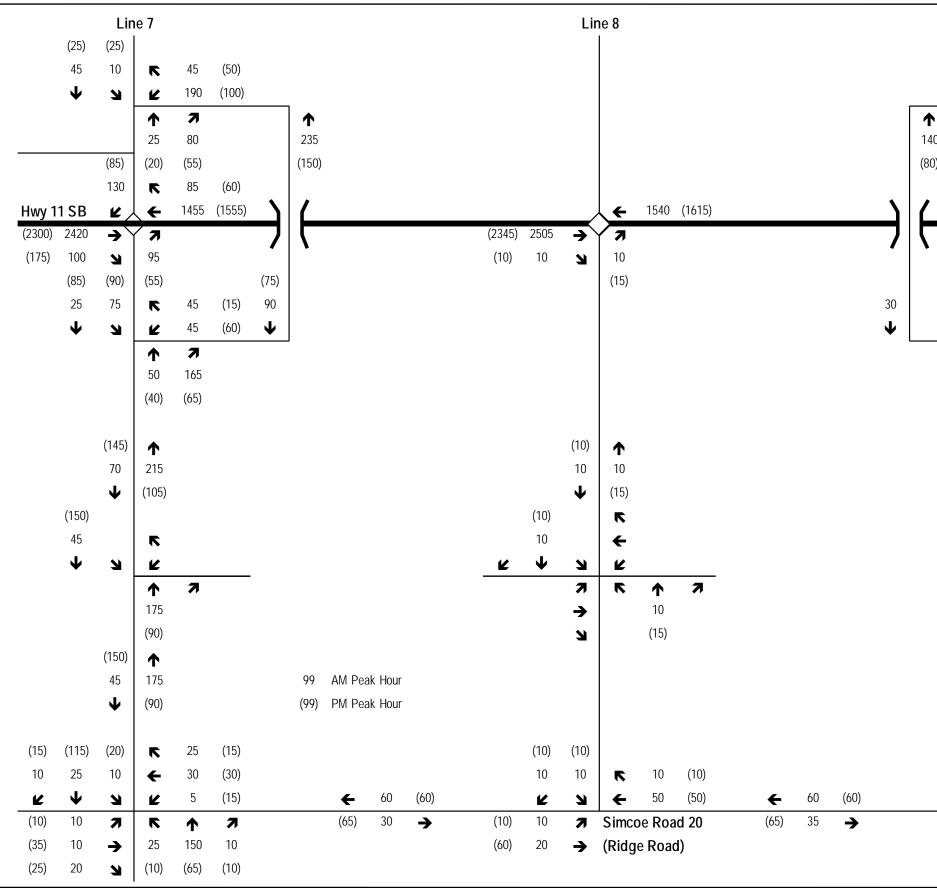
C.C. Tatham & Associates Ltd.

Burl's Creek Event Grounds Traffic Impact Study

2015 Traffic Volumes - Friday Peak Hours

		(10)		ie 9			
		(10)	(10)				
		10	25				
	(45)	Ľ	↓				
•	(15)	15	7	R	T		
5	(50)	100	K	20	5		
5)			(60)	(40)	(10)		
			125	r	25	(40)	
			Ľ	+	1100	(1445)	
	(2655)	2500	→ `	7	Hwy	11 NB	
	(80)	30	R	50			
		(25)	(55)	(35)			
		15	15				
		K	$\mathbf{\Phi}$				
	(15)	10	7	R	↑		
	(35)	15	Ы	100	40		
				(40)	(20)		
			(90)	↑			
			30	140			
			$\mathbf{\Lambda}$	(60)			
			(90)				
			30				
		Ľ	$\mathbf{\Lambda}$				
			7	R	↑		
			R		140		
					(60)		
	(10)	(75)	(10)	R	20	(10)	
	10	20	10	←	25	(25)	
	Ľ	$\mathbf{\Psi}$	R	Ľ	5	(15)	
	(10)	10	7	K	↑	7	
	(30)	10	→	20	125	10	
	(20)	15	R	(10)	(50)	(10)	
				•			

Figure



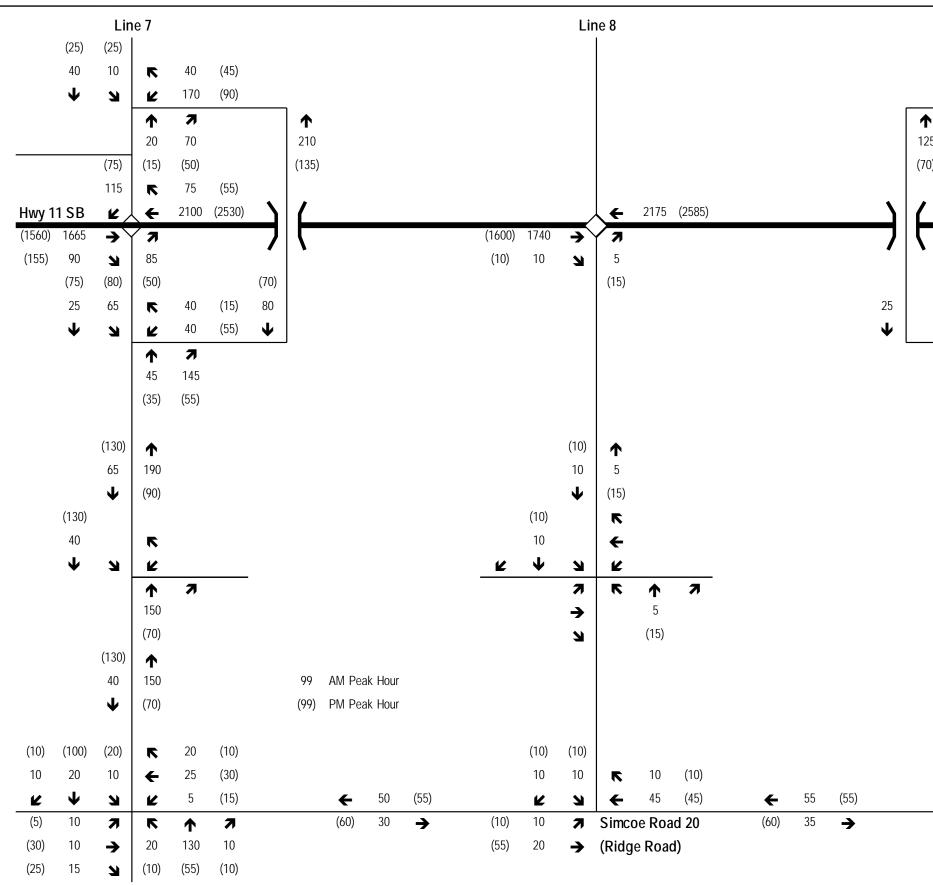


Burl's Creek Event Grounds Traffic Impact Study

2015 Traffic Volumes - Saturday Peak Hours

	Line 9												
		(15)	LII (15)										
		10	30										
		K											
	(20)	20	7	R	↑								
10	(60)	120	N	20	5								
0)	()		(75)	(50)	(10)								
,			150	7	25	(50)							
			K	÷	1390	(1540)							
	(2265)	2475	→	7	Hwy	11 NB							
	(95)	40	N	60	-								
		(30)	(65)	(40)									
		20	20										
		Ľ	$\mathbf{\Psi}$										
	(20)	10	7	R	↑								
	(45)	20	R	120	50								
				(50)	(20)								
			(110)	↑									
			40	170									
			$\mathbf{+}$	(70)									
			(110)										
			40										
		Ľ	↓										
			7	K	↑								
			R		170								
					(70)								
	(10)	(95)	(10)	R	20	(10)							
	10	20	10	←	30	(30)							
	K	$\mathbf{\Psi}$	N	Ľ	5	(15)							
	(10)	10	7	R	↑	7							
	(35)	10	→	20	150	10							
	(20)	15	R	(10)	(60)	(10)							

Figure



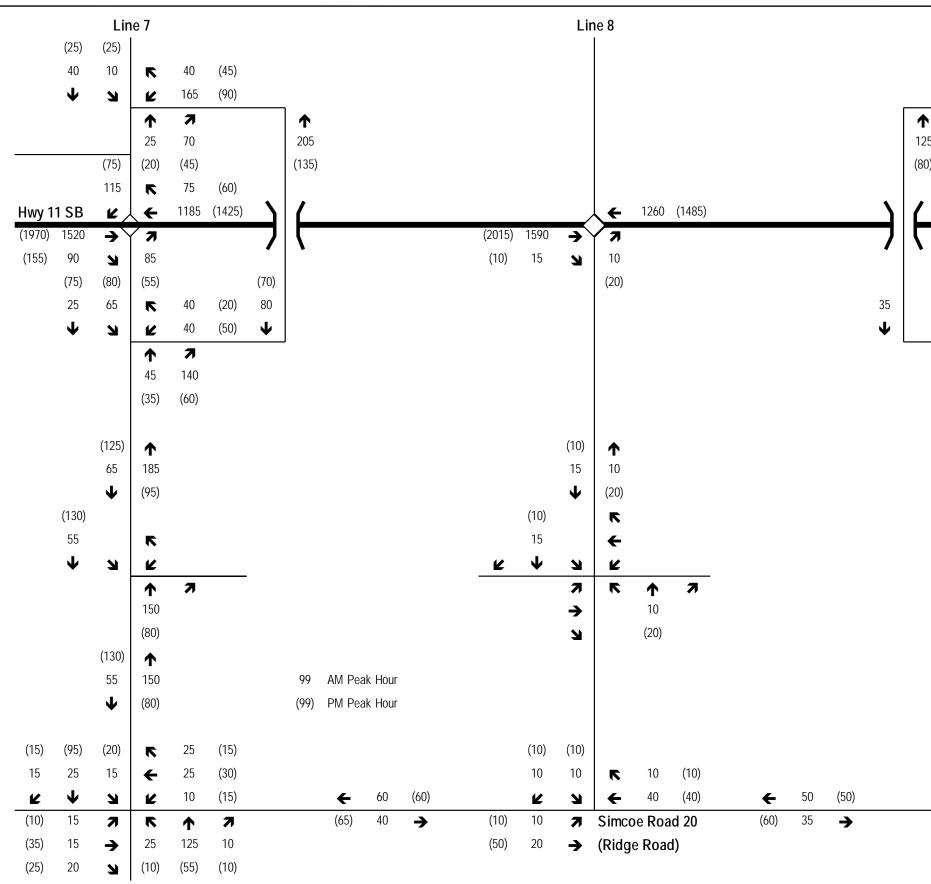


Burl's Creek Event Grounds Traffic Impact Study

2015 Traffic Volumes - Sunday Peak Hours

				ie 9								
		(10)	(15)									
		10	30									
		Ľ	\mathbf{A}									
	(15)	15	7	R	↑							
25	(55)	110	R	20	5							
0)			(70)	(45)	(10)							
			140	R	25	(45)						
			Ľ	`	2035	(2515)						
	(1530)	1710	→ `	7	Hwy	11 NB						
	(85)	35	И	55								
		(25)	(60)	(40)								
		20	15									
		Ľ	$\mathbf{\Psi}$									
	(20)	10	7	R	↑							
	(40)	15	И	110	45							
				(45)	(20)							
			(100)	♠								
			30	155								
			$\mathbf{\Psi}$	(65)								
			(100)									
			30									
		K	$\mathbf{\Psi}$									
			7	R	↑							
			R		155							
					(65)							
	(10)	(85)	(10)	R	20	(10)						
	10	20	10	÷	25	(25)						
	Ľ	$\mathbf{\Psi}$	Я	Ľ	5	(15)						
	(10)	10	7	K	↑	7						
	(30)	10	→	20	135	10						
	(20)	15	И	(10)	(55)	(10)						
				l								

Figure



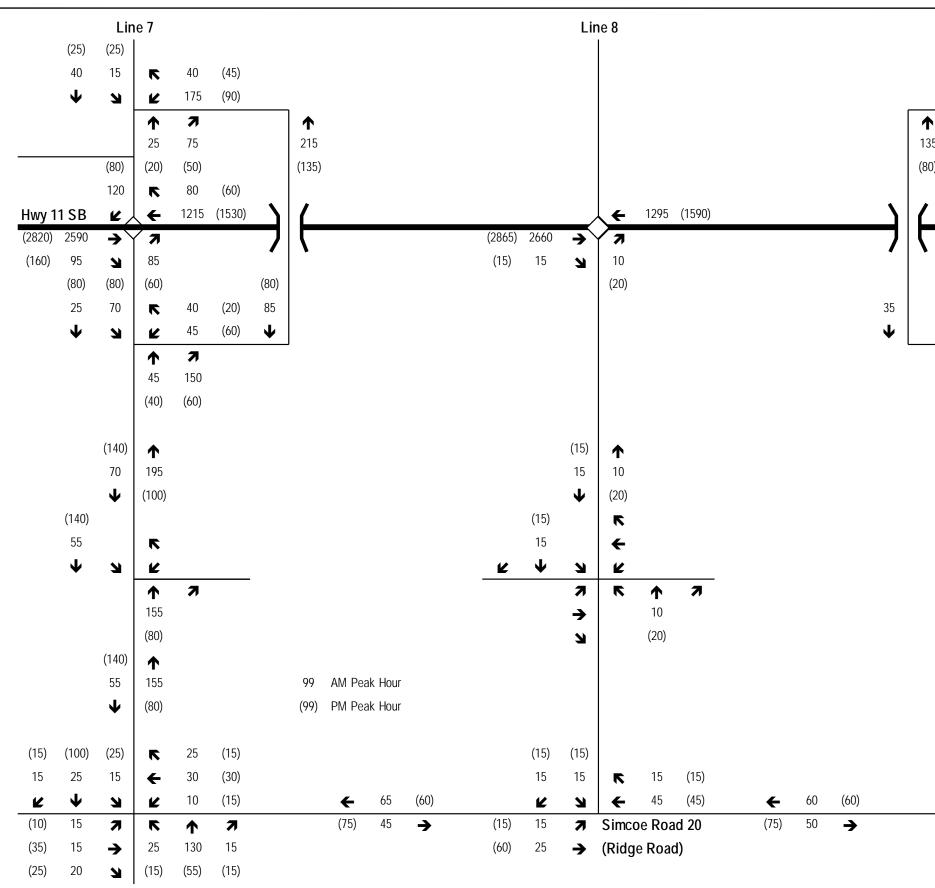
C.C. Tatham & Associates Ltd.

Burl's Creek Event Grounds Traffic Impact Study

2020 Traffic Volumes - Weekday Peak Hours

				e 9			
		(15)	(15)				
		10	30				
		Ľ	$\mathbf{\Psi}$				
	(20)	20	7	R	↑		
25	(60)	105	R	20	10		
0)			(75)	(45)	(15)		
			135	R	30	(45)	
			Ľ	`	1125	(1410)	
	(1945)	1560	→	7	Hwy	11 NB	
	(90)	40	Ľ	60			
		(30)	(60)	(40)			
		20	20				
		Ľ	1				
	(20)	15	7	R	↑		
	(40)	20	R	105	45		
				(45)	(20)		
			(100)	↑			
			40	- 150			
			1	(65)			
			(100)				
			40				
		K	\mathbf{V}				
			7	R	♠		
			R		- 150		
					(65)		
	(10)	(75)	(10)	R	20	(10)	
	10	20	10	←	25	(25)	
	K	$\mathbf{\Psi}$	Ľ	K	10	(15)	
	(10)	10	7	R	1	7	
	(30)	10	→	20	125	10	
	(20)	15	Ľ	(10)	(50)	(10)	

Figure

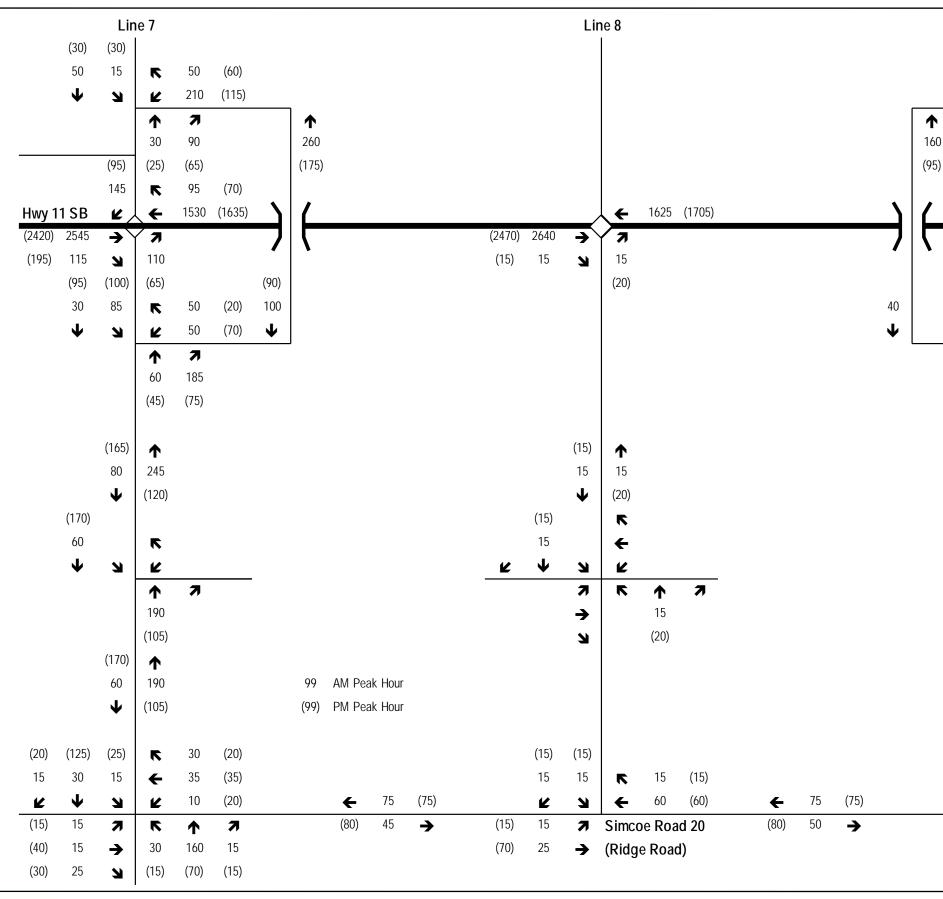


C.C. Tatham & Associates Ltd. Consulting Engineers Burl's Creek Event Grounds Traffic Impact Study

2020 Traffic Volumes - Friday Peak Hours

			Lin	ie 9			
		(15)	(15)				
		15	30				
		K	$\mathbf{\Psi}$				
	(20)	20	7	R	↑		
35	(60)	115	R	25	10		
0)			(75)	(45)	(15)		
			145	R	35	(45)	
			Ľ	∖ ←	1150	(1515)	
	(2790)	2630	→	7	Hwy	11 NB	
	(95)	40	R	60			
		(30)	(65)	(45)			
		20	20				
		Ľ	$\mathbf{1}$				
	(20)	15	7	K	↑		
	(40)	20	R	115	45		
				(45)	(25)		
			(105)	↑			
			40	160			
			\mathbf{A}	(70)			
			(105)				
			40				
		Ľ	↓				
			7	K	↑		
			R		160		
					(70)		
	(15)	(80)	(15)	R	25	(15)	
	15	25	15	⊼ ←	30	(30)	
	Ľ	↓	Ŋ	Ľ	10	(20)	
	(15)	15	7	- K	1	7	•
	(35)	15	→	25	135	15	
	(25)	20	ĸ	(15)	(55)	(15)	
				-		-	

Figure

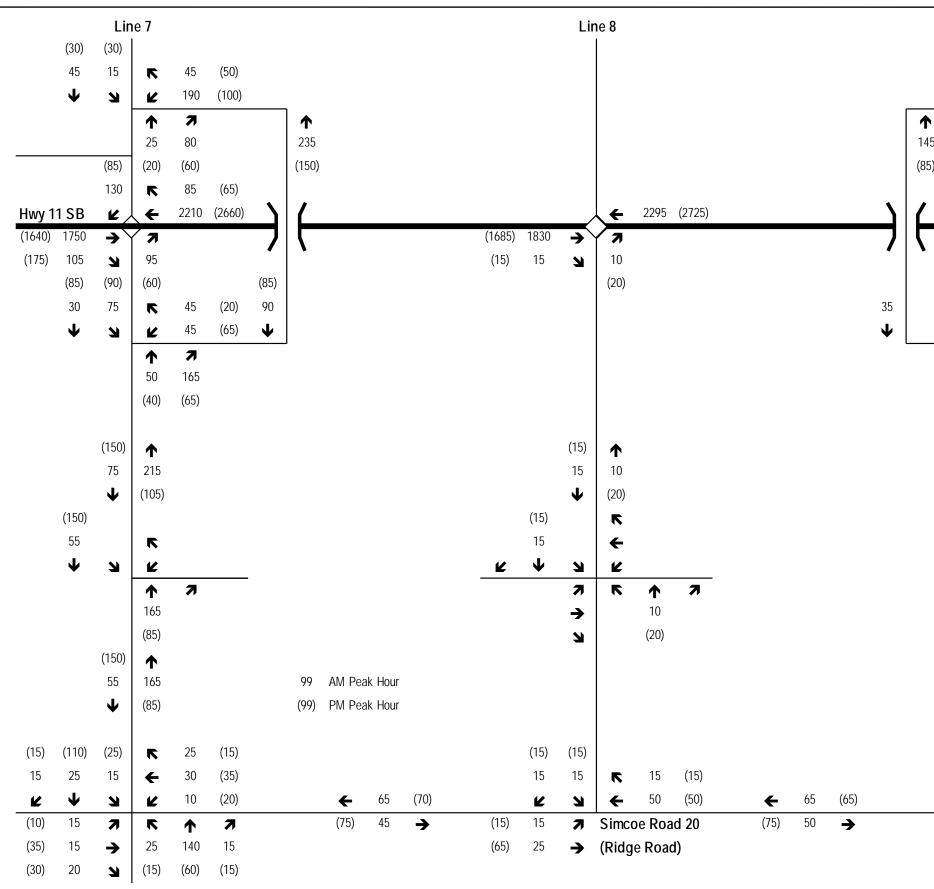


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Burl's Creek Event Grounds Traffic Impact Study

2020 Traffic Volumes - Saturday Peak Hours

	Line 9									
		(20)		e 9						
		(20)	(20)							
		15	35							
	(05)	2	◆	_						
	(25)	25	7	K	↑					
0	(70)	135	N	25	10					
)			(90)	(60)	(15)	((0)				
			170	K	35	(60)				
	()		Ľ	+	1455	(1615)				
	(2380)	2605	→	7	Hwy	11 NB				
	(110)	50	R	75						
		(35)	(75)	(50)						
		25	25							
		Ľ	↓							
	(25)	15	7	R	↑					
	(50)	25	R	135	60					
				(60)	(25)					
			(125)	↑						
			50	195						
			↓	(85)						
			(125)							
			50							
		Ľ	$\mathbf{\Psi}$							
			7	R	↑					
			И		195					
					(85)					
	(15)	(100)	(15)	R	25	(15)				
	15	25	15	←	35	(35)				
	Ľ	$\mathbf{\Psi}$	R	K	10	(20)				
	(15)	15	¥	K	↑	7				
	(40)	15	→	25	160	15				
	(25)	20	R	(15)	(65)	(15)				

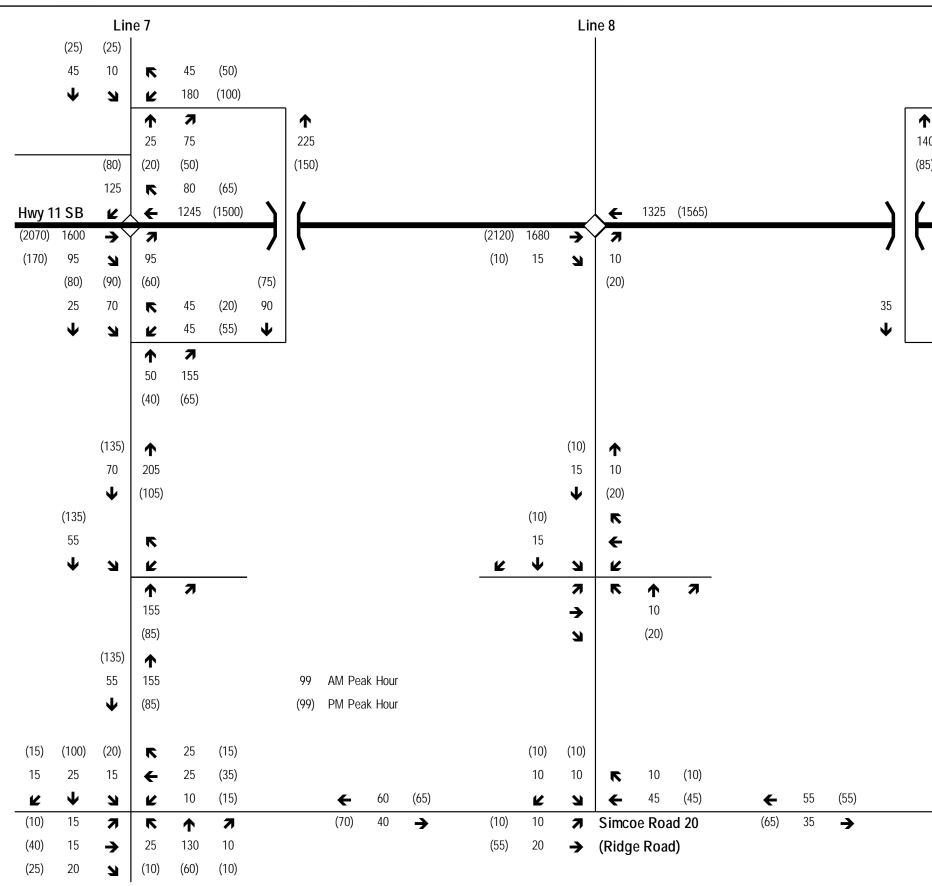




Burl's Creek Event Grounds Traffic Impact Study

2020 Traffic Volumes - Sunday Peak Hours

			Lin	e 9		
		(15)	(20)			
		15	35			
		Ľ	$\mathbf{\Psi}$			
	(20)	20	7	R	↑	
5	(65)	125	Ы	25	10	
5)			(85)	(50)	(15)	
			160	R	35	(50)
			Ľ	` +	2135	(2640)
	(1605)	1795	→	7	Hwy	11 NB
	(100)	45	Ы	65		
		(30)	(70)	(50)		
		25	20			
		Ľ	$\mathbf{\Psi}$			
	(25)	15	R	R	↑	
	(45)	20	Ы	125	50	
				(50)	(25)	
			(115)	↑		
			40	175		
			$\mathbf{\Psi}$	(75)		
			(115)			
			40			
		Ľ	$\mathbf{\Lambda}$			
			7	R	↑	
			R		175	
					(75)	
	(15)	(90)	(15)	R	25	(15)
	15	25	15	←	30	(30)
	Ľ	↓	R	Ľ	10	(20)
	(15)	15	7	R	↑	7
	(35)	15	→	25	145	15
	(25)	20	R	(15)	(60)	(15)

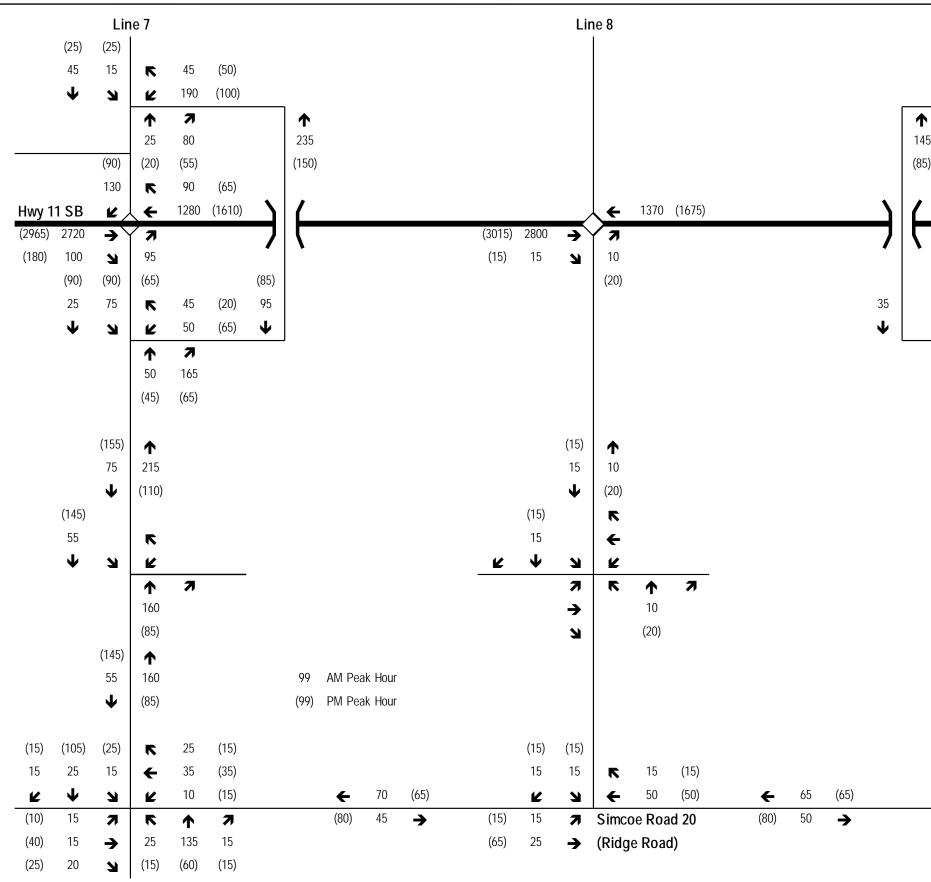


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Burl's Creek Event Grounds Traffic Impact Study

2025 Traffic Volumes - Weekday Peak Hours

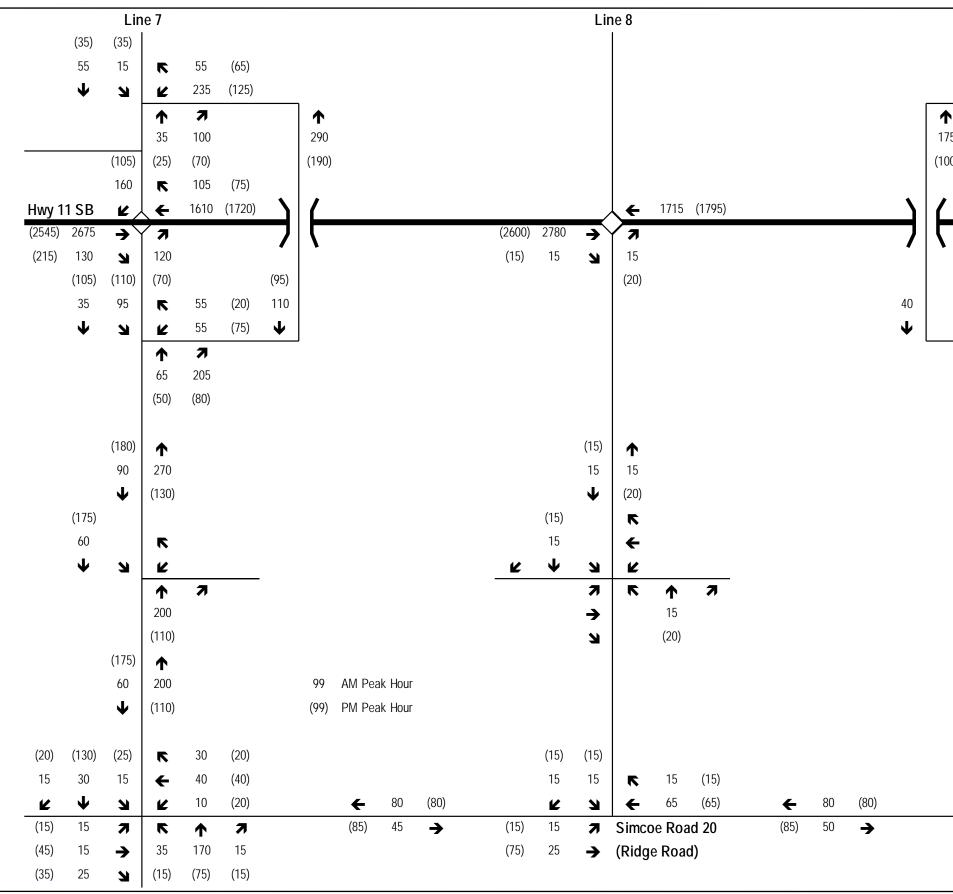
			Lin	ie 9			
		(15)	(15)				
		10	35				
		Ľ	$\mathbf{\Psi}$				
	(20)	20	7	R	↑		
10	(65)	120	R	20	10		
5)			(80)	(50)	(15)		
			155	R	30	(50)	
			Ľ	`	1170	(1485)	
	(2040)	1650	→	7	Hwy	11 NB	
	(100)	40	Ы	65			
		(35)	(65)	(40)			
		20	20				
		Ľ	$\mathbf{\Psi}$				
	(20)	15	7	R	↑		
	(45)	20	R	120	50		
				(50)	(20)		
			(110)	♠			
			40	170			
			$\mathbf{\Psi}$	(70)			
			(110)				
			40				
		K	$\mathbf{\Psi}$				
			7	R	↑		
			R		170		
					(70)		
	(10)	(80)	(10)	R	20	(10)	
	10	20	10	←	25	(25)	
	Ľ	$\mathbf{\Psi}$	R	Ľ	10	(15)	
	(10)	10	A	K	↑	7	
	(35)	10	→	20	130	10	
	(20)	15	R	(10)	(50)	(10)	



C.C. Tatham & Associates Ltd. Consulting Engineers Burl's Creek Event Grounds Traffic Impact Study

2025 Traffic Volumes - Friday Peak Hours

			Lin	e 9			
		(15)	(15)				
		15	35				
		K	$\mathbf{\Psi}$				
	(20)	20	7	R	♠		
5	(65)	125	R	25	10		
5)			(80)	(50)	(15)		
			160	R	35	(50)	
			Ľ	` (1210	(1595)	
	(2930)	2770	→	7	Hwy	11 NB	
	(105)	40	R	65			
		(35)	(70)	(45)			
		20	20				
		K	$\mathbf{\Psi}$				
	(20)	15	7	R	↑		
	(45)	20	R	125	50		
				(50)	(25)		
			(115)	↑			
			40	175			
			$\mathbf{+}$	(75)			
			(115)				
			40				
		Ľ	↓				
			7	R	↑		
			И		175		
					(75)		
	(15)	(05)	(15)	_	٩Ľ	(15)	
	(15) 15	(85) 25	(15) 15	R	25 25	(15)	
	15	25 ✔	15	+	35 10	(35) (20)	
	(15)	▼ 15	7	Ľ	10	(20)	
	(13)	15	7	K 25	↑ 140	7 15	
	(40)	20	→	(15)	(60)	(15)	
	(23)	20	R	(13)	(00)	(13)	

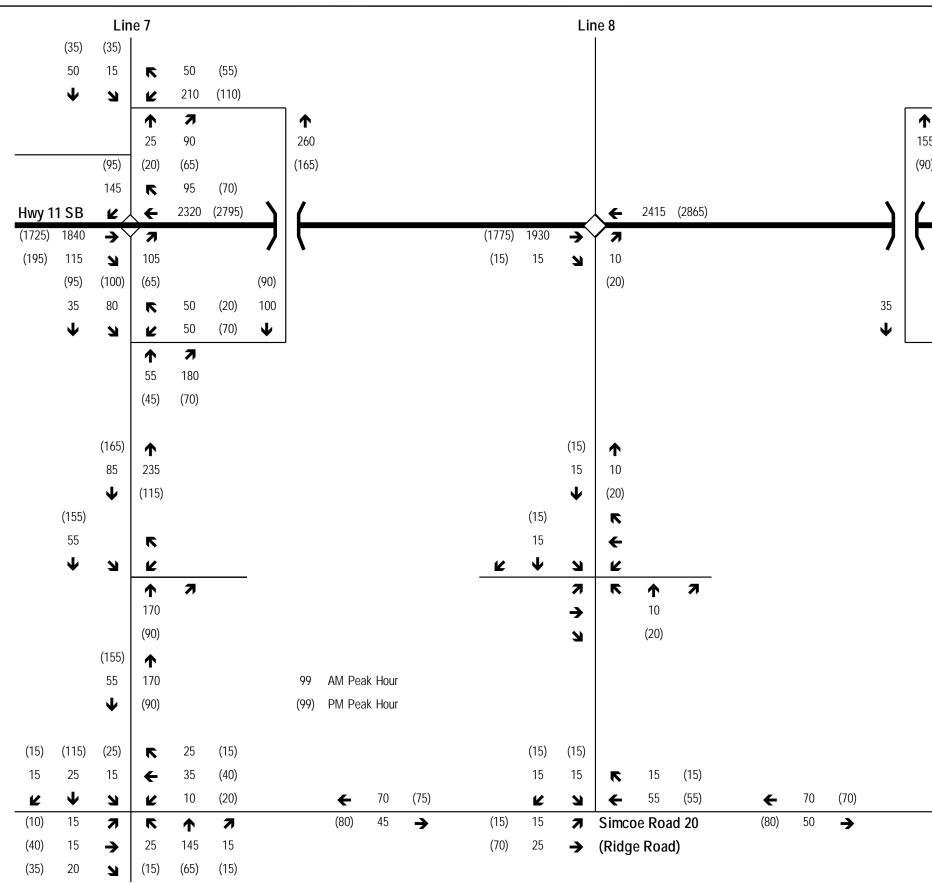


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Burl's Creek Event Grounds Traffic Impact Study

2025 Traffic Volumes - Saturday Peak Hours

_	_	_	Lin	e 9		
		(20)	(20)			
		15	40			
		Ľ	$\mathbf{\Phi}$			
	(25)	25	¥	K	↑	
75	(75)	150	R	25	10	
)0)			(95)	(65)	(15)	
			190	R	35	(65)
			Ľ	∖ ←	1525	(1700)
	(2500)	2745	→	7	Hwy	11 NB
	(120)	50	Ы	80		
		(40)	(80)	(50)		
		25	25			
		Ľ	$\mathbf{\Psi}$			
	(25)	15	7	R	↑	
	(55)	25	R	150	65	
				(65)	(25)	
			(135)	↑		
			50	215		
			$\mathbf{\Psi}$	(90)		
			(135)			
			50			
		Ľ	$\mathbf{\Psi}$			
			7	R	↑	
			R		215	
					(90)	
	(15)	(105)	(15)	R	25	(15)
	15	25	15	←	40	(40)
	Ľ	$\mathbf{\Psi}$	R	Ľ	10	(20)
	(15)	15	7	K	↑	7
	(45)	15	→	25	170	15
	(25)	20	R	(15)	(70)	(15)

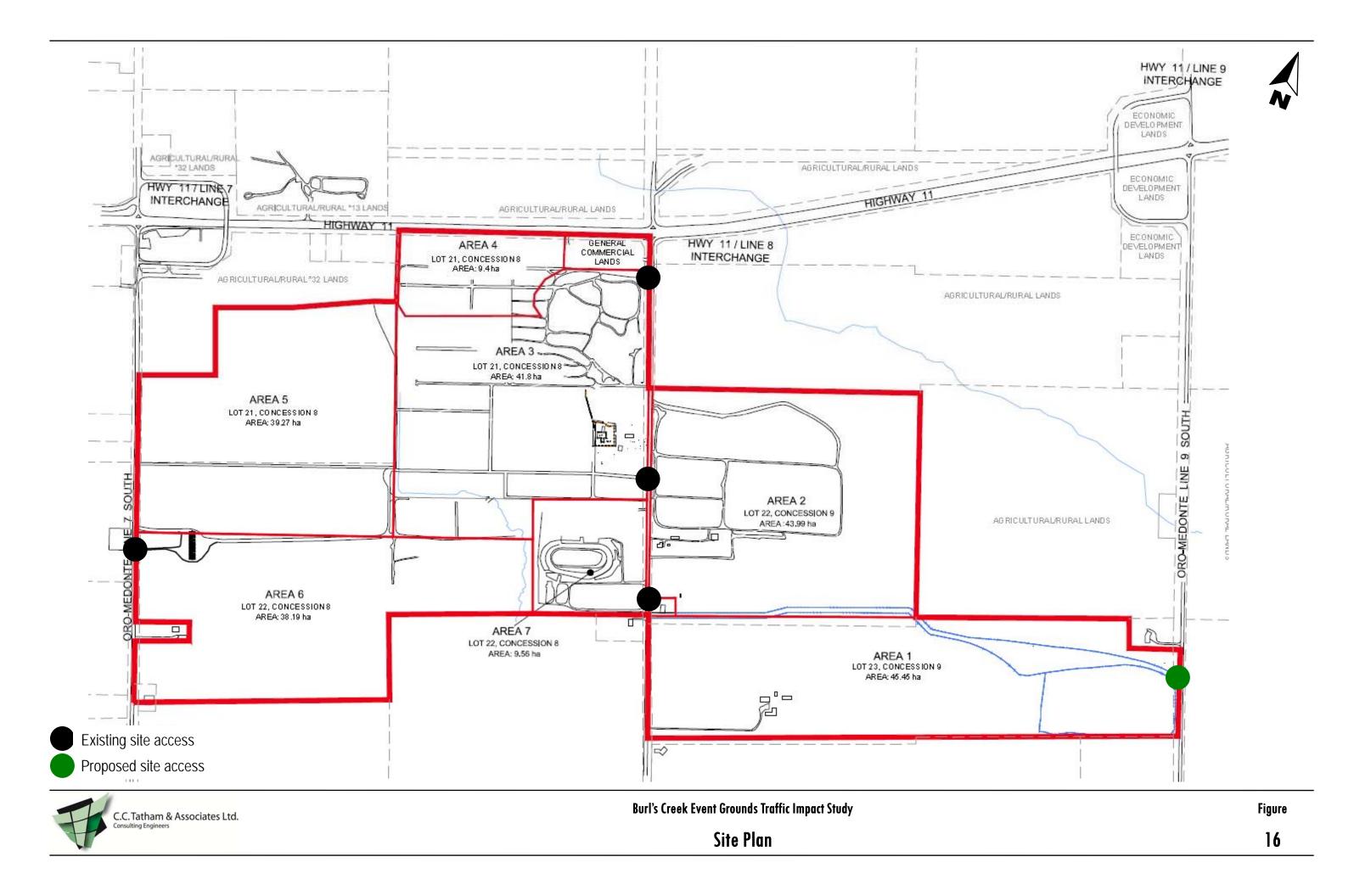


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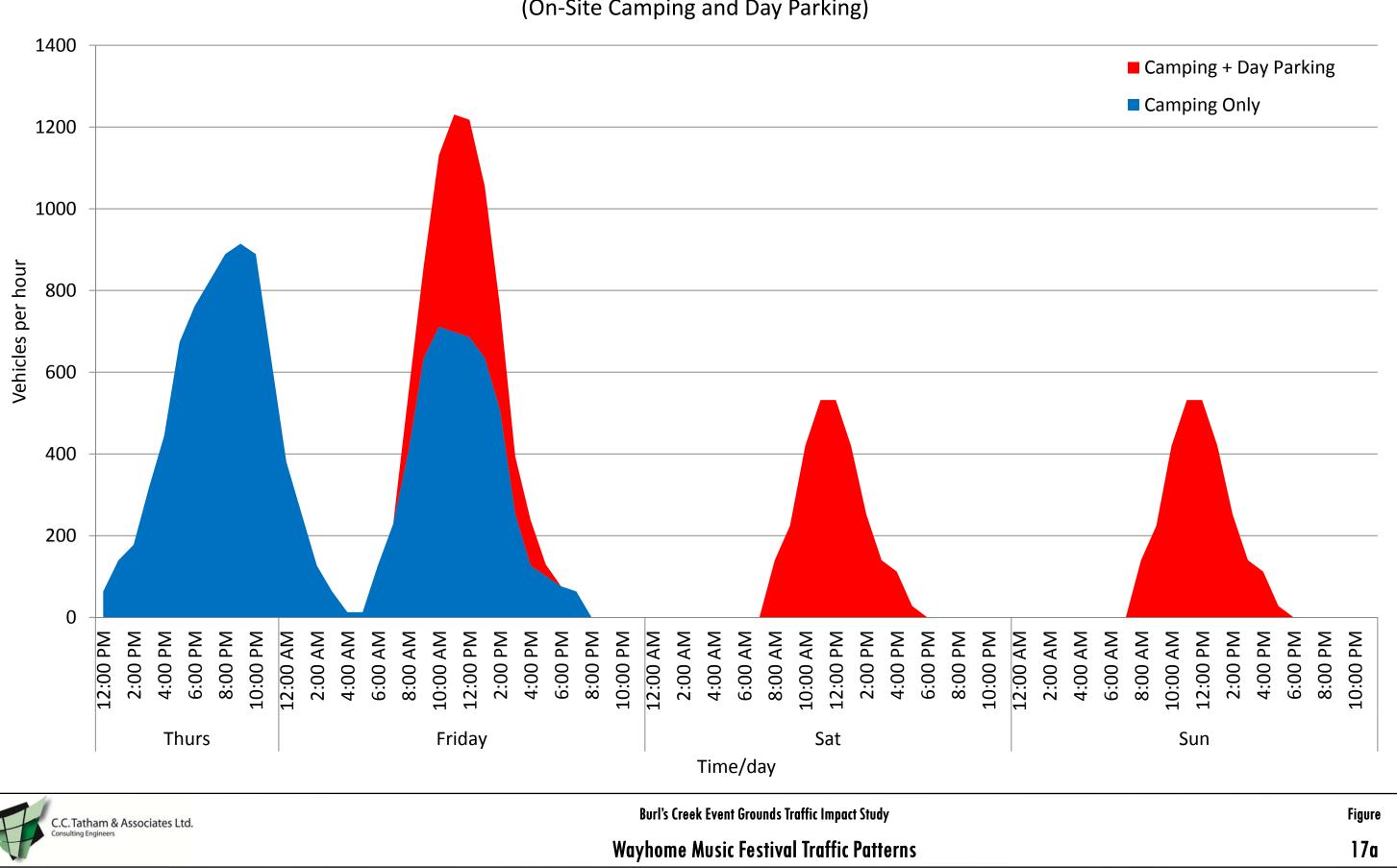
Burl's Creek Event Grounds Traffic Impact Study

2025 Traffic Volumes - Sunday Peak Hours

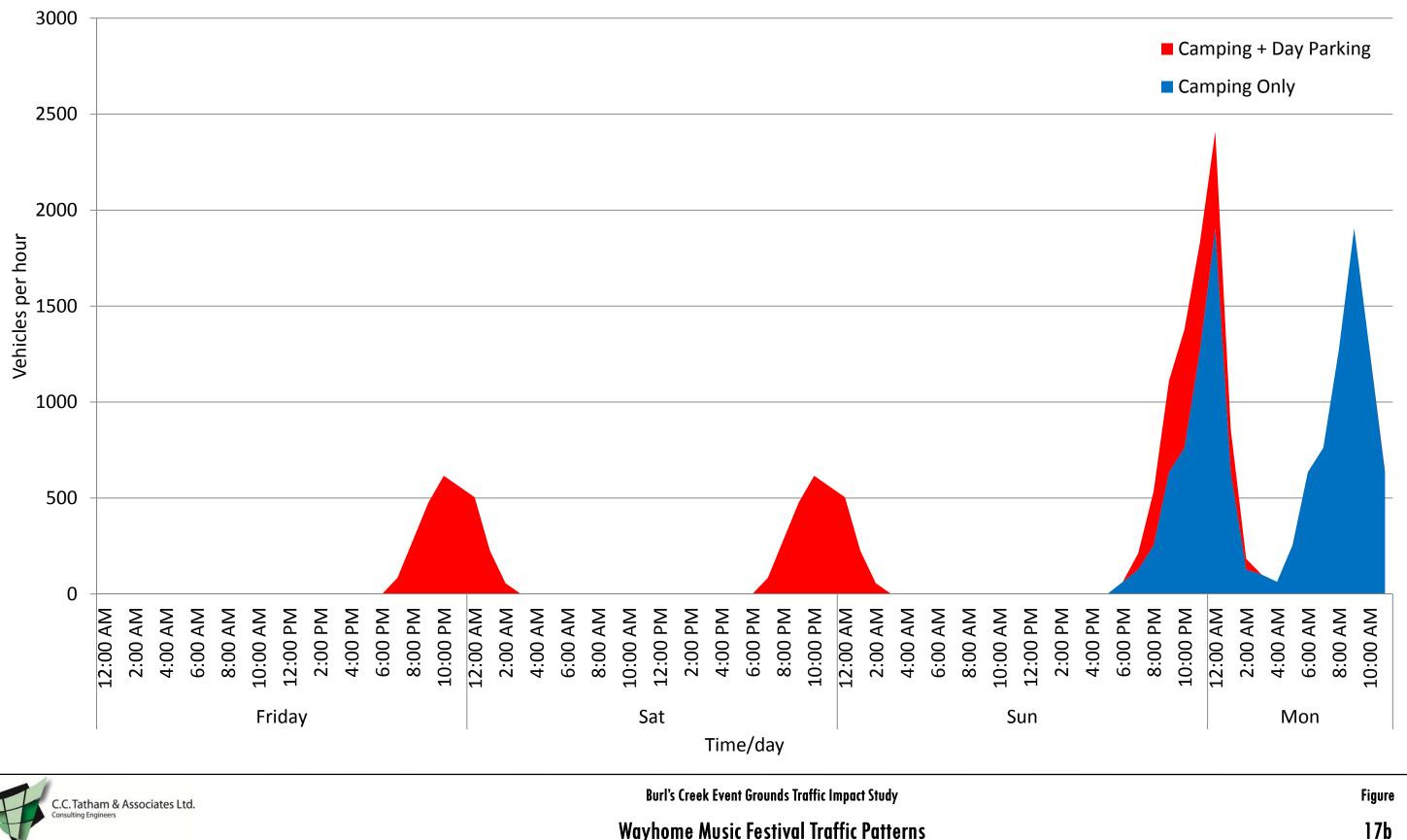
	Line 9									
		(15)	(20)	ie 9						
		15	(20) 40							
			40							
	(20)	20		_	•					
	(20)	20 125	7	K	↑					
55	(70)	135)	25 (FF)	10					
0)			(90)	(55)	(15)	(55)				
			175	K	35	(55)				
		1005	Ľ	+	2240	(2775)				
	(1685)	1895	→ `	7	Hwy	11 NB				
	(110)	45	J	70						
		(35)	(75)	(50)						
		25	20							
	()	Ľ	✦							
	(25)	15	7	K	1					
	(50)	20	R	135	55					
				(55)	(25)					
			(
			(125)	1						
			40	190						
			↓	(80)						
			(125)							
			40							
		Ľ	↓							
			7	K	↑					
			R		190					
					(80)					
	(15)	(0E)	(15)	_	٦ ٢	(15)				
	(15) 15	(95) 25	(15) 15	R	25 25	(15)				
	15	25	15	+	35 10	(35)				
	(15)	↓ 15	2	Ľ	10	(20)				
	(15) (40)	15	7	K 25	↑ 150	7 15				
	(40) (25)	20	→	25 (15)		(15)				
	(20)	20	R	(13)	(65)	(13)				



WayHome Event Arrivals (On-Site Camping and Day Parking)

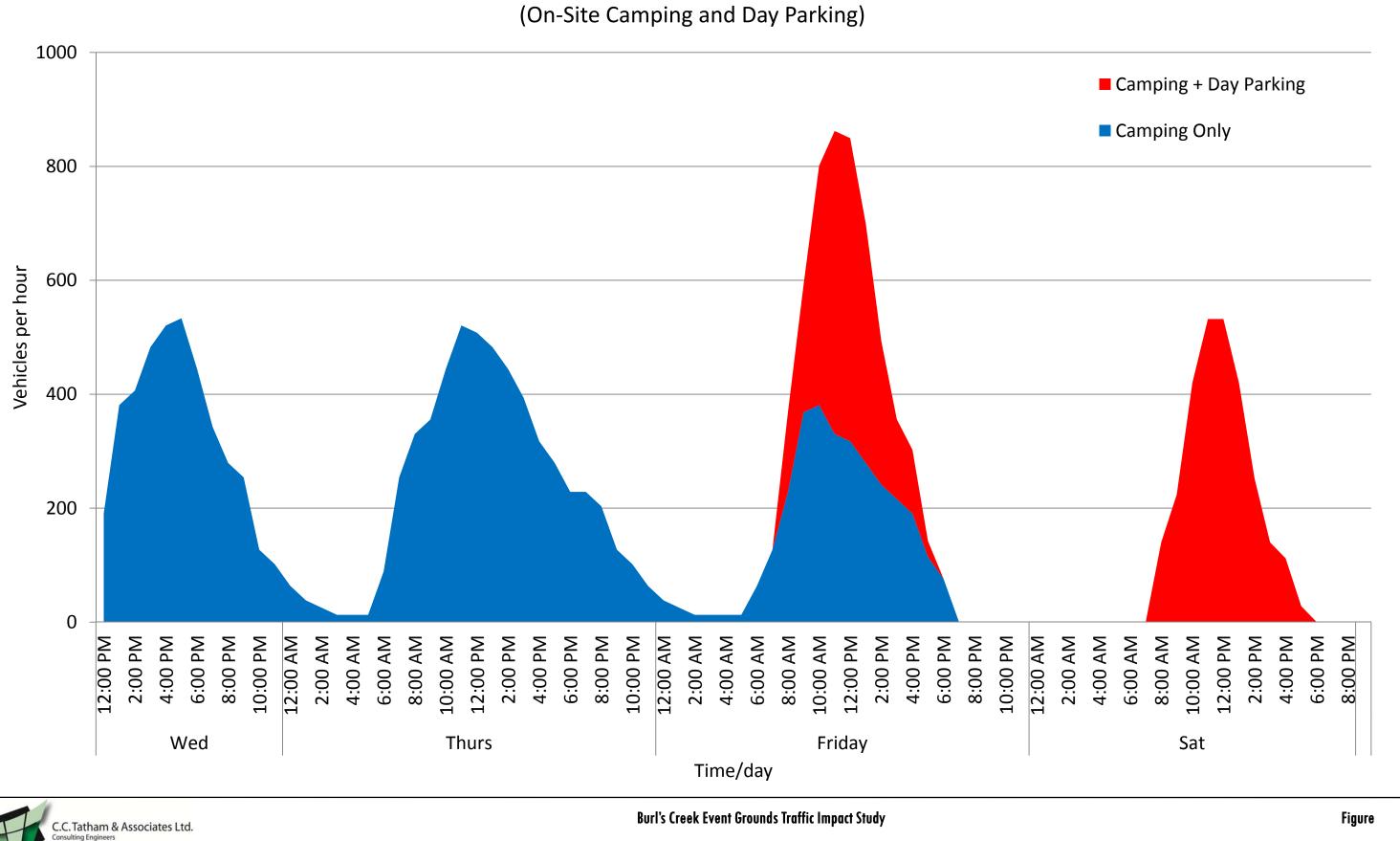


WayHome Event Exit (On-Site Camping and Day Parking)



Wayhome Music Festival Traffic Patterns

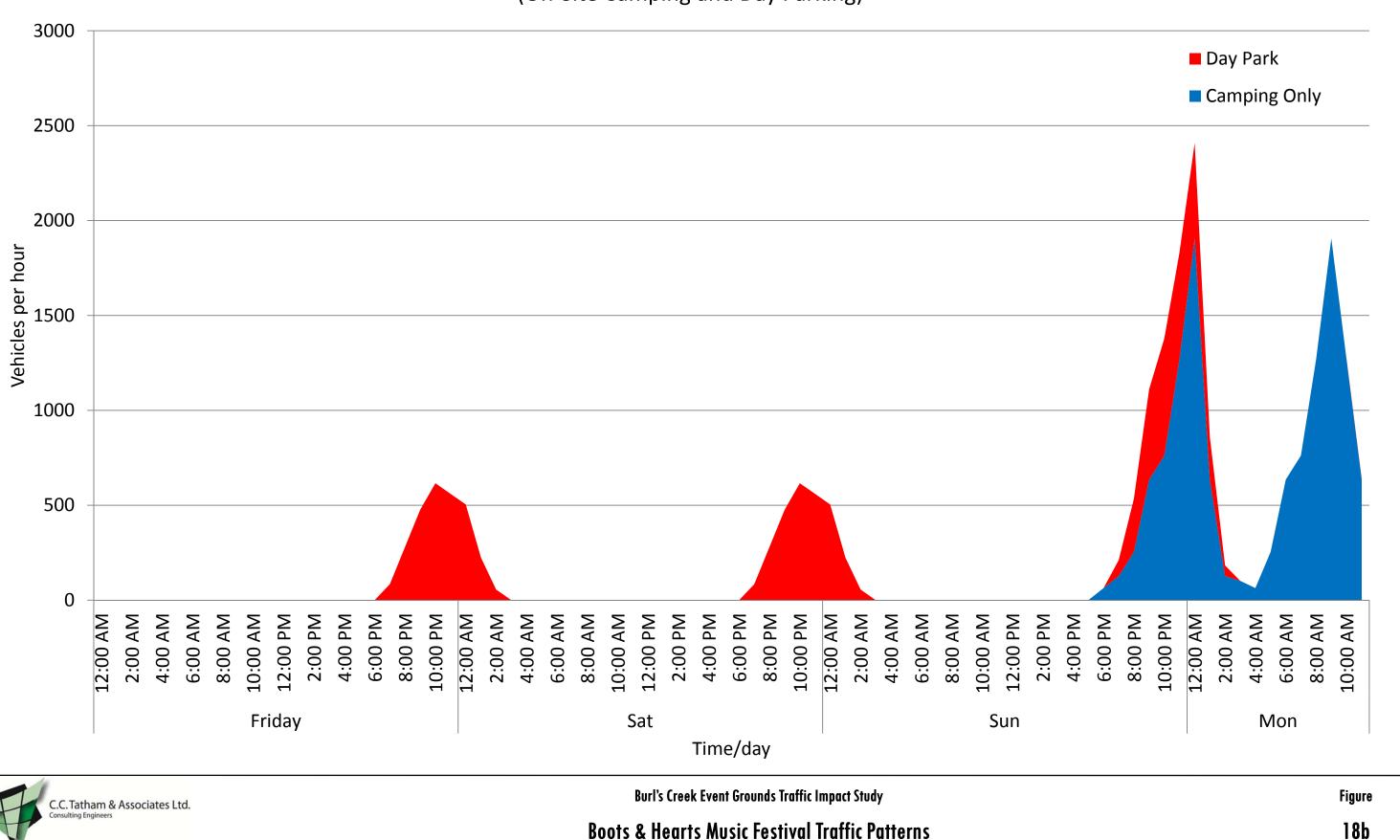
Boots & Hearts Arrivals



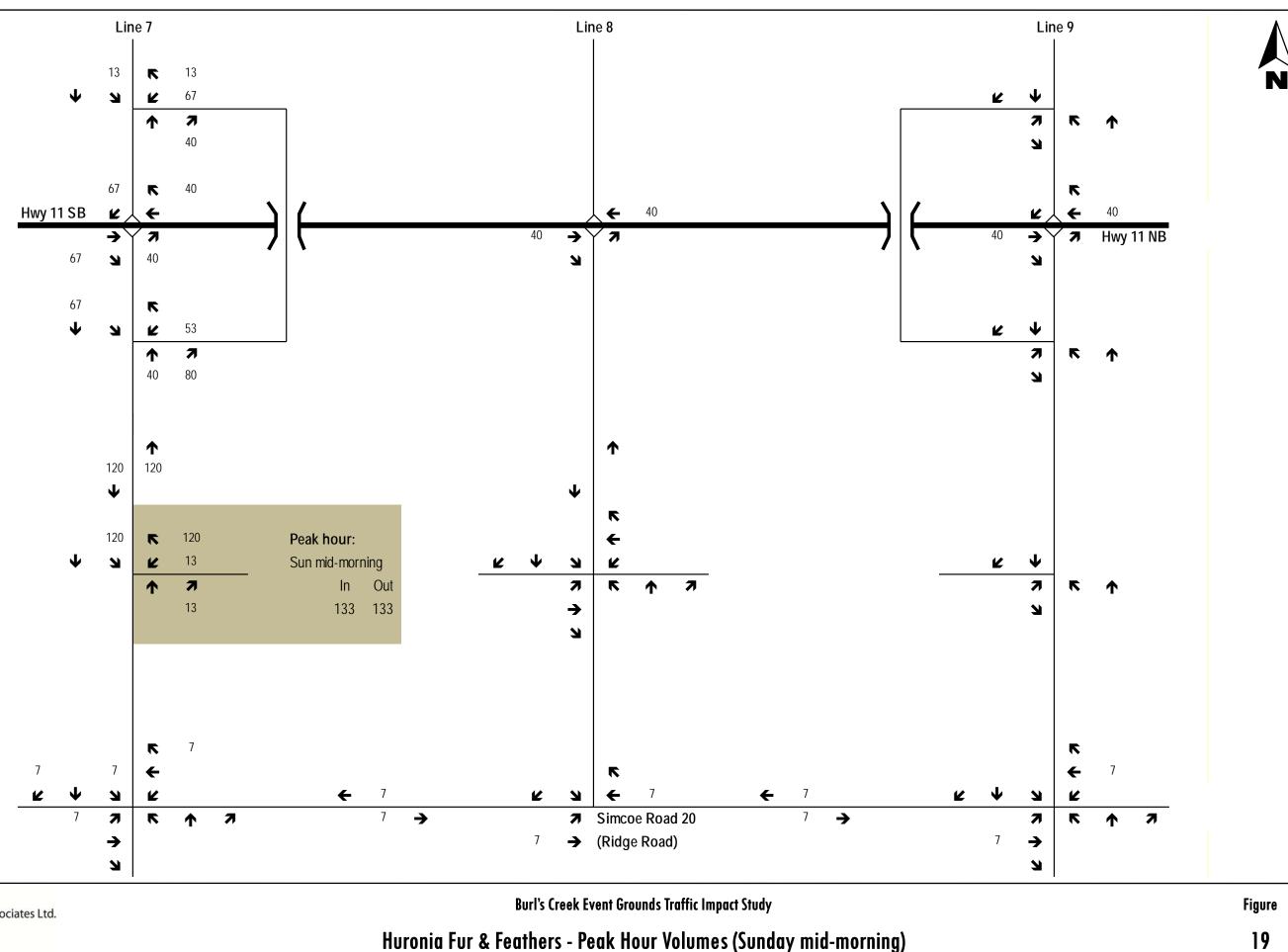
Boots & Hearts Music Festival Traffic Patterns

18a

Boots & Hearts Exit (On-Site Camping and Day Parking)

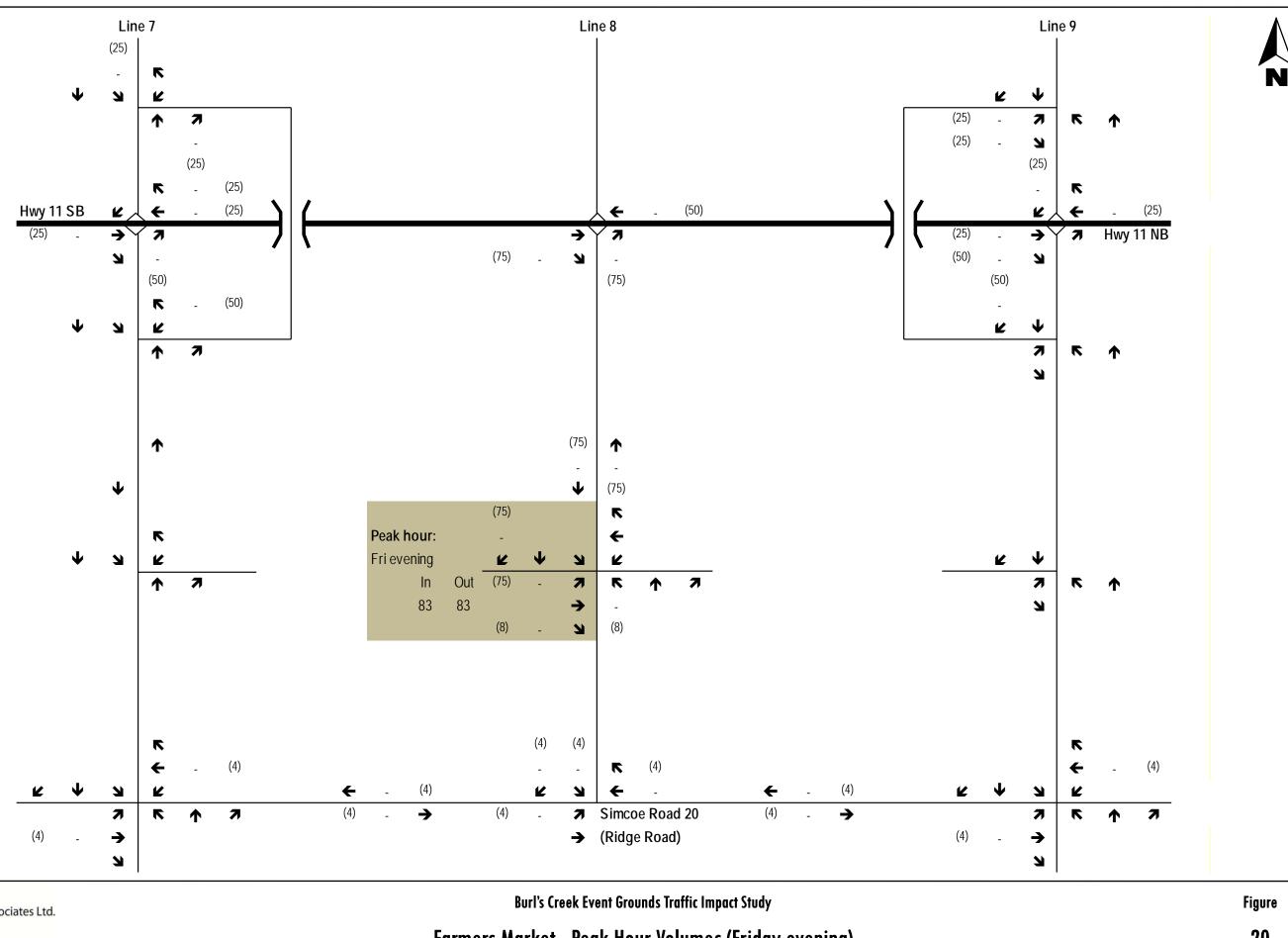


Boots & Hearts Music Festival Traffic Patterns



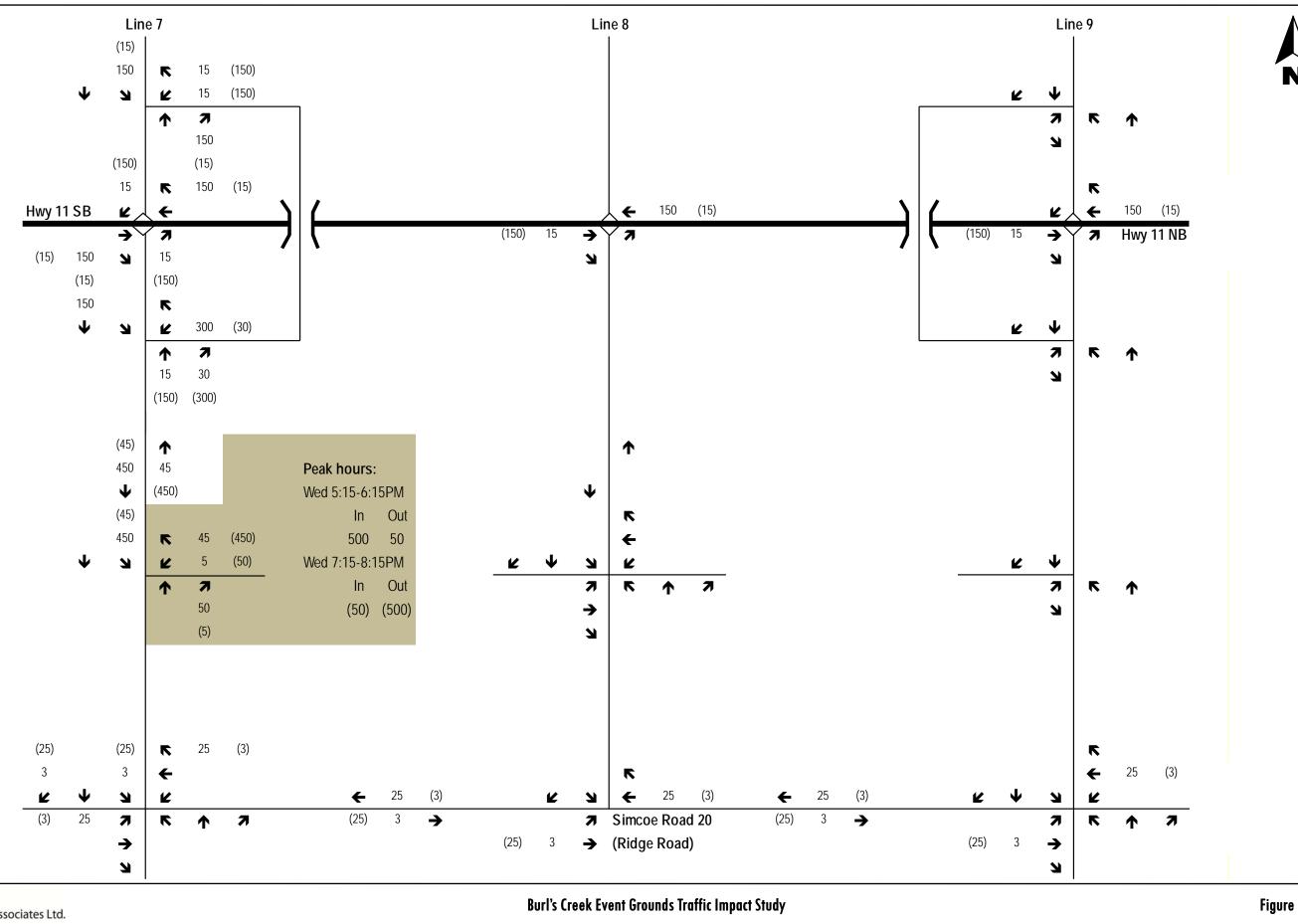


Huronia Fur & Feathers - Peak Hour Volumes (Sunday mid-morning)





Farmers Market - Peak Hour Volumes (Friday evening)

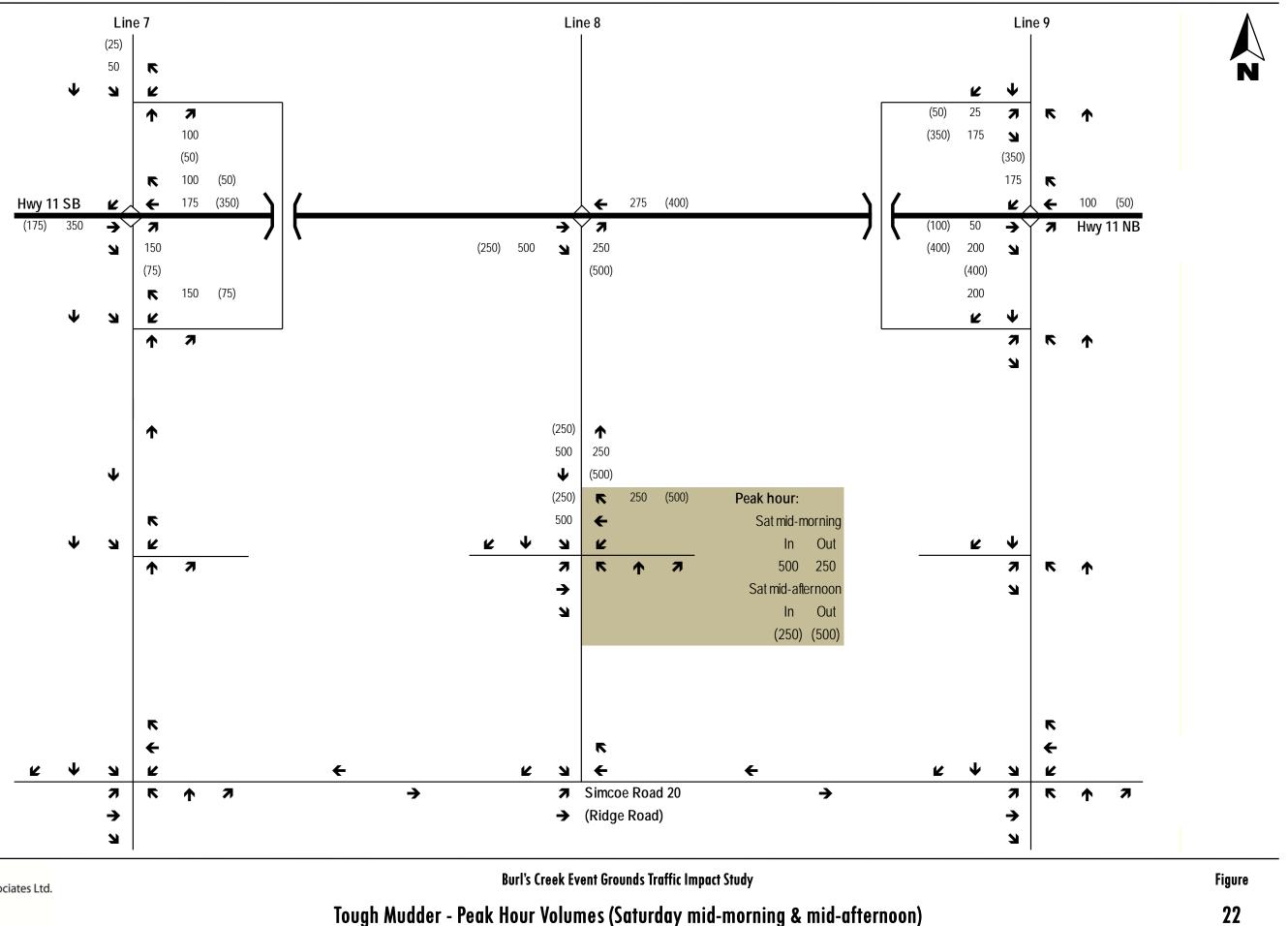




Minor Soccer - Peak Hour Volumes (Wednesday evening)

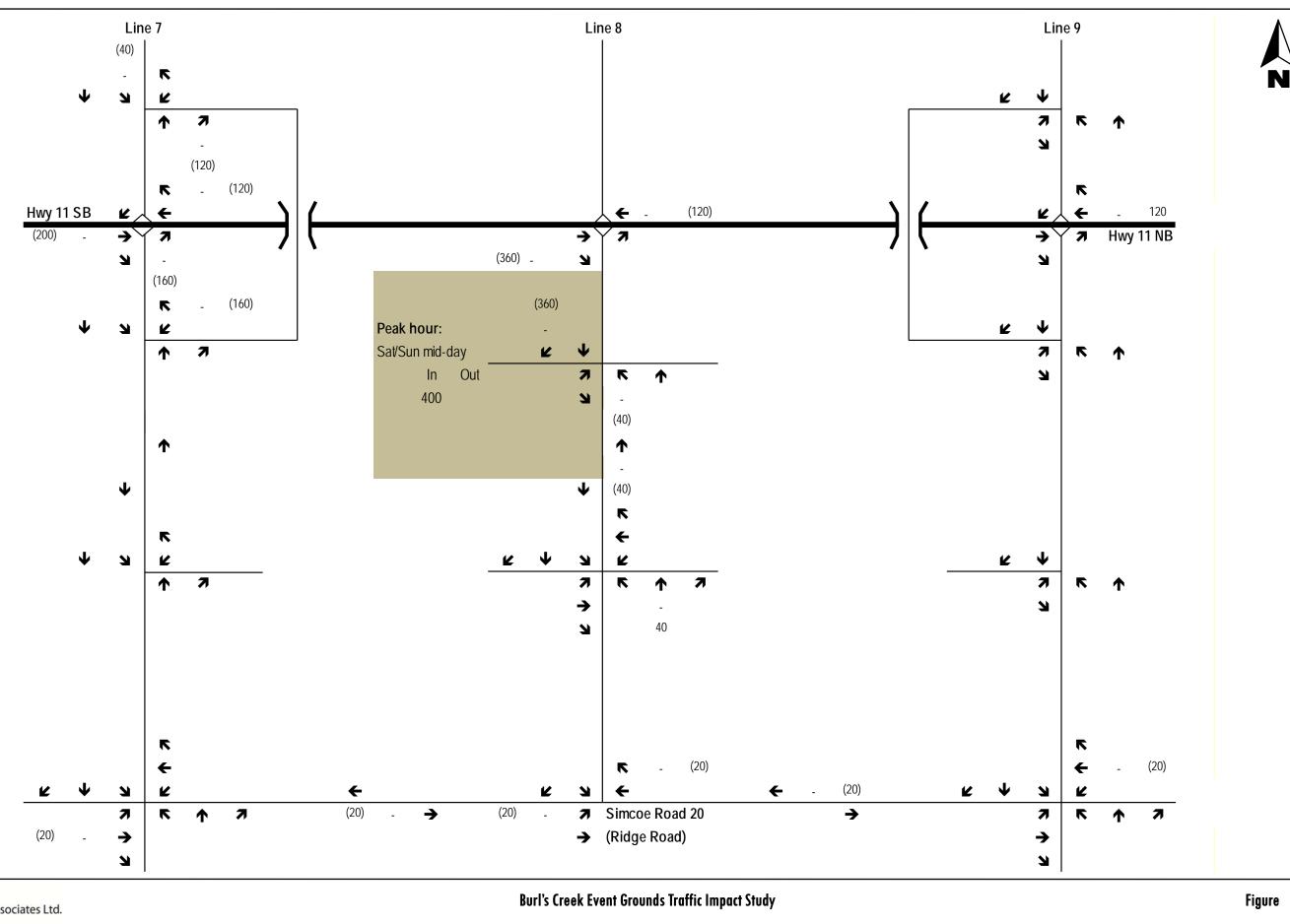
21

N



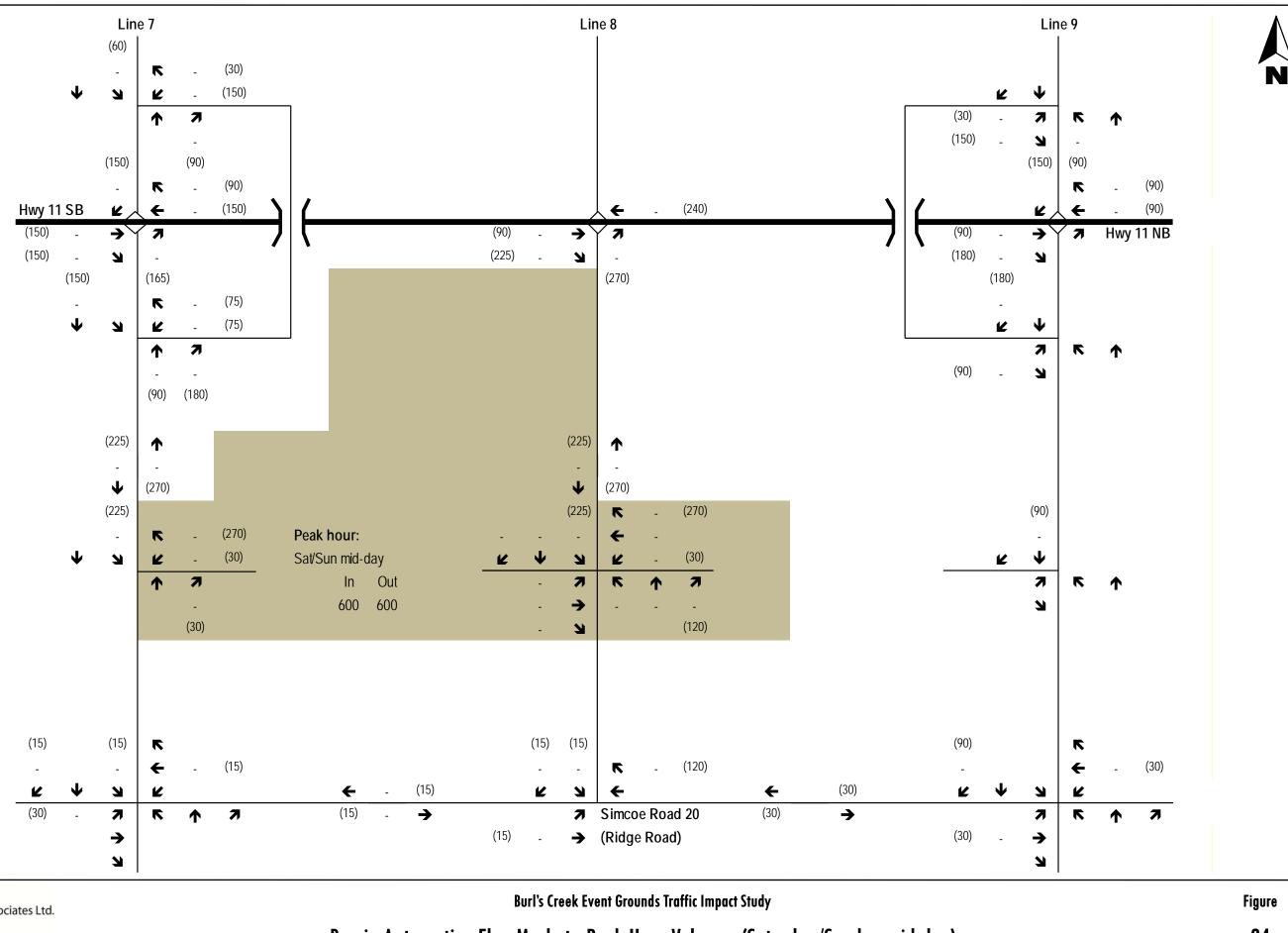
C.C. Tatham & Associates Ltd. Consulting Engineers

Tough Mudder - Peak Hour Volumes (Saturday mid-morning & mid-afternoon)



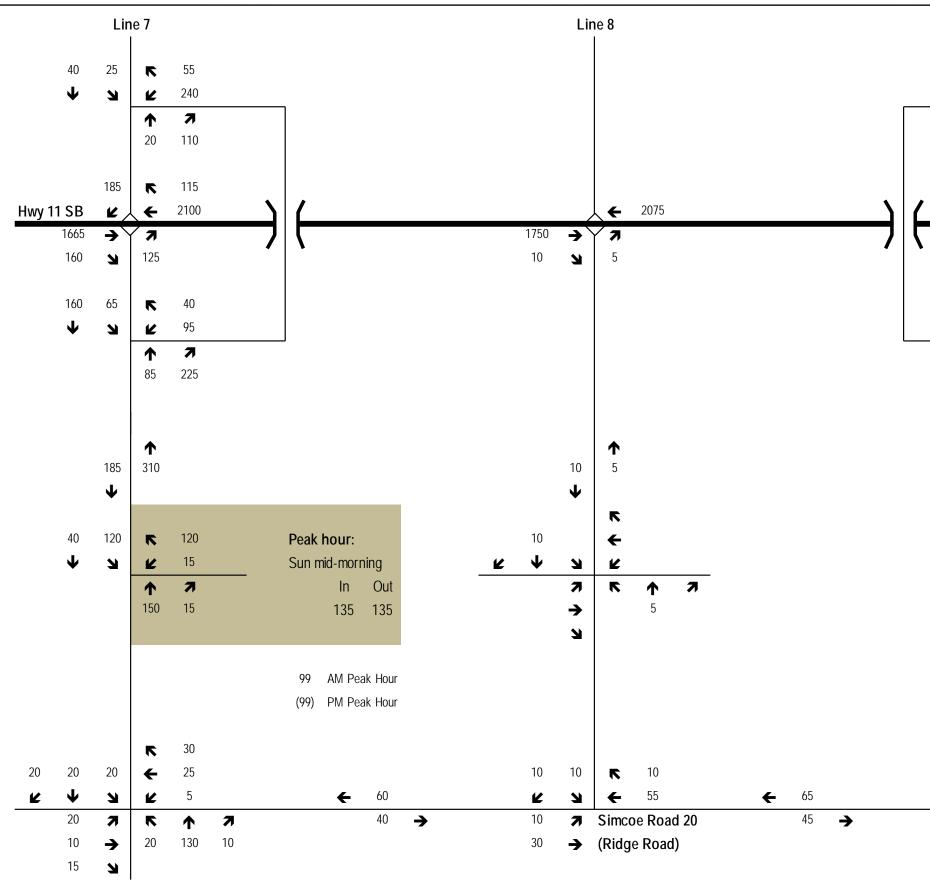
Contemporary Music & Camping Festival - Peak Hour Volumes (Saturday/Sunday mid-day)





C.C. Tatham & Associates Ltd. Consulting Engineers

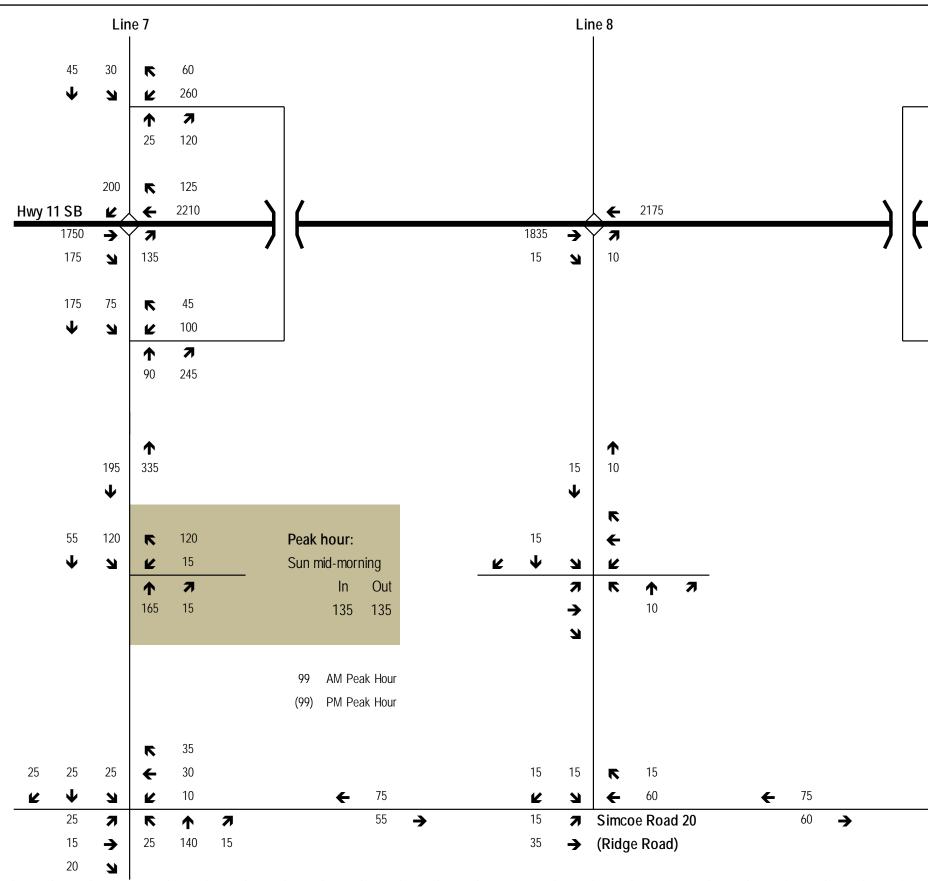
Barrie Automotive Flea Market - Peak Hour Volumes (Saturday/Sunday mid-day)





2015 Huronia Fur & Feathers - Peak Hour Volumes (Sunday mid-morning)

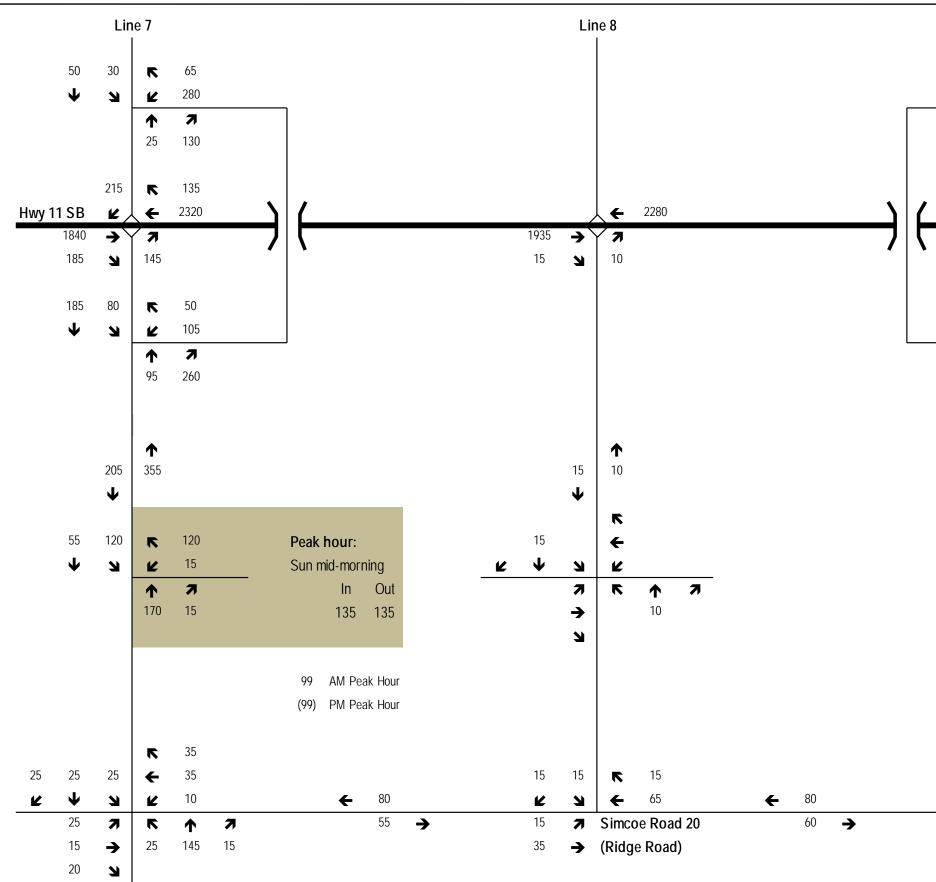
		Lir	ne 9 I		
	10	30			
	Ľ	$\mathbf{\Psi}$			
	15	7	⊼	↑	
	110	Ы	20	5	
		140	R	25	
		Ľ,	+	2075	
	1750	∢	7	Hwy	11 NB
	35	Ы	55		
	20	15			
	Ľ	$\mathbf{\Psi}$			
	10	7	⊼	♠	
	15	Ы	110	45	
		20	↑		
		30	155		
		¥			
		30			
	Ľ	$\mathbf{\Psi}$			
		7	⊼	↑	
		R		155	
			R	20	
10	20	10	←	35	
Ľ	$\mathbf{\Psi}$	R	Ľ	5	
	10	7	K	↑	7
	20	→	20	135	10
	15	R			





2020 Huronia Fur & Feathers - Peak Hour Volumes (Sunday mid-morning)

		Lir	ne 9		
	15	35			
	Ľ	$\mathbf{\Psi}$			
	20	7	7	↑	
	125	R	25	10	
		160	R	35	
		Ľ,	+	2175	
	1835	→`	7	Hwy	11 NB
	45	R	65		
	25	20			
	Ľ	$\mathbf{\Psi}$			
	15	7	7	♠	
	20	R	125	50	
		40 ✔	↑ 175		
		40			
	Ľ	¥			
		7	7	↑ 175	
15	25	15	+ ۲	25 40	
Ľ	$\mathbf{\Psi}$	Ы	Ľ	10	
_	15	7	r	↑	7
	25	→	25	145	15
	20	R			

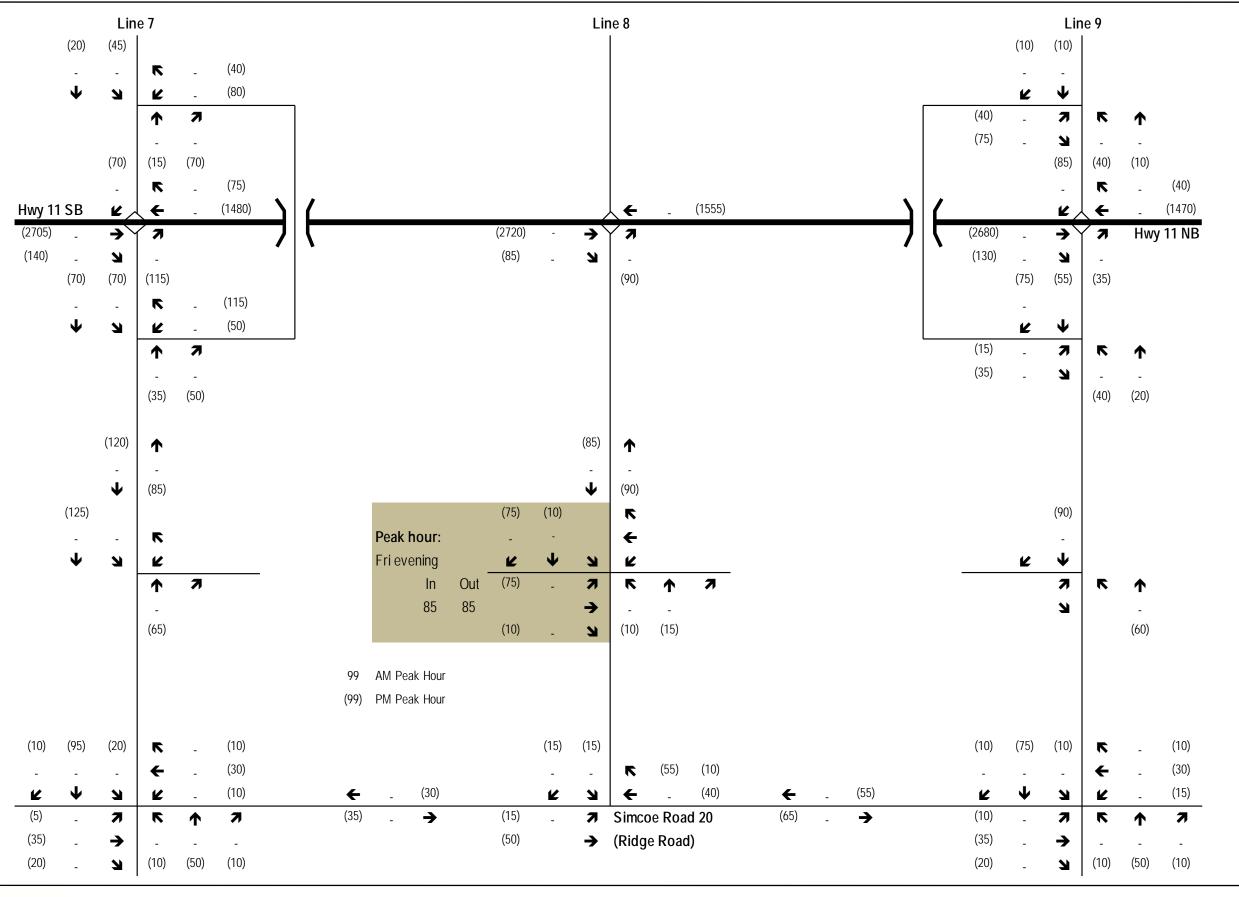




2025 Huronia Fur & Feathers - Peak Hour Volumes (Sunday mid-morning)

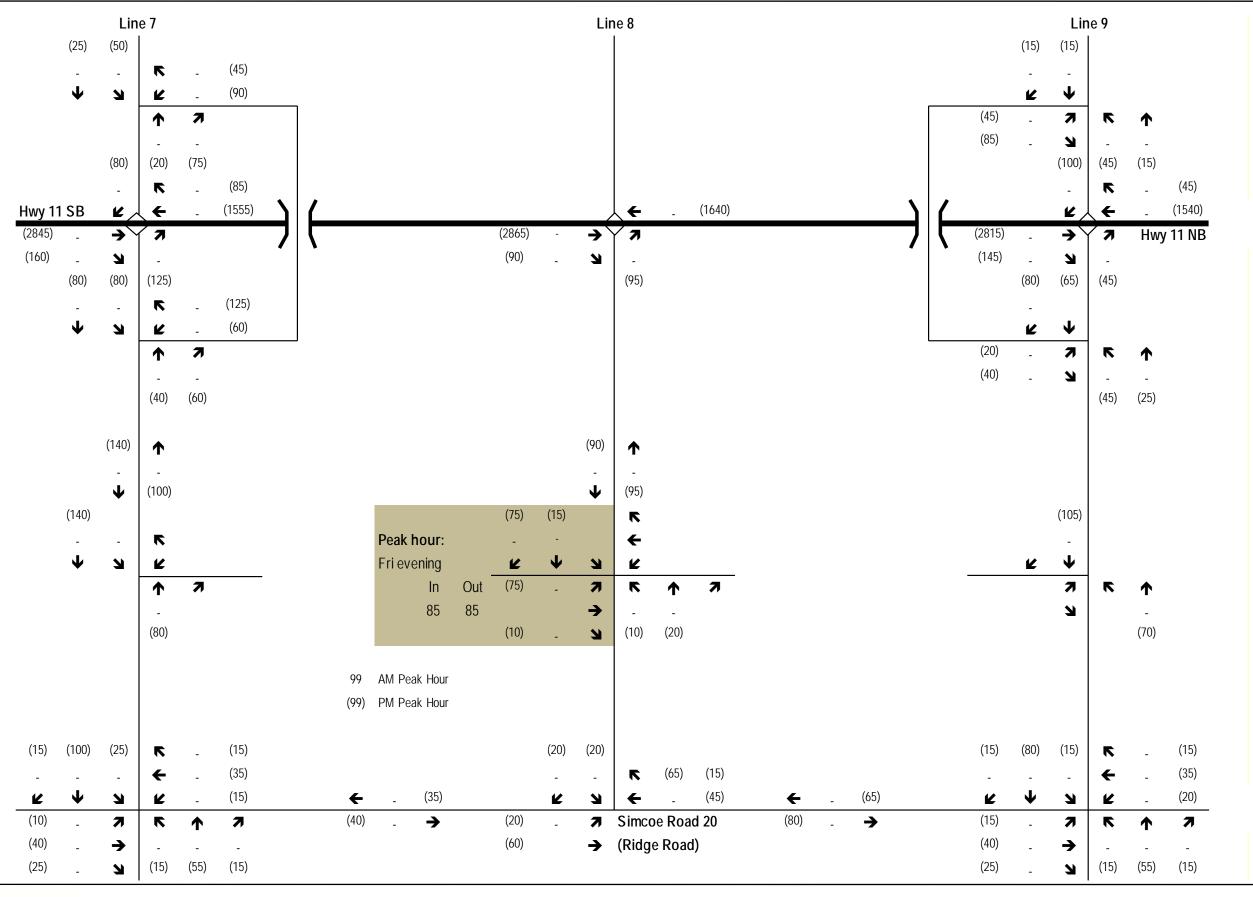
		Lir	ne 9		
	15	40			
	Ľ	$\mathbf{\Psi}$			
	20	7	7	↑	
	135	Ы	25	10	
		175	⊼	35	
		Ľ	∖ ←	2280	
	1935	→ \	7	Hwy 11 NB	
	45	J	70		
	25	20			
	15	•	_		
	15 20	7	125		
	20	R	135	55	
		40	↑ 190		
		40 ₩	170		
		•			
		40			
	Ľ	¥			
	-	7	7	↑	
		N		Τ 190	
		-			
			⊼	25	
15	25	15	←	45	
 Ľ	$\mathbf{\Psi}$	Ы	Ľ	10	
	15	7	R	↑ 7	
	25	→	25	150 15	
	20	Ы			

N



2015 Farmers Market - Peak Hour Volumes (Friday evening)





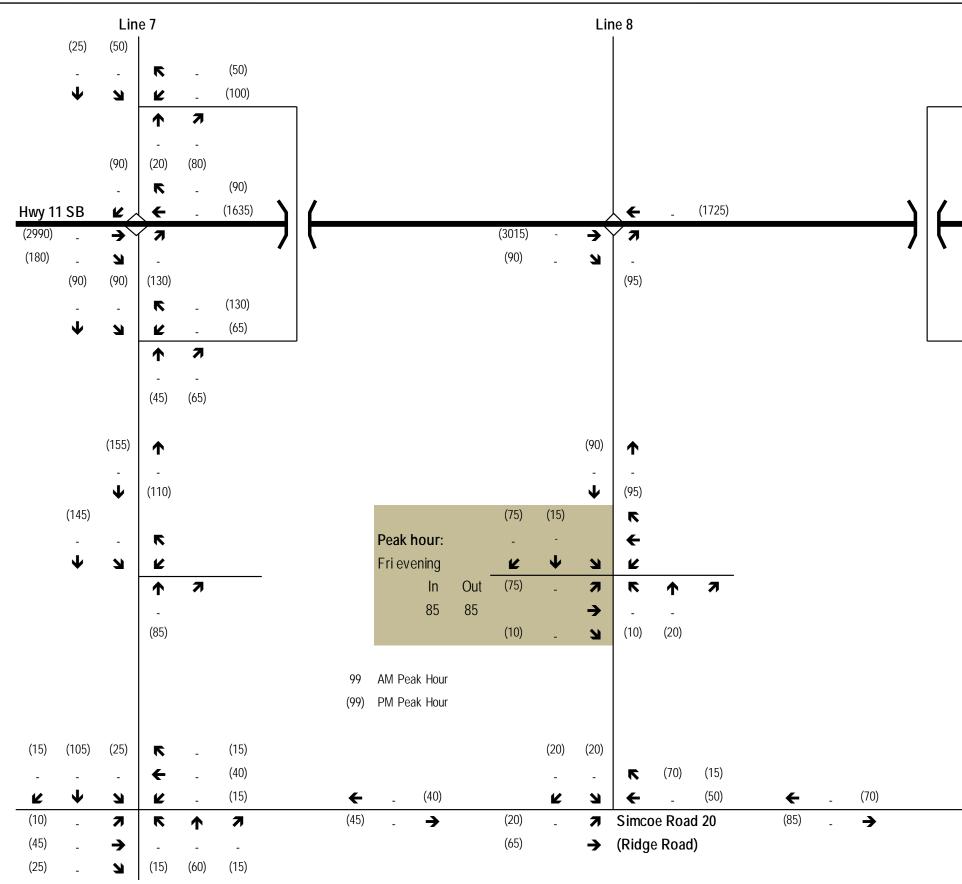
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Burl's Creek Event Grounds Traffic Impact Study

2020 Farmers Market - Peak Hour Volumes (Friday evening)

Figure

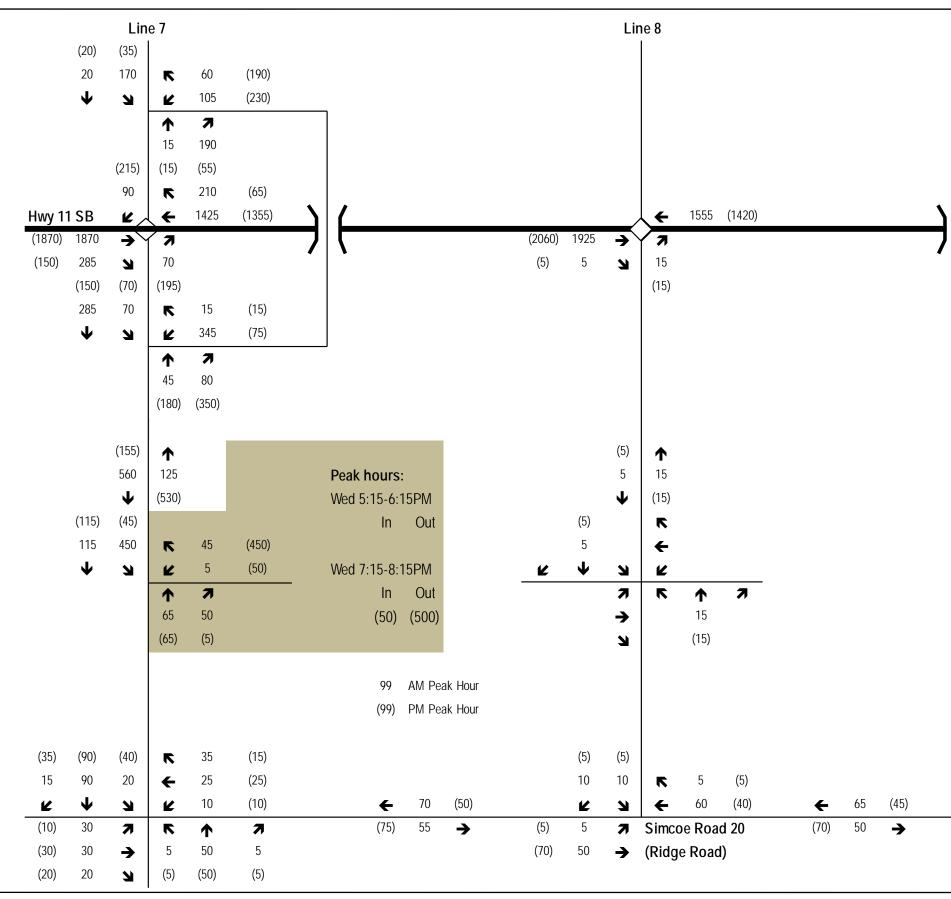
N



2025 Farmers Market - Peak Hour Volumes (Friday evening)



20)
NB
5)
•
5)
N 5) 0) 0)

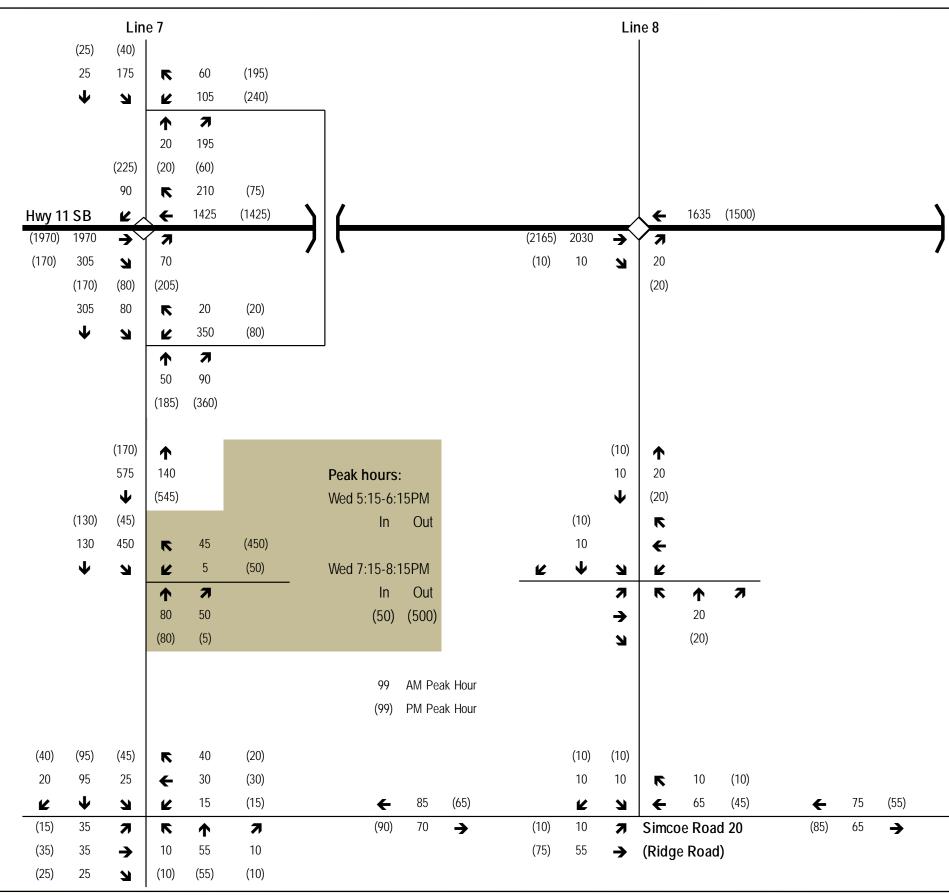


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2015 Soccer - Peak Hour Volumes (Wednesday evening)

			Lir	ne 9		
		(10)	(10)			
		10	10			
		Ľ	$\mathbf{\Psi}$			
	(15)	15	7	7	↑	
	(50)	50	R	40	10	
			(60)	(40)	(10)	
			75	R	40	(40)
			Ľ,	+	1495	(1360)
$ \subset $	(2000)	1865	→	7	Hwy	11 NB
•	(75)	75	Ы	30		
		(25)	(50)	(30)		
		25	50			
		Ľ	$\mathbf{\bullet}$			
	(15)	15	7	7	↑	
	(35)	35	Ы	40	15	
				(40)	(15)	
		Ľ	(85) 85 ✔			
			7	7	•	
			Я		Т 55 (55)	
	(5) 5	(70) 70	(5) 5	۲ ۲	5 45	(5) (25)
	Ľ	$\mathbf{\Psi}$	Ы	Ľ	10	(10)
	(5)	5	7	R	↑	7
	(50)	30	→	5	45	5



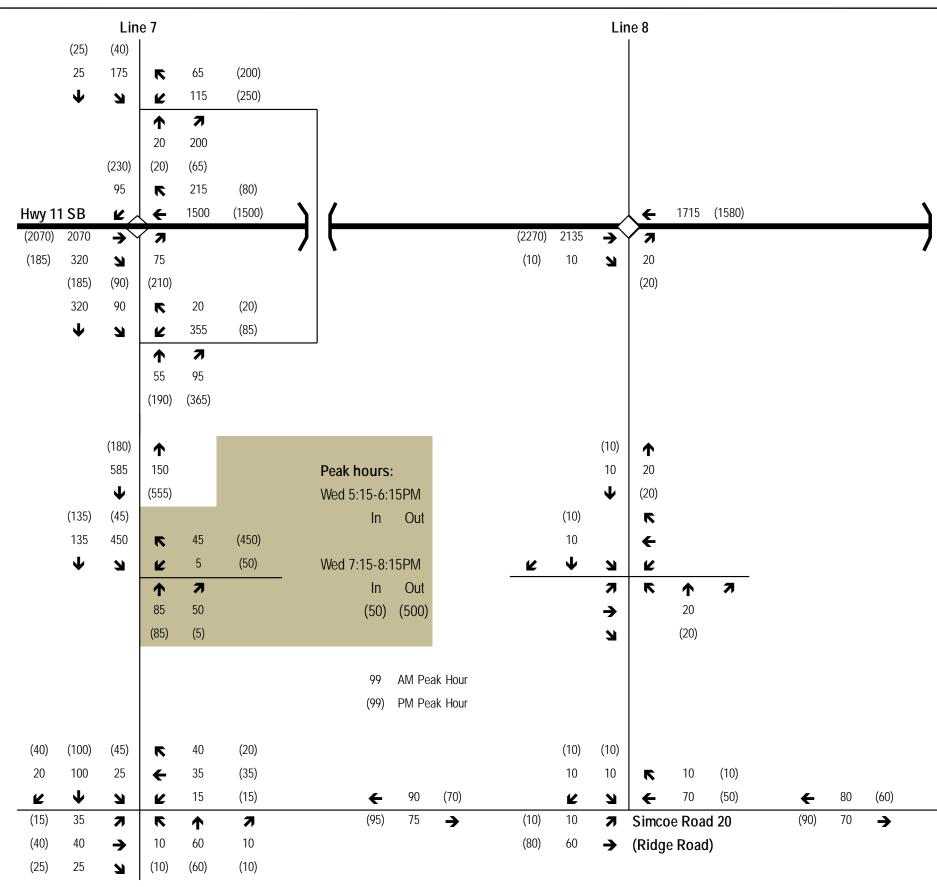


C.C. Tatham & Associates Ltd. Consulting Engineers Burl's Creek Event Grounds Traffic Impact Study

2020 Soccer - Peak Hour Volumes (Wednesday evening)

N

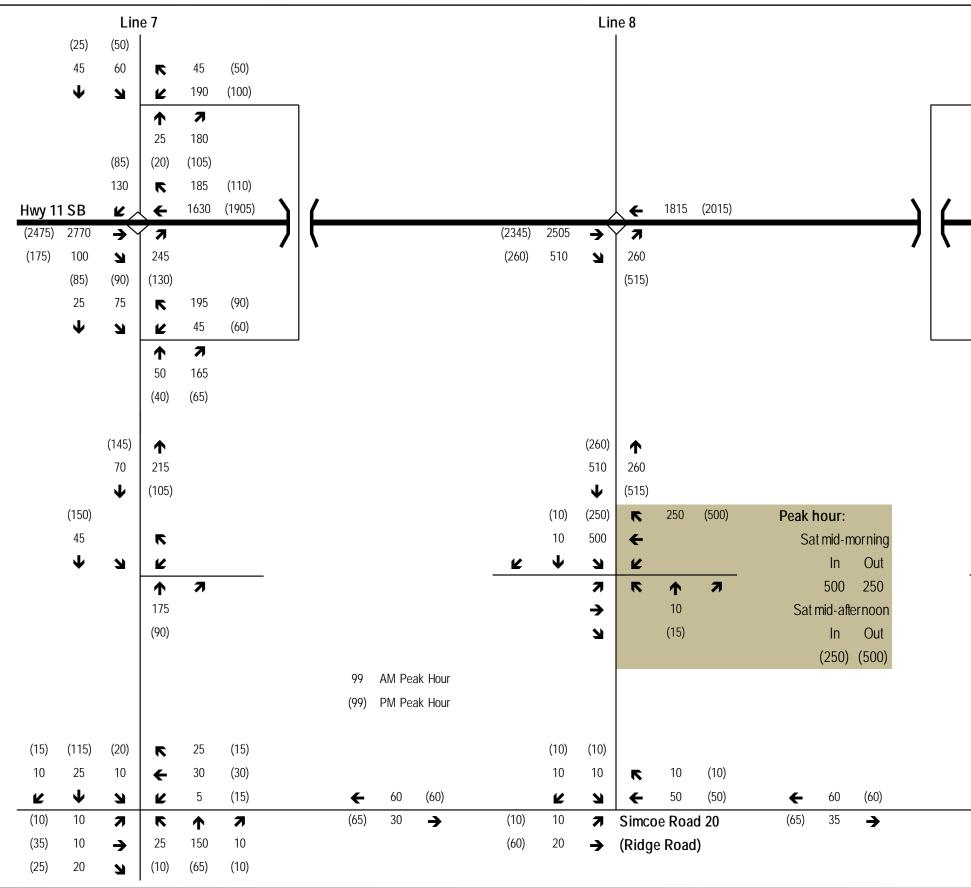
		Lin	ie 9			
	(15)	(15)				
	15	15				
	Ľ	↓				
(20)	20	7	K	↑		
(60)	60	R	45	15		
		(75)	(45)	(15)		
		75	K	45	(45)	
		Ľ	+	1560	(1425)	
(2095)	1960	→	7	Hwy	11 NB	
(90)	90	R	40			
	(30)	(60)	(40)			
	30	60				
	Ľ	↓				
(20)	20	7	K	↑		
(40)	40	Я	45	20		
			(45)	(20)		
		(100)				
		100				
	K	↓				
		¥	R	↑		
		Ы		65		
				(65)		
(10)	(75)	(10)	R	10	(10)	
10	75	10	←	50	(30)	
 Ľ	$\mathbf{\Psi}$	Ы	Ľ	15	(15)	
(10)	10	7	K	↑	7	
(55)	35	→	10	50	10	
(20)	20	R	(10)	(50)	(10)	
			-		· · ·	





2025 Soccer - Peak Hour Volumes (Wednesday evening)

			Lin	ne 9		
		(15)	(15)			
		15	15			
		Ľ	¥			
	(20)	20	7	R	↑	
	(65)	65	Ы	50	15	
			(80)	(50)	(15)	
•			80	R	50	(50)
			Ľ	∖ ←	1635	(1500)
	(2190)	2055	→ `	7	Hwy	11 NB
	(100)	100	R	40		
		(35)	(65)	(40)		
		35	65			
		Ľ	$\mathbf{\Lambda}$			
	(20)	20	7	R	↑	
	(45)	45	Ы	50	20	
				(50)	(20)	
		Ľ	(110) 110 ♥			
			7	ĸ	↑ 70 (70)	
	(10)	(80)	(10)	ĸ	10	(10)
	10	80	10	←	50	(30)
	Ľ	$\mathbf{\Psi}$	R	Ľ	15	(15)
	(10)	10	7	K	↑	7
	()	40	→	10	50	10
	(60)	40	-			

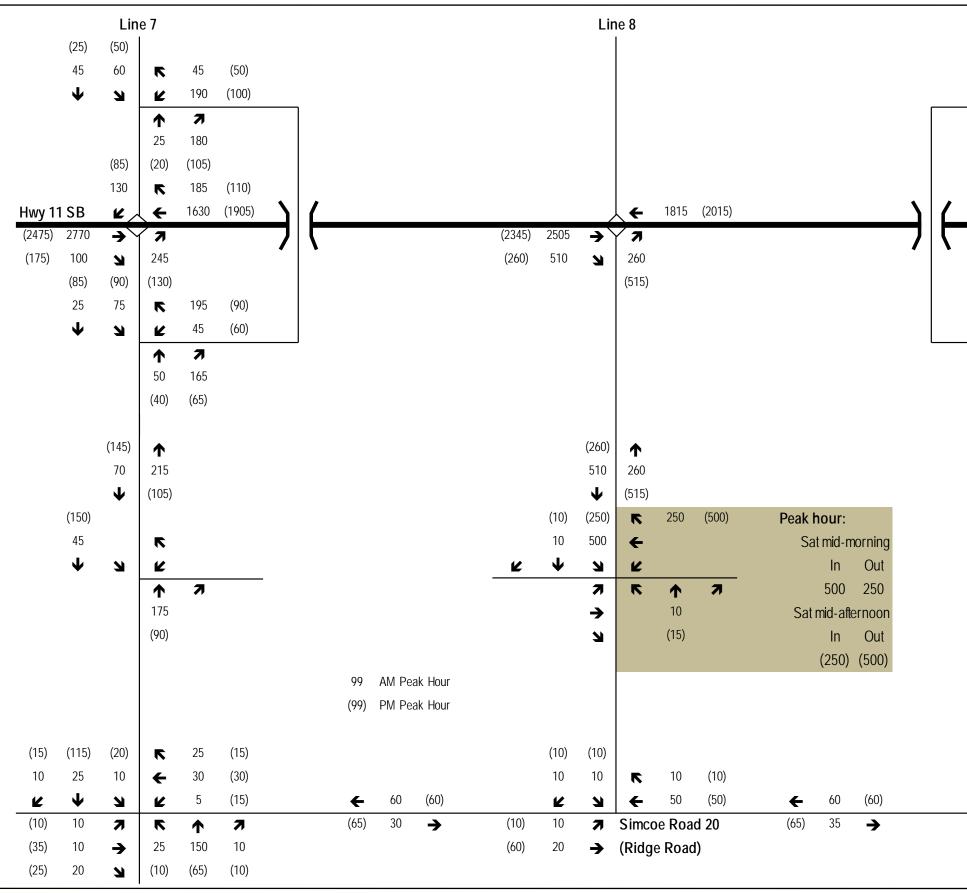


2015 Tough Mudder - Peak Hour Volumes (Saturday mid-morning & mid-afternoon)



		Lin	e 9		
	(15)	(15)			
	10	30			
	Ľ	$\mathbf{\Psi}$			
(70)	45	7	R	♠	
(410)	295	R	20	5	
		(425)	(50)	(10)	
		325	R	25	(50)
		Ľ	` +	1490	(1590)
(2365)	2525	→	7	Hwy	' 11 NB
(495)	240	R	60		
	(480)	(65)	(40)		
	340	20			
	Ľ	$\mathbf{\Psi}$			
(20)	10	7	R	♠	
(45)	20	Ы	120	50	
			(50)	(20)	
		(110)			
		40			
	Ľ	¥			
		7	R	↑	
		R		170	
				(70)	
(10)	(OF)	(10)	-	20	(10)
(10) 10	(95) 20	(10) 10	R	20 30	(10) (30)
	20 ✔		+	30 5	(30)
(10)	▼ 10	2	Ľ		(15)
(35)	10	7	K 20	个 150	7 10
	10	→	20	100	10
(20)	15	R	(10)	(60)	(10)

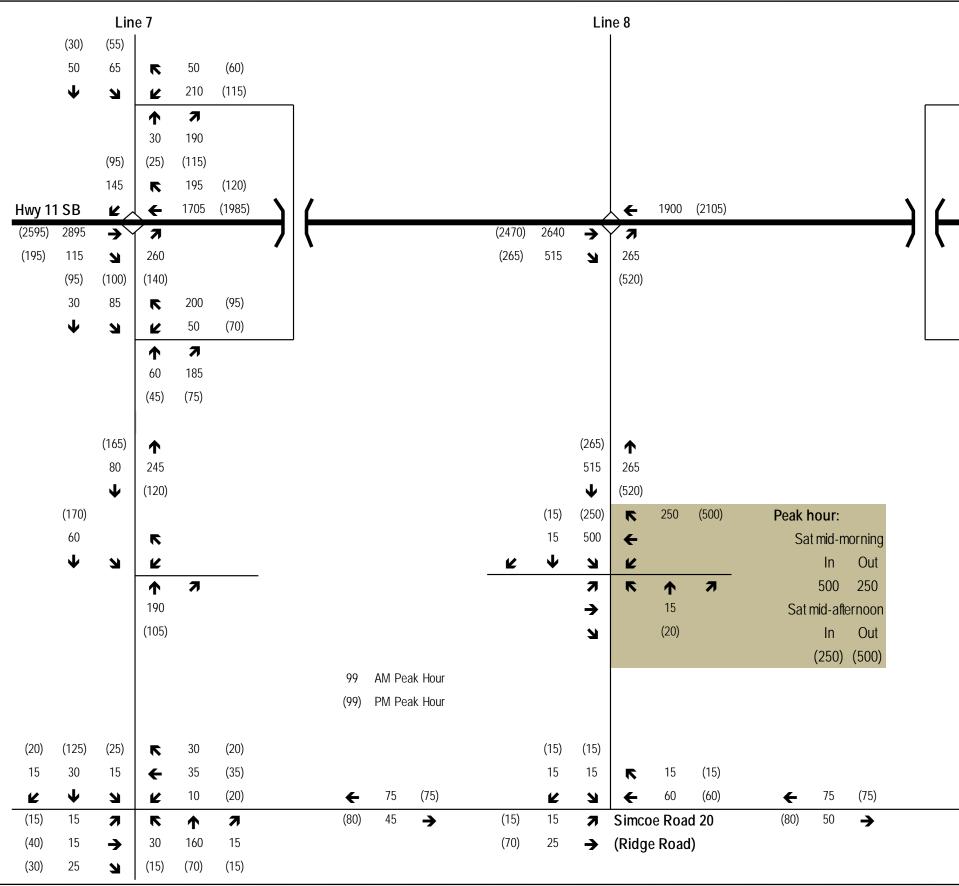
Figure



2020 Tough Mudder - Peak Hour Volumes (Saturday mid-morning & mid-afternoon)



		l in	ie 9		
	(15)	(15)			
	10	30			
	Ľ	$\mathbf{\Psi}$			
(70)	45	7	R	♠	
(410)	295	R	20	5	
		(425)	(50)	(10)	
		325	R	25	(50)
		Ľ	`	1490	(1590)
(2365)	2525	→	7	Hwy	11 NB
(495)	240	R	60		
	(480)	(65)	(40)		
	340	20			
	Ľ	↓			
(20)	10	7	r	↑	
(45)	20	R	120	50	
			(50)	(20)	
		(110)			
		40			
	K	40 ♥			
		▼ 7	R	•	
		X	**	170	
		-		(70)	
				,	
(10)	(95)	(10)	R	20	(10)
10	20	10	÷	30	(30)
Ľ	$\mathbf{\Psi}$	R	K	5	(15)
(10)	10	7	R	↑	7
	10	→	20	150	10
(35)	10	7			

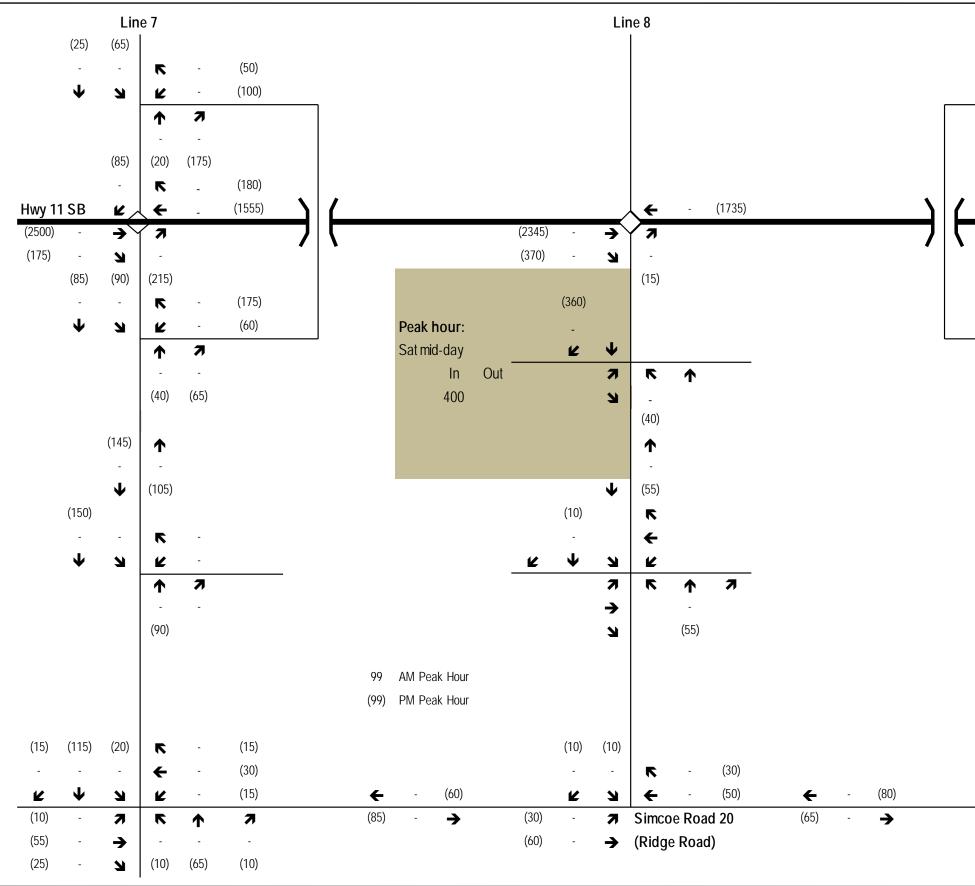


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2025 Tough Mudder - Peak Hour Volumes (Saturday mid-morning & mid-afternoon)

Line 9 (20) (20) 15 35 $\mathbf{\Psi}$ K (75) 50 7 R ♠ (420) 310 25 10 N (440) (60) (15) 35 345 (60) R 1555 (1665) K ← Hwy 11 NB 2655 (2480) → 7 75 250 (510) K (495) (75) (50) 360 25 $\mathbf{\Psi}$ Ľ (25) 15 7 R (50) 25 135 60 N (25) (60) (125) 50 $\mathbf{\Psi}$ Ľ 7 R ♠ 195 N (85) (15) (100) (15) 25 (15) R (35) 25 35 15 15 ← (20) $\mathbf{1}$ 10 K K K (15) 15 7 R ♠ 7 (40) 15 25 160 15 → (15) (25) 20 (65) (15) N

Figure

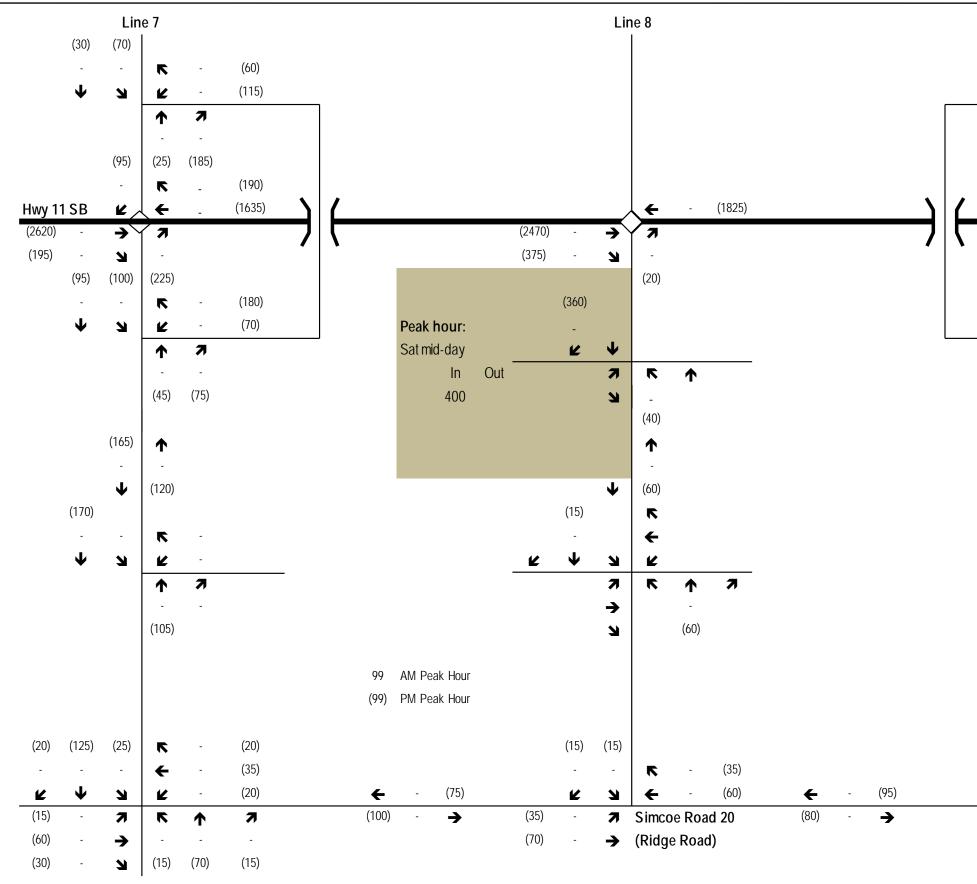


C.C. Tatham & Associates Ltd.

2015 Contemporary Music & Camping Festival - Peak Hour Volumes (Saturday mid-day)

	(15)	Lin (15)	ie 9		
	Ľ	- •			
(20)	-	7	7	↑	
(60)	-	R	-	-	
		(75)	(50)	(10)	
		-	R	-	(50)
		Ľ,	\	-	(1660)
2265)	-	→ `	7	Hwy	/ 11 NB
(95)	-	Ч	-		
	(30)	(65)	(40)		
	-	-			
	Ľ	$\mathbf{\Psi}$			
(20)	-	7	R	↑	
(45)	-	Ы	-	-	
			(50)	(20)	
		(110)			
		-			
	K	$\mathbf{\Psi}$			
		7	7	↑	
		Ы		-	
				(70)	
(10)	(95)	(10)	7	-	(10)
-	-	-	←	-	(50)
Ľ	$\mathbf{\Psi}$	Ы	Ľ	-	(15)
(10)	-	7	K	↑	7
		→	-	-	-
(35)	-	7			

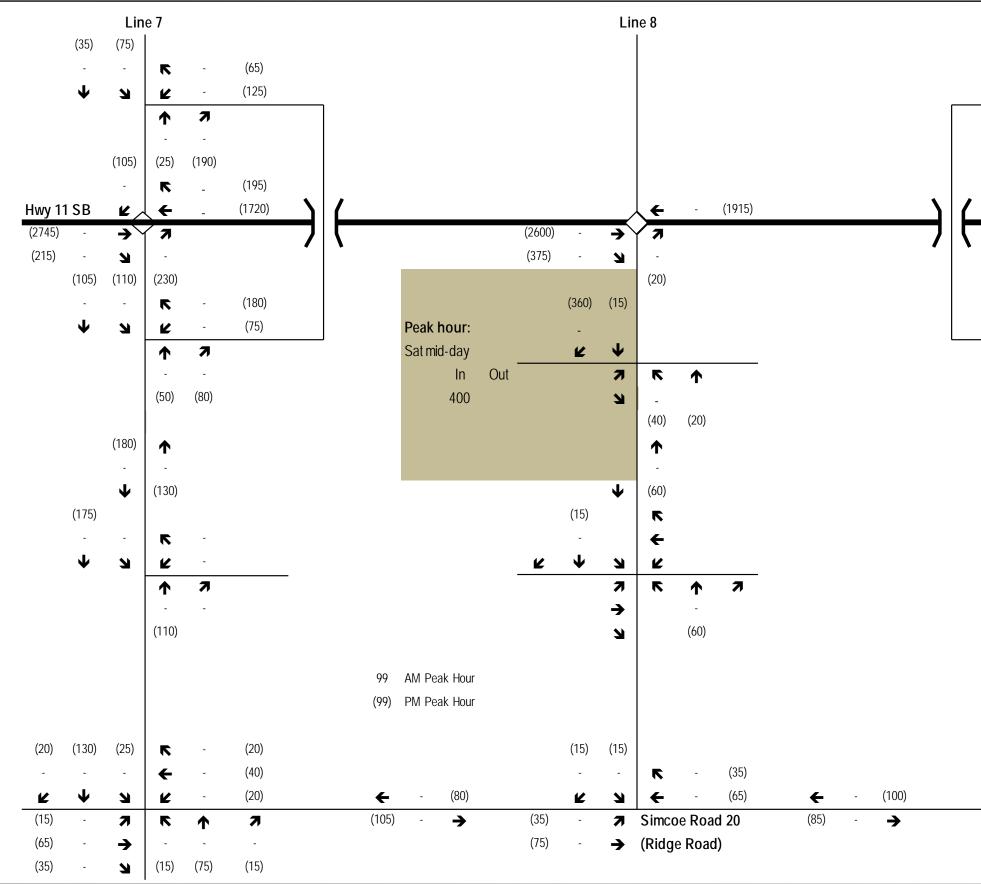
Figure



C.C. Tatham & Associates Ltd. Consulting Engineers

2020 Contemporary Music & Camping Festival - Peak Hour Volumes (Saturday mid-day)

	(20)	Lir (20)	ne 9 			
	-	-				
()	Ľ	V				
25) 70)	-	7	7	↑		
(70)	-) (90)	(60)	- (15)		
		-	(00) K	-	(60)	
		r,	(-	(1735)	
380)	-	→	7	Hwy	/ 11 NB	
10)	-	Ы	-	-		
	(35)	(75)	(50)			
	-	-				
05)	Ľ	•	_			
25) 50)	-	7	R	↑		
50)	-	Ы	(60)	- (25)		
			(00)	(20)		
		(125)				
	Ľ	Ŀ				
	Ľ	▼ 7	7	•		
		N		-		
		_		(85)		
'1E\	(100)	(15)			(15)	
15)	(100)	(15)		-	(15) (55)	
	J	Ľ	ר ר	-	(20)	
	-			•		
Ľ 15)	-	7		T		
∠ 15) 40)	-	7 →	N	T	7	



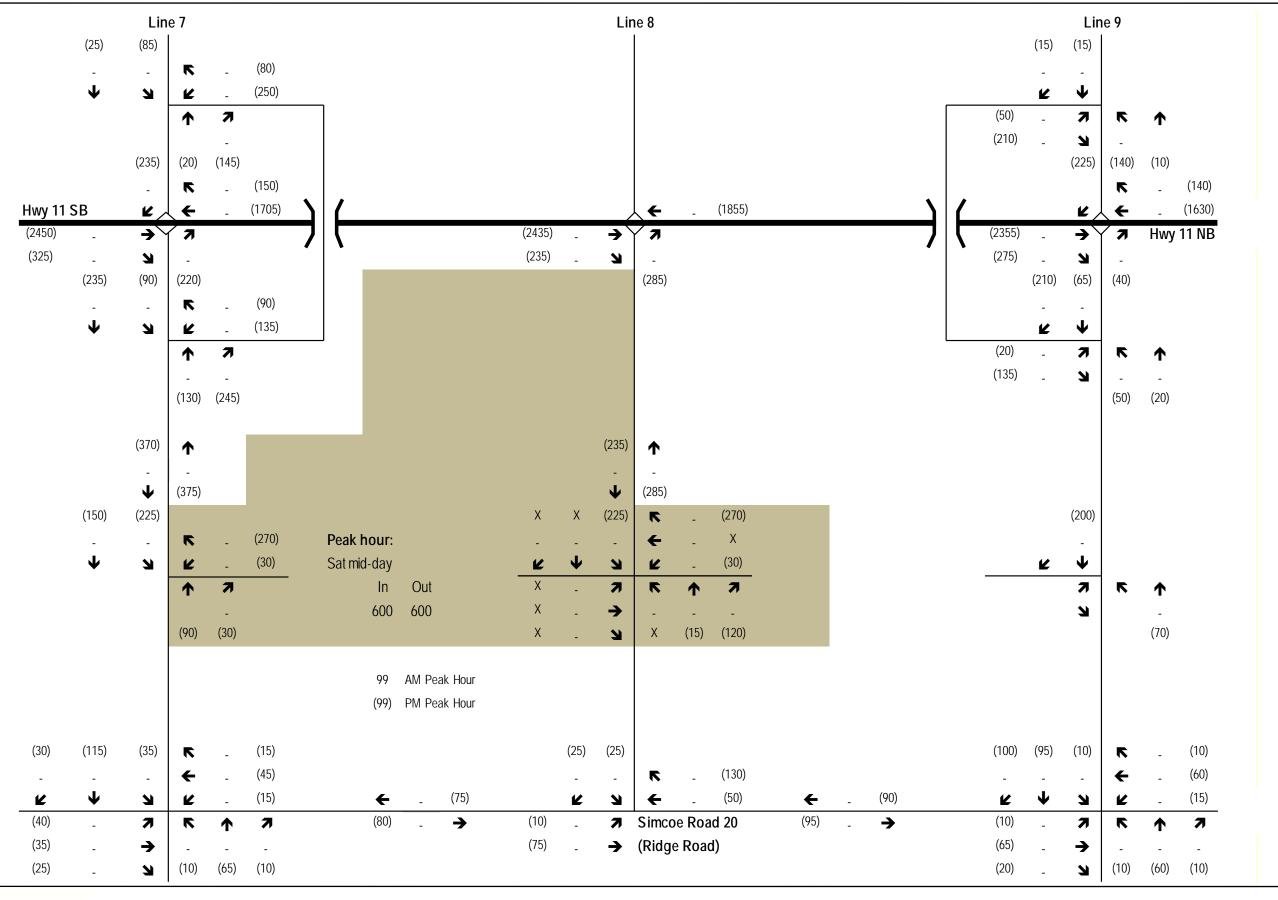
C.C. Tatham & Associates Ltd. Consulting Engineers

2025 Contemporary Music & Camping Festival - Peak Hour Volumes (Saturday mid-day)

	(20)	Lir (20)	ne 9		
	-	-			
	K	$\mathbf{\Psi}$			
(25)	-	7	7	↑	
(75)	-	Ы	-	-	
		(95)	(65)	(15)	
		-	R	-	(65)
		Ľ	\ ←	-	(1820)
(2500)	-	→ `	7	Hwy	/ 11 NB
(120)	-	R	-		
	(40)	(80)	(50)		
	-	-			
()5)	Ľ	<u>↓</u>	_	•	
(25) (55)	-	7	R	Τ	
(55)	-	R	- (65)	- (25)	
			(05)	(23)	
		(135)			
		-			
	Ľ	$\mathbf{\Psi}$			
		7	R	↑	
		R		-	
				(90)	
(15)	(105)	(15)	R	-	(15)
-	-	-	+	-	(60)
Ľ	$\mathbf{\Psi}$	N	Ľ	-	(20)
(15)	-	7	7	↑	7
		→	-	-	-
(45)	-	-			

Figure

N

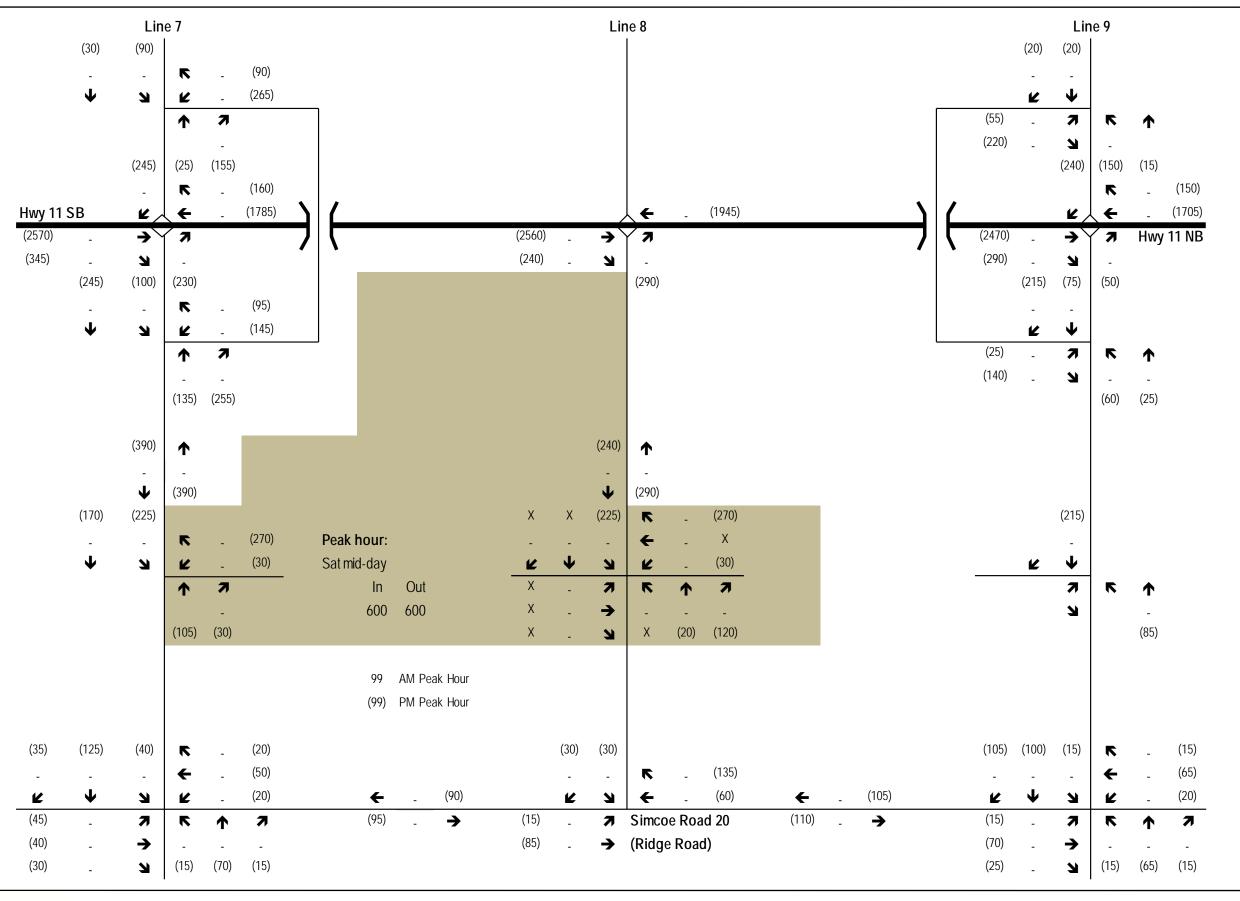


2015 Barrie Automotive Flea Market - Peak Hour Volumes (Saturday mid-day)



Figure

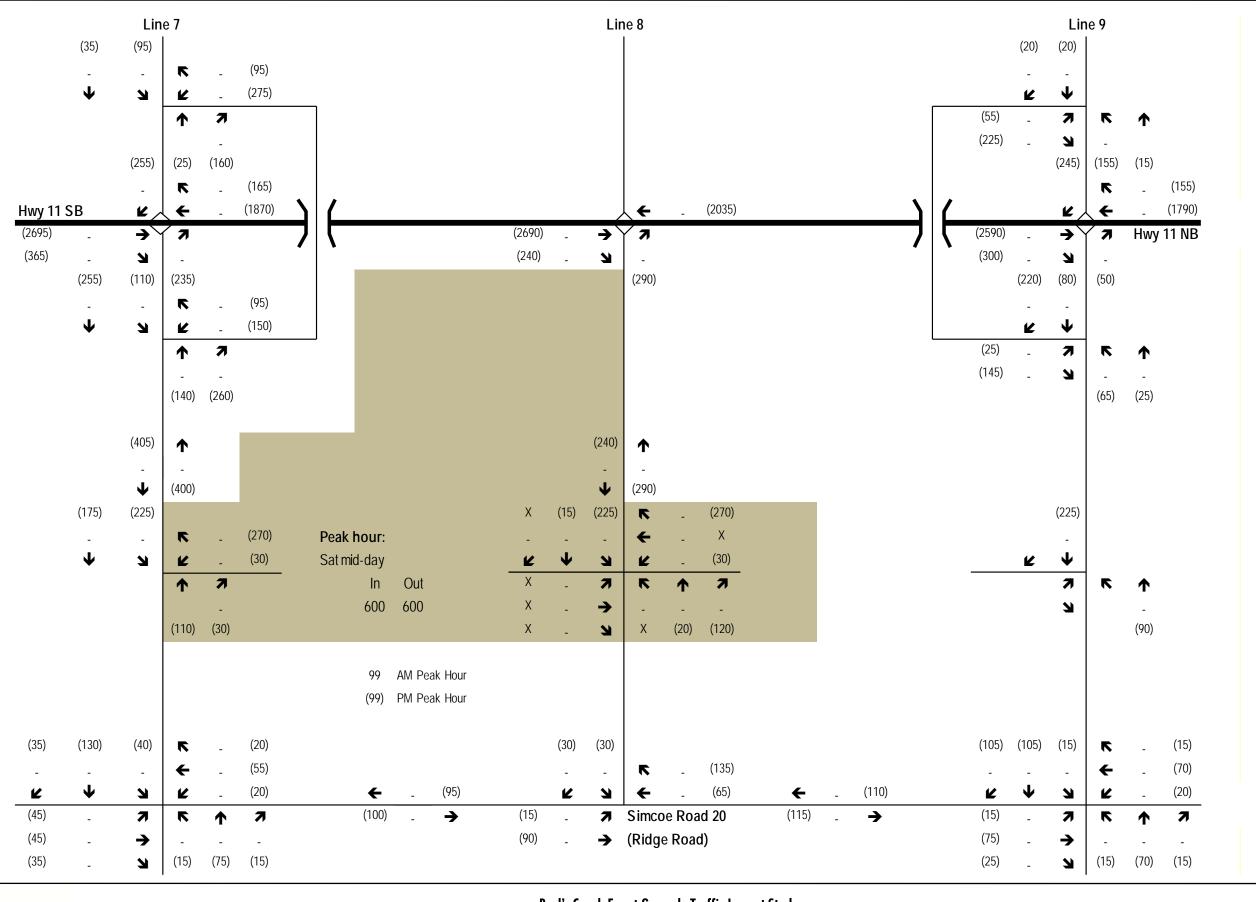
40



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2020 Barrie Automotive Flea Market - Peak Hour Volumes (Saturday mid-day)

Figure



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Burl's Creek Event Grounds Traffic Impact Study

2025 Barrie Automotive Flea Market - Peak Hour Volumes (Saturday mid-day)

Figure

42

APPENDIX A: TRAFFIC COUNTS Traffic

Software

Engineering

Weekly Volume Summary

Tue, Aug 12, 2014

LHRS/Offset	t: 16950 / 8	.23	Regi	on: Centra	1			
Pattern Type	e: Intermedi	ate Recreation	PC	S#: 4	Hwy. T	VIS#: 11116	6	
Count Direction	n: NB		Rep	ort Dates:	Jul 15, 2014	to Jul 21,	2014	
Hour	Tue	Wed	Thu	Fri	Sat	Sun	Mon	Tue
Interval	14/07/15	16	17	18	19	20	21	22
0:00- 1:00	168	200	218	276	412	227	161	
1:00-2:00	182	177	179	171	317	206	110	
2:00-3:00	122	149	115	141	189	110	76	
3:00- 4:00	76	92	90	76	113	62	58	
4:00- 5:00	130	110	91	148	147	59	105	
5:00- 6:00	561	319	311	293	209	95	241	
6:00- 7:00	645	679	627	669	494	197	496	
7:00- 8:00	1,148	1,158	1,180	1,266	1,052	414	1,177	
8:00- 9:00	1,290	1,365	1,322	1,540	2,090	753	1,364	
9:00-10:00	1,223	1,293	1,291	1,650	2,341	1,036	1,283	
10:00-11:00	1,208	1,334	1,497	1,896	2,285	1,394	1,352	
11:00-12:00	1,386	1,509	1,608	2,513	2,491	1,734	1,539	
AM Total	8,139	8,385	8,529	10,639	12,140	6,287	7,962	(
12:00-13:00	1,368	1,492	1,686	2,353	2,268	1,806	1,608	
13:00-14:00	1,366	1,476	1,823	2,541	2,447	1,695	1,577	
14:00-15:00	1,558	1,575	1,817	2,593	2,429	1,198	1,515	
15:00-16:00	1,644	1,743	2,028	2,645	2,305	1,572	1,582	
16:00-17:00	1,813	1,792	2,099	2,738	1,645	1,200	1,758	
17:00-18:00	1,838	1,936	2,180	2,789	1,331	1,166	1,723	
18:00-19:00	1,287	1,455	1,697	2,661	1,393	914	1,187	
19:00-20:00	979	1,002	1,628	2,487	1,075	845	985	
20:00-21:00	828	875	1,394	2,318	871	699	706	
21:00-22:00	686	729	1,159	2,129	711	554	606	
22:00-23:00	408	504	744	1,233	494	461	392	
23:00-24:00	328	332	413	728	368	237	312	
PM Total	14,103	14,911	18,668	27,215	17,337	12,347	13,951	(
24 Hr. Total	22,242	23,296	27,197	37,854	29,477	18,634	21,913	
Noon - Noon	22,4	488 23,44	40 29,3	307 30	9,355 23,0	524 20,3	309 13,95	1

Traffic

Software

Engineering

Weekly Volume Summary

LHRS/Offset	t: 16950 / 8.	23	Regio	on: Centra	1			
Pattern Type	e: Intermedia	ate Recreation	PCS	S#: 4	Hwy. T	VIS#: 11116	ō	
Count Direction	: SB		Rep	ort Dates:	Jul 15, 2014	to Jul 21,	2014	
Hour	Tue	Wed	Thu	Fri	Sat	Sun	Mon	Tu
Interval	14/07/15	16	17	18	19	20	21	22
0:00-1:00	105	280	104	245	169	432	174	
1:00-2:00	50	93	63	102	110	227	80	
2:00-3:00	66	75	71	84	98	129	86	
3:00- 4:00	63	87	69	83	72	113	77	
4:00- 5:00	190	175	177	156	94	81	245	
5:00- 6:00	423	487	452	413	126	97	553	
6:00- 7:00	752	725	769	695	256	183	967	
7:00- 8:00	1,106	1,073	1,027	967	398	298	1,229	
8:00-9:00	1,163	1,128	1,084	968	647	537	1,332	
9:00-10:00	1,108	1,036	1,049	1,040	944	950	1,337	
10:00-11:00	1,262	1,161	1,153	1,093	1,237	1,630	1,496	
11:00-12:00	1,273	1,211	1,128	1,244	1,565	2,191	1,492	
AM Total	7,561	7,531	7,146	7,090	5,716	6,868	9,068	
12:00-13:00	1,247	1,247	1,131	1,430	1,673	2,360	1,459	
13:00-14:00	1,224	1,236	1,197	1,434	1,633	2,357	1,516	
14:00-15:00	1,328	1,196	1,258	1,406	1,519	2,578	1,391	
15:00-16:00	1,313	1,284	1,308	1,505	1,494	2,322	1,440	
16:00-17:00	1,347	1,394	1,464	1,467	1,621	2,143	1,543	
17:00-18:00	1,292	1,353	1,388	1,349	1,388	2,364	1,432	
18:00-19:00	911	950	981	1,076	1,205	2,002	1,088	
19:00-20:00	634	754	772	935	1,057	1,639	846	
20:00-21:00	520	637	619	736	1,055	1,805	750	
21:00-22:00	500	490	522	642	1,038	1,541	602	
22:00-23:00	307	309	360	420	690	873	483	
23:00-24:00	312	187	498	269	548	312	248	
PM Total	10,935	11,037	11,498	12,669	14,921	22,296	12,798	
24 Hr. Total	18,496	18,568	18,644	19,759	20,637	29,164	21,866	

Traffic

Software

Engineering

Weekly Volume Summary

Tue, Aug 12, 2014

LHRS/Offs	et: 16950/8	.23	Regi	ion: Centra	1			
Pattern Typ	e: Intermed	iate Recreation	РС	CS#: 4	Hwy. T	VIS#: 1111	6	
Count Directio	n: NB/SB		Rej	port Dates:	Jul 15, 2014	to Jul 21,	2014	
Hour	Tue	Wed	Thu	Fri	Sat	Sun	Mon	Tu
Interval	14/07/15	16	17	18	19	20	21	2
0:00-1:00	273	480	322	521	581	659	335	
1:00-2:00	232	270	242	273	427	433	190	
2:00-3:00	188	224	186	225	287	239	162	
3:00- 4:00	139	179	159	159	185	175	135	
4:00- 5:00	320	285	268	304	241	140	350	
5:00- 6:00	984	806	763	706	335	192	794	
6:00- 7:00	1,397	1,404	1,396	1,364	750	380	1,463	
7:00- 8:00	2,254	2,231	2,207	2,233	1,450	712	2,406	
8:00-9:00	2,453	2,493	2,406	2,508	2,737	1,290	2,696	
9:00-10:00	2,331	2,329	2,340	2,690	3,285	1,986	2,620	
10:00-11:00	2,470	2,495	2,650	2,989	3,522	3,024	2,848	
11:00-12:00	2,659	2,720	2,736	3,757	4,056	3,925	3,031	
AM Total	15,700	15,916	15,675	17,729	17,856	13,155	17,030	
12:00-13:00	2,615	2,739	2,817	3,783	3,941	4,166	3,067	
13:00-14:00	2,590	2,712	3,020	3,975	4,080	4,052	3,093	
14:00-15:00	2,886	2,771	3,075	3,999	3,948	3,776	2,906	
15:00-16:00	2,957	3,027	3,336	4,150	3,799	3,894	3,022	
16:00-17:00	3,160	3,186	3,563	4,205	3,266	3,343	3,301	
17:00-18:00	3,130	3,289	3,568	4,138	2,719	3,530	3,155	
18:00-19:00	2,198	2,405	2,678	3,737	2,598	2,916	2,275	
19:00-20:00	1,613	1,756	2,400	3,422	2,132	2,484	1,831	
20:00-21:00	1,348	1,512	2,013	3,054	1,926	2,504	1,456	
21:00-22:00	1,186	1,219	1,681	2,771	1,749	2,095	1,208	
22:00-23:00	715	813	1,104	1,653	1,184	1,334	875	
23:00-24:00	640	519	911	997	916	549	560	
PM Total	25,038	25,948	30,166	39,884	32,258	34,643	26,749	
24 Hr. Total	40,738	41,864	45,841	57,613	50,114	47,798	43,779	
Noon - Noon	40,	954 41,62	23 47,	895 5	7,740 45,	413 51,6	573 26,7	49
		AWD			SADT			DH
	46,821	43,056	36,928	36,574	43,944	39,866	31,389	4,43

Page 3 of 3

Highway	Location Description	Dist	Year	Patt Type	AADT	SADT	SAWDT	WADT	AR
			1999		31,100	38,300			
			2000		32,000	38,700	-		
			2001	IR	32,900	40,100	-		
			2002		33,800	41,300			
			2003		34,700	42,300			
			2004		35,600	42,600			
			2005 2006		36,500 37,900	43,500 48,200			
			2000		37,900	49,200		-	
			2007		39,300	50,500			
			2000		40,200				
			2003		41,100		-		
11	SIMCOE RD 20-ORO MEDONTE LINE 11 IC 121	78	1988		22,400				
		7.0	1989		24,300				
			1990		26,250				
			1991	IR	26,500	33,300			
			1992		26,800	32,900			
			1993	IR	28,000	34,400			
			1994	IR	28,900	36,100		24,600	0.4
			1995	IR	29,400	36,500	31,500	24,700	0.5
			1996	IR	30,000	39,300	34,800	24,200	0.7
			1997	IR	30,500	40,000	35,400	25,000	0.5
			1998		31,000	40,000			
			1999		31,500				
			2000		33,500		-		
			2001	IR	34,300				
			2002		35,200				
			2003		36,100				
			2004		37,300				
			2005		37,600	47,300			
			2006 2007		37,800 38,600				
			2007		40,200				
			2000			47,300	-		
			2003			43,200		34,600	
11	MEMORIAL AVE IC		1988	_	14,300				
			1989		16,300		-		
			1990		18,350				
			1991		19,600				
			1992		20,050				
			1993		22,250				
			1994		23,100				
				LR		36,600	-	17,600	
			1996	LR			31,100		

County Road 20 - Spring 2014 Line 2 Oro-Medonte to Line 7 Oro-Medonte

County of Simcoe Transportation and Engineering Department Midhurst, Ontario (705)-726-9300

Site Code: 020 02

Line 2 Oro-Medonte Line 7 Oro-Medonte

Start	19-May	-14	Τι	ie	We	ed	Tł	าน	Fri		Sat		Sun		Week Ave	rage
Time	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	ŴВ
12:00 AM	*	*	1	6	1	1	2	1	*	*	*	*	*	*	1	3
01:00	*	*	3	0	4	1	2	1	*	*	*	*	*	*	3	1
02:00	*	*	2	0	4	1	1	1	*	*	*	*	*	*	2	1
03:00	*	*	1	0	0	0	0	0	*	*	*	*	*	*	0	0
04:00	*	*	1	1	0	3	0	2	*	*	*	*	*	*	0	2
05:00	*	*	0	5	2	4	2	6	*	*	*	*	*	*	1	5
06:00	*	*	6	17	6	26	3	24	*	*	*	*	*	*	5	22
07:00	*	*	17	73	15	73	16	73	*	*	*	*	*	*	16	73
08:00	*	*	24	71	35	71	34	66	*	*	*	*	*	*	31	69
09:00	*	*	28	54	32	74	36	69	*	*	*	*	*	*	32	66
10:00	*	*	33	49	28	32	35	53	*	*	*	*	*	*	32	45
11:00	*	*	43	45	44	40	44	48	*	*	*	*	*	*	44	44
12:00 PM	*	*	56	54	57	54	54	54	*	*	*	*	*	*	56	54
01:00	*	*	50	53	50	49	51	59	*	*	*	*	*	*	50	54
02:00	*	*	66	46	50	53	50	57	*	*	*	*	*	*	55	52
03:00	*	*	55	46	52	50	62	33	*	*	*	*	*	*	56	43
04:00	*	*	79	61	72	63	69	61	*	*	*	*	*	*	73	62
05:00	*	*	80	46	88	50	100	55	*	*	*	*	*	*	89	50
06:00	*	*	71	46	106	47	84	68	*	*	*	*	*	*	87	54
07:00	*	*	58	33	57	34	81	45	*	*	*	*	*	*	65	37
08:00	*	*	44	34	54	113	67	36	*	*	*	*	*	*	55	61
09:00	*	*	16	11	32	26	22	20	*	*	*	*	*	*	23	19
10:00	*	*	17	7	19	10	19	9	*	*	*	*	*	*	18	9
11:00	*	*	12	2	8	4	9	7	*	*	*	*	*	*	10	4
Lane	0	0	763	760	816	879	843	848	0	0	0	0	0	0	804	830
Day	0		152	3	169	5	169	1	0		0		0		1634	
AM Peak	-	-	11:00	07:00	11:00	09:00	11:00	07:00	-	-	-	-	-	-	11:00	07:00
Vol.	-	-	43	73	44	74	44	73	-	-	-	-	-	-	44	73
PM Peak	-	-	17:00	16:00	18:00	20:00	17:00	18:00	-	-	-	-	-	-	17:00	16:00
Vol.	-	-	80	61	106	113	100	68	-	-	-	-	-	-	89	62

Comb.	
Total	

ADT

0

ADT 1,636

AADT 1,636

1523

1695

1691

0

0

1634

County Road 20 - Summer 2014 Line 3 Oro-Medonte to Line 7 Oro-Medonte

County of Simcoe Transportation and Engineering Department Midhurst, Ontario (705)-726-9300

Site Code: 020 02

Line 3 Line 7

Start	11-Au	g-14	Tu	е	We	ed	T۲	าน	Fri		Sat		Sun	1	Week Av	/erage
Time	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	ŴВ
12:00 AM	4	5	3	5	6	4	3	1	*	*	*	*	*	*	4	
01:00	2	0	3	1	4	3	5	1	*	*	*	*	*	*	4	
02:00	2	2	2	2	2	2	2	2	*	*	*	*	*	*	2	
03:00	4	1	3	1	2	0	4	1	*	*	*	*	*	*	3	
04:00	1	3	0	1	0	2	0	0	*	*	*	*	*	*	0	
05:00	1	3	1	3	2	7	1	7	*	*	*	*	*	*	1	
06:00	5	12	3	13	5	20	4	15	*	*	*	*	*	*	4	1
07:00	14	31	7	47	8	39	14	43	*	*	*	*	*	*	11	4
08:00	18	58	27	69	24	70	20	71	*	*	*	*	*	*	22	6
09:00	25	50	24	38	33	55	34	38	*	*	*	*	*	*	29	4
10:00	33	51	29	42	36	42	35	41	*	*	*	*	*	*	33	4
11:00	46	55	44	46	39	47	30	46	*	*	*	*	*	*	40	4
12:00 PM	48	65	43	59	41	52	48	49	*	*	*	*	*	*	45	5
01:00	54	57	58	62	45	48	45	52	*	*	*	*	*	*	50	5
02:00	85	53	36	48	50	49	52	34	*	*	*	*	*	*	56	4
03:00	43	55	54	57	51	36	52	43	*	*	*	*	*	*	50	4
04:00	68	55	75	46	66	38	68	50	*	*	*	*	*	*	69	4
05:00	93	59	124	53	78	55	96	45	*	*	*	*	*	*	98	5
06:00	54	43	73	43	60	40	80	50	*	*	*	*	*	*	67	4
07:00	50	37	46	30	35	27	47	23	*	*	*	*	*	*	44	2
08:00	37	31	28	26	40	10	27	16	*	*	*	*	*	*	33	2
09:00	20	19	32	21	17	15	32	13	*	*	*	*	*	*	25	1
10:00	16	11	21	24	18	11	15	10	*	*	*	*	*	*	18	1
11:00	12	6	5	6	12	6	10	9	*	*	*	*	*	*	10	
Lane	735	762	741	743	674	678	724	660	0	0	0	0	0	0	718	71
Day	149	7	148	4	135	2	138	4	0		0		0		1429	
AM Peak	11:00	08:00	11:00	08:00	11:00	08:00	10:00	08:00	-	-	-	-	-	-	11:00	08:0
Vol.	46	58	44	69	39	70	35	71	-	-	-	-	-	-	40	6
PM Peak	17:00	12:00	17:00	13:00	17:00	17:00	17:00	13:00	-	-	-	-	-	-	17:00	12:0
Vol.	93	65	124	62	78	55	96	52	-	-	-	-	-	-	98	5

Comb.	1407	1484	1252	1384	0	0	0	1/20
Total	1497	1404	1352	1304	0	0	0	1429

ADT ADT 1,429 AADT 1,429 Page 1

County Road 20 - Fall 2014 Line 3 - Oro Medonte to Line 7 - Oro Medonte

County of Simcoe Transporation and Engineering Department Midhurst, Ontario (705) 726-9300

Site Code: 020 02

Date Start: 14-Oct-14 Date End: 16-Oct-14

Start	13-Oct	-14	Τι	le	We	ed	Tł	าน	Fri		Sat		Sun		Week Av	erage
Time	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	ŴВ
12:00 AM	*	*	3	1	3	1	6	1	*	*	*	*	*	*	4	
01:00	*	*	3	1	1	0	1	1	*	*	*	*	*	*	2	
02:00	*	*	6	2	3	1	2	1	*	*	*	*	*	*	4	
03:00	*	*	0	1	0	1	0	0	*	*	*	*	*	*	0	
04:00	*	*	1	4	2	2	1	2	*	*	*	*	*	*	1	3
05:00	*	*	2	7	2	4	0	5	*	*	*	*	*	*	1	Ę
06:00	*	*	4	30	3	32	3	29	*	*	*	*	*	*	3	30
07:00	*	*	15	84	9	85	13	85	*	*	*	*	*	*	12	85
08:00	*	*	27	61	34	72	27	61	*	*	*	*	*	*	29	65
09:00	*	*	38	45	21	51	30	53	*	*	*	*	*	*	30	50
10:00	*	*	28	42	27	42	31	41	*	*	*	*	*	*	29	42
11:00	*	*	61	44	40	38	36	44	*	*	*	*	*	*	46	42
12:00 PM	*	*	72	56	53	48	46	34	*	*	*	*	*	*	57	46
01:00	*	*	45	62	52	40	36	38	*	*	*	*	*	*	44	47
02:00	*	*	71	66	48	40	42	35	*	*	*	*	*	*	54	47
03:00	*	*	67	58	66	34	73	51	*	*	*	*	*	*	69	48
04:00	*	*	95	55	80	51	98	48	*	*	*	*	*	*	91	51
05:00	*	*	92	51	86	35	77	49	*	*	*	*	*	*	85	45
06:00	*	*	45	27	54	40	62	52	*	*	*	*	*	*	54	4(
07:00	*	*	26	18	40	16	46	29	*	*	*	*	*	*	37	2′
08:00	*	*	25	12	30	28	19	13	*	*	*	*	*	*	25	18
09:00	*	*	20	10	11	18	23	17	*	*	*	*	*	*	18	15
10:00	*	*	7	8	10	8	12	5	*	*	*	*	*	*	10	7
11:00	*	*	4	3	5	5	5	2	*	*	*	*	*	*	5	3
Lane	0	0	757	748	680	692	689	696	0	0	0	0	0	0	710	714
Day	0		150	5	137	2	138	5	0		0		0		1424	
AM Peak	-	-	11:00	07:00	11:00	07:00	11:00	07:00	-	-	-	-	-	-	11:00	07:00
Vol.	-	-	61	84	40	85	36	85	-	-	-	-	-	-	46	85
PM Peak	-	-	16:00	14:00	17:00	16:00	16:00	18:00	-	-	-	-	-	-	16:00	16:00
Vol.	-	-	95	66	86	51	98	52	-	-	-	-	-	-	91	5′

Comb.	0	1505	1372	1385	0	0	0	1424
Total	0	1505	1372	1969	0	0	0	1424

ADT AADT 1,421 ADT 1,421

Page 1

County Road 20 - Spring 2014 Line 7 Oro-Medonte to Line 11 Oro-Medonte

County of Simcoe Transportation and Engineering Department Midhurst, Ontario (705)-726-9300

Site Code: 020 03

Line 7 Oro-Medonte Line 11 Oro-Medonte

Start	19-May	-14	Tu	е	We	ed	Tł	าน	Fri		Sat		Sun		Week Av	rerage
Time	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	ŴВ
12:00 AM	*	*	0	2	1	0	0	0	*	*	*	*	*	*	0	1
01:00	*	*	2	0	0	1	0	2	*	*	*	*	*	*	1	1
02:00	*	*	1	1	2	2	1	1	*	*	*	*	*	*	1	1
03:00	*	*	0	6	0	10	1	8	*	*	*	*	*	*	0	8
04:00	*	*	3	17	3	13	4	12	*	*	*	*	*	*	3	14
05:00	*	*	4	20	5	25	5	21	*	*	*	*	*	*	5	22
06:00	*	*	21	52	14	56	15	50	*	*	*	*	*	*	17	53
07:00	*	*	22	36	33	45	29	56	*	*	*	*	*	*	28	46
08:00	*	*	19	36	20	35	19	46	*	*	*	*	*	*	19	39
09:00	*	*	25	32	27	29	20	34	*	*	*	*	*	*	24	32
10:00	*	*	25	31	20	30	27	34	*	*	*	*	*	*	24	32
11:00	*	*	31	29	28	26	34	31	*	*	*	*	*	*	31	29
12:00 PM	*	*	31	43	32	44	31	39	*	*	*	*	*	*	31	42
01:00	*	*	30	35	42	41	34	43	*	*	*	*	*	*	35	40
02:00	*	*	27	38	34	55	36	31	*	*	*	*	*	*	32	41
03:00	*	*	43	45	30	54	34	53	*	*	*	*	*	*	36	51
04:00	*	*	31	56	42	56	48	61	*	*	*	*	*	*	40	58
05:00	*	*	44	33	36	74	28	53	*	*	*	*	*	*	36	53
06:00	*	*	27	29	25	27	45	29	*	*	*	*	*	*	32	28
07:00	*	*	13	21	43	26	32	30	*	*	*	*	*	*	29	26
08:00	*	*	10	17	14	11	5	18	*	*	*	*	*	*	10	15
09:00	*	*	3	10	6	8	7	10	*	*	*	*	*	*	5	g
10:00	*	*	4	2	4	2	4	3	*	*	*	*	*	*	4	2
11:00	*	*	0	3	1	3	0	0	*	*	*	*	*	*	0	2
Lane	0	0	416	594	462	673	459	665	0	0	0	0	0	0	443	645
Day	0		101	00	113	5	112	4	0		0		0		1088	
AM Peak	-	-	11:00	06:00	07:00	06:00	11:00	07:00	-	-	-	-	-	-	11:00	06:00
Vol.	-	-	31	52	33	56	34	56	-	-	-	-	-	-	31	53
PM Peak	-	-	17:00	16:00	19:00	17:00	16:00	16:00	-	-	-	-	-	-	16:00	16:00
Vol.	-	-	44	56	43	74	48	61	-	-	-	-	-	-	40	58

Comb.	٥	1010	1135	1124	0	0	0	1088
Total	0	1010	1100	1124	0	0	0	1000

ADT ADT 1,090 AADT 1,090

County Road 20 - Summer 2014 Line 7 Oro-Medonte to Line 11 Oro-Medonte

County of Simcoe Transportation and Engineering Department Midhurst, Ontario (705)-726-9300

Site Code: 020 03

1046

Line 7 Line 11

Start	11-Au	g-14	Tu	е	We	ed	Th	u	Fri		Sat		Sun		Week Av	rage
Time	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	ŴВ
12:00 AM	4	2	0	3	2	1	8	3	*	*	*	*	*	*	4	2
01:00	0	0	1	1	1	0	1	1	*	*	*	*	*	*	1	(
02:00	0	0	1	0	3	2	1	1	*	*	*	*	*	*	1	1
03:00	3	2	1	1	0	1	2	1	*	*	*	*	*	*	2	1
04:00	2	0	2	0	3	1	2	2	*	*	*	*	*	*	2	1
05:00	12	0	10	0	12	1	11	0	*	*	*	*	*	*	11	C
06:00	21	3	21	1	26	1	20	2	*	*	*	*	*	*	22	2
07:00	31	11	45	12	33	9	39	9	*	*	*	*	*	*	37	10
08:00	50	16	51	24	53	30	52	22	*	*	*	*	*	*	52	23
09:00	47	23	38	26	47	25	50	39	*	*	*	*	*	*	46	28
10:00	25	27	32	15	39	28	50	26	*	*	*	*	*	*	36	24
11:00	31	34	29	26	21	21	31	26	*	*	*	*	*	*	28	27
12:00 PM	57	33	31	34	32	30	40	32	*	*	*	*	*	*	40	32
01:00	38	27	55	31	48	27	44	34	*	*	*	*	*	*	46	30
02:00	42	39	41	30	36	28	26	33	*	*	*	*	*	*	36	32
03:00	43	38	34	20	29	33	38	38	*	*	*	*	*	*	36	32
04:00	52	31	46	49	41	30	32	36	*	*	*	*	*	*	43	36
05:00	53	51	44	43	64	45	41	38	*	*	*	*	*	*	50	44
06:00	44	45	35	41	37	24	39	54	*	*	*	*	*	*	39	41
07:00	43	26	23	23	30	21	31	29	*	*	*	*	*	*	32	25
08:00	39	26	18	13	18	14	16	18	*	*	*	*	*	*	23	18
09:00	22	12	14	5	12	10	13	9	*	*	*	*	*	*	15	ç
10:00	9	7	12	11	11	7	10	5	*	*	*	*	*	*	10	8
11:00	3	1	1	6	2	7	4	11	*	*	*	*	*	*	2	6
Lane	671	454	585	415	600	396	601	469	0	0	0	0	0	0	614	432
Day	112		100		996		107		0		0		0		1046	
AM Peak	08:00	11:00	08:00	09:00	08:00	08:00	08:00	09:00	-	-	-	-	-	-	08:00	09:00
Vol.	50	34	51	26	53	30	52	39	-	-	-	-	-	-	52	28
PM Peak	12:00	17:00	13:00	16:00	17:00	17:00	13:00	18:00	-	-	-	-	-	-	17:00	17:00
Vol.	57	51	55	49	64	45	44	54	-	-	-	-	-	-	50	44

Comb.	
Total	

ADT

1125

ADT 1,048

AADT 1,048

1000

996

0

0

0

County Road 20 - Fall 2014 Line 7 - Oro Medonte to Line 11 - Oro Medonte

County of Simcoe Transporation and Engineering Department Midhurst, Ontario (705) 726-9300

Site Code: 020 03

Date Start: 14-Oct-14 Date End: 16-Oct-14

Start	13-Oct	-14	Tu	ie	We	ed	Tł	าน	Fri		Sat		Sun		Week Av	erage
Time	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	ŴВ
12:00 AM	*	*	1	3	1	0	0	0	*	*	*	*	*	*	1	
01:00	*	*	1	5	1	4	0	2	*	*	*	*	*	*	1	4
02:00	*	*	1	0	0	1	0	0	*	*	*	*	*	*	0	
03:00	*	*	1	2	1	2	3	3	*	*	*	*	*	*	2	:
04:00	*	*	0	6	0	14	0	10	*	*	*	*	*	*	0	1
05:00	*	*	0	23	1	17	2	19	*	*	*	*	*	*	1	2
06:00	*	*	2	34	2	40	2	40	*	*	*	*	*	*	2	3
07:00	*	*	19	44	13	46	19	57	*	*	*	*	*	*	17	4
08:00	*	*	15	40	19	46	22	57	*	*	*	*	*	*	19	4
09:00	*	*	25	31	19	28	16	34	*	*	*	*	*	*	20	3
10:00	*	*	31	29	24	20	29	34	*	*	*	*	*	*	28	2
11:00	*	*	40	37	24	42	27	48	*	*	*	*	*	*	30	4
12:00 PM	*	*	31	33	26	35	27	21	*	*	*	*	*	*	28	3
01:00	*	*	28	43	27	32	29	31	*	*	*	*	*	*	28	3
02:00	*	*	28	54	28	26	21	41	*	*	*	*	*	*	26	4
03:00	*	*	30	34	28	38	47	36	*	*	*	*	*	*	35	3
04:00	*	*	35	37	42	31	40	49	*	*	*	*	*	*	39	3
05:00	*	*	45	43	33	35	41	46	*	*	*	*	*	*	40	4
06:00	*	*	29	34	33	42	30	35	*	*	*	*	*	*	31	3
07:00	*	*	14	20	16	21	15	28	*	*	*	*	*	*	15	2
08:00	*	*	9	12	6	20	17	10	*	*	*	*	*	*	11	1
09:00	*	*	9	13	11	5	4	10	*	*	*	*	*	*	8	
10:00	*	*	2	6	2	6	6	12	*	*	*	*	*	*	3	
11:00	*	*	4	1	5	4	2	3	*	*	*	*	*	*	4	
Lane	0	0	400	584	362	555	399	626	0	0	0	0	0	0	389	58
Day	0		984		917		102		0		0		0		977	
AM Peak	-	-	11:00	07:00	10:00	07:00	10:00	07:00	-	-	-	-	-	-	11:00	07:0
Vol.	-	-	40	44	24	46	29	57	-	-	-	-	-	-	30	4
PM Peak	-	-	17:00	14:00	16:00	18:00	15:00	16:00	-	-	-	-	-	-	17:00	17:0
Vol.	-	-	45	54	42	42	47	49	-	-	-	-	-	-	40	4

Comb.	0	984	017	1025	0	0	0	077
Total	0	504	317	1025	0	0	0	511

ADT 975 ADT

AADT 975

County Road 20 - Spring 2014 Line 11 Oro-Medonte to Line 11 / Highway 11

County of Simcoe Transportation and Engineering Department Midhurst, Ontario (705)-726-9300

Site Code: 020 04

Line 11 Oro-Medonte Line 11 / Highway 11

Start	19-May-	14	Tue	9	We	ed	Tł	าน	Fri		Sat		Sun		Week Ave	erage
Time	SB	NB	SB	NB	SB	NB	SB	NB	SB	NB	SB	NB	SB	NB	SB	ŇВ
12:00 AM	*	*	*	*	1	10	3	9	*	*	*	*	*	*	2	1(
01:00	*	*	*	*	1	3	0	1	*	*	*	*	*	*	0	2
02:00	*	*	*	*	1	5	0	3	*	*	*	*	*	*	0	4
03:00	*	*	*	*	2	3	2	2	*	*	*	*	*	*	2	:
04:00	*	*	*	*	7	1	6	2	*	*	*	*	*	*	6	1
05:00	*	*	*	*	19	1	19	0	*	*	*	*	*	*	19	
06:00	*	*	*	*	41	3	46	5	*	*	*	*	*	*	44	4
07:00	*	*	*	*	63	26	64	24	*	*	*	*	*	*	64	2
08:00	*	*	*	*	92	49	89	43	*	*	*	*	*	*	90	4
09:00	*	*	*	*	94	37	78	40	*	*	*	*	*	*	86	3
10:00	*	*	*	*	43	51	52	47	*	*	*	*	*	*	48	4
11:00	*	*	*	*	46	54	37	47	*	*	*	*	*	*	42	5
12:00 PM	*	*	*	*	52	54	50	62	*	*	*	*	*	*	51	5
01:00	*	*	*	*	56	88	51	55	*	*	*	*	*	*	54	7
02:00	*	*	*	*	46	72	48	55	*	*	*	*	*	*	47	6
03:00	*	*	*	*	40	79	46	77	*	*	*	*	*	*	43	78
04:00	*	*	*	*	54	119	58	115	*	*	*	*	*	*	56	11
05:00	*	*	*	*	64	164	56	156	*	*	*	*	*	*	60	16
06:00	*	*	*	*	54	108	49	117	*	*	*	*	*	*	52	11
07:00	*	*	*	*	50	65	39	88	*	*	*	*	*	*	44	7
08:00	*	*	*	*	28	66	27	57	*	*	*	*	*	*	28	6
09:00	*	*	*	*	18	39	13	50	*	*	*	*	*	*	16	4
10:00	*	*	*	*	11	41	22	42	*	*	*	*	*	*	16	4
11:00	*	*	*	*	1	17	3	12	*	*	*	*	*	*	2	1
Lane	0	0	0	0	884	1155	858	1109	0	0	0	0	0	0	872	113
Day	0		0		203		196		0		0		0		2003	
AM Peak	-	-	-	-	09:00	11:00	08:00	10:00	-	-	-	-	-	-	08:00	11:0
Vol.	-	-	-	-	94	54	89	47	-	-	-	-	-	-	90	5
PM Peak	-	-	-	-	17:00	17:00	16:00	17:00	-	-	-	-	-	-	17:00	17:0
Vol.	-	-	-	-	64	164	58	156	-	-	-	-	-	-	60	16

Comb. Total	0	0	2039	1967	0	0	0	2003
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ADT

ADT 1,965

AADT 1,965

County Road 20 - Summer 2014 Line 11 Oro-Medonte to Line 11/ Highway 11

County of Simcoe Transportation and Engineering Department Midhurst, Ontario (705)-726-9300

Site Code: 020 04

Line 11 Line 11/ Highway 11

Start	11-Aug	j-14	Tu	le	We	ed	Tł	าน	Fri		Sat		Sur	1	Week Ave	erage
Time	SB	NB	SB	NB	SB	NB	SB	NB	SB	NB	SB	NB	SB	NB	SB	ŇB
12:00 AM	14	3	4	1	4	1	10	9	*	*	*	*	*	*	8	4
01:00	4	4	6	0	2	0	4	1	*	*	*	*	*	*	4	1
02:00	2	0	2	0	4	1	1	1	*	*	*	*	*	*	2	0
03:00	2	2	2	1	1	1	4	2	*	*	*	*	*	*	2	2
04:00	1	11	3	6	2	6	1	11	*	*	*	*	*	*	2	8
05:00	2	16	1	22	0	24	2	20	*	*	*	*	*	*	1	20
06:00	4	18	2	22	5	17	2	25	*	*	*	*	*	*	3	20
07:00	11	48	18	36	13	37	16	41	*	*	*	*	*	*	14	40
08:00	42	94	24	87	41	95	26	90	*	*	*	*	*	*	33	92
09:00	39	55	38	60	34	69	35	70	*	*	*	*	*	*	36	64
10:00	45	42	35	59	33	48	48	54	*	*	*	*	*	*	40	51
11:00	43	61	61	50	34	54	42	45	*	*	*	*	*	*	45	52
12:00 PM	60	56	43	50	51	40	61	46	*	*	*	*	*	*	54	48
01:00	71	59	64	52	64	34	58	48	*	*	*	*	*	*	64	48
02:00	71	40	56	37	62	42	46	40	*	*	*	*	*	*	59	40
03:00	74	50	67	39	81	50	59	41	*	*	*	*	*	*	70	45
04:00	103	51	95	50	95	49	102	58	*	*	*	*	*	*	99	52
05:00	130	68	129	73	144	57	123	60	*	*	*	*	*	*	132	64
06:00	93	56	98	44	101	37	122	50	*	*	*	*	*	*	104	47
07:00	69	40	62	24	50	28	62	40	*	*	*	*	*	*	61	33
08:00	58	28	47	28	47	28	53	23	*	*	*	*	*	*	51	27
09:00	44	14	35	14	44	11	46	20	*	*	*	*	*	*	42	15
10:00	34	10	30	3	28	9	32	6	*	*	*	*	*	*	31	7
11:00	20	9	12	1	18	10	20	5	*	*	*	*	*	*	18	6
Lane	1036	835	934	759	958	748	975	806	0	0	0	0	0	0	975	786
Day	187	1	169	3	170	6	178	1	0		0		0		1761	
AM Peak	10:00	08:00	11:00	08:00	08:00	08:00	10:00	08:00	-	-	-	-	-	-	11:00	08:00
Vol.	45	94	61	87	41	95	48	90	-	-	-	-	-	-	45	92
PM Peak	17:00	17:00	17:00	17:00	17:00	17:00	17:00	17:00	-	-	-	-	-	-	17:00	17:00
Vol.	130	68	129	73	144	57	123	60	-	-	-	-	-	-	132	64

Comb.	
Total	

ADT

1871

1706

1781

0

ADT 1,763

AADT 1,763

1693

0

0

1761

Page 1

County Road 20 - Fall 2014 Line 11 - Oro Medonte to Highway 11

County of Simcoe Transporation and Engineering Department Midhurst, Ontario (705) 726-9300

Site Code: 020 04

Date Start: 14-Oct-14 Date End: 16-Oct-14

Start	13-Oct-1	4	Tu	Ie	We	ed	Th	าน	Fri		Sat		Sun		Week Ave	rage
Time	SB	NB	SB	NB	SB	NB	SB	NB	SB	NB	SB	NB	SB	NB	SB	ŇВ
12:00 AM	*	*	9	4	7	1	8	0	*	*	*	*	*	*	8	2
01:00	*	*	7	0	5	1	6	0	*	*	*	*	*	*	6	(
02:00	*	*	3	1	4	1	3	0	*	*	*	*	*	*	3	1
03:00	*	*	1	3	1	4	2	4	*	*	*	*	*	*	1	2
04:00	*	*	1	12	1	15	1	14	*	*	*	*	*	*	1	14
05:00	*	*	4	19	2	21	2	16	*	*	*	*	*	*	3	19
06:00	*	*	9	48	10	48	7	44	*	*	*	*	*	*	9	47
07:00	*	*	26	98	19	90	38	85	*	*	*	*	*	*	28	91
08:00	*	*	40	75	35	83	32	95	*	*	*	*	*	*	36	84
09:00	*	*	47	56	31	51	33	45	*	*	*	*	*	*	37	51
10:00	*	*	45	54	38	41	32	59	*	*	*	*	*	*	38	51
11:00	*	*	47	39	45	57	44	51	*	*	*	*	*	*	45	49
12:00 PM	*	*	63	42	67	49	49	37	*	*	*	*	*	*	60	43
01:00	*	*	60	47	58	40	57	41	*	*	*	*	*	*	58	43
02:00	*	*	74	53	71	44	74	40	*	*	*	*	*	*	73	46
03:00	*	*	95	54	97	51	66	46	*	*	*	*	*	*	86	50
04:00	*	*	120	52	100	54	124	54	*	*	*	*	*	*	115	53
05:00	*	*	128	44	136	39	128	59	*	*	*	*	*	*	131	47
06:00	*	*	89	50	164	49	83	38	*	*	*	*	*	*	112	46
07:00	*	*	63	30	53	74	84	22	*	*	*	*	*	*	67	42
08:00	*	*	41	23	39	44	47	21	*	*	*	*	*	*	42	29
09:00	*	*	45	13	40	10	37	27	*	*	*	*	*	*	41	17
10:00	*	*	20	11	18	6	22	9	*	*	*	*	*	*	20	ç
11:00	*	*	12	2	13	5	13	1	*	*	*	*	*	*	13	3
Lane	0	0	1049	830	1054	878	992	808	0	0	0	0	0	0	1033	841
Day	0		187	9	193	2	180	0	0		0		0		1874	
AM Peak	-	-	09:00	07:00	11:00	07:00	11:00	08:00	-	-	-	-	-	-	11:00	07:00
Vol.	-	-	47	98	45	90	44	95	-	-	-	-	-	-	45	91
PM Peak	-	-	17:00	15:00	18:00	19:00	17:00	17:00	-	-	-	-	-	-	17:00	16:00
Vol.	-	-	128	54	164	74	128	59	-	-	-	-	-	-	131	53

Comb.	
Total	

Total

ADT

0

ADT 1,870

AADT 1,870

1879

1932

1800

0

0

0

Page 1



	TRANSP	ORTATION AND			Transport	ation & Er	ngineering	7				
	ENGINE	ERING	A	Annual Av	erage Dai	ly Traffic :	Summary	(A.A.D.T.	.)		Updated Nov	2013
Road# - Section #	Distance	Link Description	<u>2004</u>	<u>2005</u>	<u>2006</u>	<u>2007</u>	<u>2008</u>	<u>2009</u>	<u>2010</u>	<u>2011</u>	<u>2012</u>	<u>2013</u>
		CR 27										
001-01	6.1		5,000			4,300			4,100			3,900
004.00		15th SR New Tecumseth	5 000			4.000			4 700			4.400
001-02	2.0	East Limits / Beeton	5,300			4,900			4,700			4,400
001-03	1.9		5,000			4,900			4,500			4,400
		CR 10	-,			.,			.,			.,
001-04	3.1		2,300			3,000			3,200			2,400
		Adj/Tos Townline										
001-04A	2.9	CD 50	N/A			N/A			N/A			2,800
001-05	7.2	CR 50	2,000			1,800			1,900			1,900
001-03	1.2	Simcoe Boundary	2,000			1,000			1,300			1,500
		CR 39										
003-01	3.1		4,600			4,400			4,700			4,100
		CR 4										
		Bradford Limits										
004-01	9.8	Bradiora Limits	13,400			12,700			12,500			14,800
004 01	0.0	CR 89 / CR 3	10,400			12,700			12,000			14,000
004-02	4.4		8,600			9,600			9,200			11,500
		Line 4 / Churchill										
004-03	5.5		8,000			10,100			9,000			10,800
004.04	0.0	CR 21	40.500			44.000			44.000			44.000
004-04	2.8	Victoria St. / Stroud	10,500			11,200			11,200			11,900
004-05	2.9	Victoria St. / Stroud	10,800			11,700			13,000			13,600
		Lockhart Dr. / Barrie Limit	10,000						.0,000			.0,000
		CR 15										
005-01	4.1	CD 12	3,600			3,800			4,100			4,000
005-02	5.6	CR 13	1,800			1,700			2,400			1,800
000.02	0.0	County Boundary	1,000			1,700			2,400			1,000
		CR 27 N										
006-01	5.5			4,700			4,400			4,600		
000.00	0.0	Conc 4 / Tiny		2.000			0.700			4.000		
006-02	8.2	CR 25 / Perkinsfield		3,900			3,700			4,200		
006-03	1.4			4,700			4,400			4,700		
		Conc 11/ Tiny		.,			.,			.,		
006-04	2.8			3,400			3,400			3,900		
		Conc 13 / Tiny										
006-05	4.1			3,000			3,100			3,200		
		CR 26										

Road# - Section #	<u>Distance</u>	Link Description	2004	<u>2005</u>	2006	<u>2007</u>	2008	<u>2009</u>		<u>2011</u>	<u>2012</u>	<u>2013</u>
		CR 13										
012-01	3.9	CR 13	1,400			1,400			1,300			1,400
		County Boundary				,			.,			
013-01	5.7	Hwy 89	2,800			2,800			2,600			2,500
		CR 5										
013-02	9.5		2,000			1,900			1,900			2,900
		CR 12										
		CR 50										
014-01	3.8		1,000			1,000			950			1,200
014.02	2.0	Adjala-New Tech Townline	1 700			2.000			1 700			1 700
014-02	2.9	CR 10	1,700			2,000			1,700			1,700
		Victoria St. / Alliston										
015-01A	1.1	Essa Rd	8,950			8,000			8,800			9,100
015-01	4.7		5,100			5,800			5,600			5,200
		CR 5										
015-02	3.5		4,800			4,900			5,000			4,800
015-03	1.2	CR 21	3,100			3,000			2,400			2,100
010 00	1.2	Base Borden South Limit	3,100			3,000			2,400			2,100
016-01	6.2	CR 23		5,100			4.500			E 100		
010-01	0.2	Hwy 400		5,100			4,500			5,100		
0.17.0.1		Coldwater / North Limits		4.000			0.400			4 500		
017-01	6.8	Quarry Road		1,600			2,100			1,500		
017-02	4.3			1,200			1,200			1,200		
		4th Conc. Silkline										
017-03	11.9	Dia Chuta	_	450			400			400		
		Big Chute										
		Hwy 12										
019-01	2.5			1,000			900			1,200		
019-02	2.0	Hwy 400		1,700			1,500			1,600		
013-02	2.0	8th Conc / Moonstone		1,700			1,500			1,000		
019-03	10.8			1,000			950			1,000		
		Hwy 93										
019-04	8.1	00.07		1,700			1,500			1,900		
		CR 27										
		Barrie Limits										
020-01	7.4			3,500			2,800			3,100		
		Line 3 Oro-Medonte										
020-02	6.0			1,800			1,800			1,500		
020-03	6.2	Line 7 Oro-Medonte		1,000			1,000			1,000		
020-03	0.2	Line 11 Oro-Medonte		1,000			1,000			1,000		
020-04	1.1			1,900			1,800			1,800		
		Line 11 / Hwy 11		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			,			,		



TRANSPORTATION AND ENGINEERING

County of Simcoe

Transportation & Engineering

Annual Average Daily Traffic Summary (A.A.D.T.)

Road# - Section #	<u>Distance</u>	Link Description	<u>2002</u>	<u>2003</u>	<u>2004</u>	<u>2005</u>	<u>2006</u>	<u>2007</u>	<u>2008</u>	<u>2009</u>
		CR 27						4.000		
001-01	6.1				5,000			4,300		
004.00		15th SR New Tecumseth			5 000			1.000		
001-02	2.0				5,300			4,900		
		East Limits / Beeton						4.000		
001-03	1.9				5,000			4,900		
		CR 10								
001-04	6.6				2,300			3,000		
		CR 50								
001-05	7.2				2,000			1,800		
		Simcoe Boundary								
		CR 39						4.400		
003-01	3.1				4,600			4,400		
		CR 4								
		Bradford Limits								
004-01	9.8				13,400			12,700		
		CR 89 / CR 3								
004-02	4.4				8,600			9,600		
		Line 4 / Churchill								
004-03	5.5				8,000			10,100		
		CR 21								
004-04	2.8				10,500			11,200		
		Victoria St. / Stroud								
004-05	2.9				10,800			11,700		
		Lockhart Dr. / Barrie Limit								
		CR 15								
005-01	4.1				3,600			3,800		
		CR 13								
005-02	5.6				1,800			1,700		
		County Boundary								
		CR 27 N								
006-01	5.5		4,700			4,700			4,400	
		Conc 4 / Tiny								
006-02	8.2		4,200			3,900			3,700	
		CR 25 / Perkinsfield								
006-03	1.4		4,700			4,700			4,400	
		Conc 11/ Tiny								
006-04	2.8					3,400			3,400	
		Conc 13 / Tiny								
006-05	4.1		3,200			3,000			3,100	
		CR 26								

Updated Nov 2	2011
<u>2010</u>	<u>2011</u>
4,100	
4,700	
4,500	
3,200	
1,900	
4,700	
12,500	
9,200	
9,000	
11,200	
13,000	
4,100	
2,400	
	4,600
	4,200
	4,700
	3,900
	3,200

Road# - Section #	Distance	Link Description	2002	2003	2004	2005	2006	2007	2008	2009	2010	<u>2011</u>
	Distance			2000	2004	2000	2000	2007	2000	2003	2010	2011
		Barrie Limits										
020-01	7.4		3,050			3,500			2,800			3,100
		Line 3 Oro-Medonte				·			·			
020-02	6.0		1,600			1,800			1,800			1,500
		Line 7 Oro-Medonte										
020-03	6.2		900			1,000			1,000			1,000
		Line 11 Oro-Medonte										
020-04	1.1		1,600			1,900			1,800			1,800
		Line 11 / Hwy 11										
004.04		CR 39			40.000			0.000			44.400	
021-01	3.0	CR 4			10,600			9,800			11,100	
021-02	3.1	CR 4	-		10,600			11,400			11,800	
021-02	5.1	CR 54 (10th SR)			10,000			11,400			11,000	
021-03	2.2				12,000			13,800			14,000	
02100	LiL	Hwy 400			12,000			10,000			14,000	
021-04	0.6				10,900			11,700			13,100	
		5th SR Innisfil									,	
021-05	3.0				6,900			9,000			9,000	
		CR 27										
021-06	6.4				4,800			5,100			5,700	
		CR 56										
021-07	4.2				4,400			5,700			6,100	
		CR10										
021-08	4.2				3,300			3,600			4,700	
		CR 15										
000.01		Hwy 12	4.400			4.500			4.000			4.000
022-01	9.6	Couloop / 7th Line	4,100			4,500			4,600			4,300
022-02	4.9	Coulson/ 7th Line	4,100			4,100			4,600			4,300
022-02	4.9	Horseshoe Valley Resort Ent.	4,100			4,100			4,000			4,300
022-03	5.2	Torseshoe valley Resort Ent.	5,700			6,100			6,200			5,500
022 00	0.2	CR 93	0,100			0,100			0,200			0,000
022-04	1.4		5,600			6,100			6,200			5,800
		Hwy 400				-,			.,			- ,
022-05	4.6		4,900			5,300			5,300			5,600
		CR 27				·						
022-06	2.6		4,000			4,100			4,400			4,400
		Wilson Drive - Anten Mills										
022-07	3.9		4,200			3,900			3,900			3,900
		CR 29										
022-08	0.5		5,500			5,200			5,200			6,200
		Hwy 26										



Morning Peak	Diagram	Specified Period From: 7:00:00 To: 10:00:00	One Hour Peak From: 7:30:00 To: 8:30:00
TFR File #:1Count date:2-Apr-15	2 Highway 11 overpass	Weather conditions: Person counted: Person prepared: Person checked:	
North Entering:66TruNorth Peds:0C	rys 10 0 0 1 cks 3 0 1 4 ars <u>19 8 25</u> 5 als 32 8 26	0 2 Heavys 10 Trucks 7 Cars <u>161</u> Totals 178 ighway 11 overpass	runs W/E East Leg Total: 205 East Entering: 162 East Peds: 0 Peds Cross: X Cars Trucks Heavys Totals
12 4 53 69			119 2 1 122 34 1 2 37 3 0 0 3 156 3 3
Heavys Trucks Cars Totals 8 5 41 54 54 0 1 16 17 16 0 0 1 1 1 8 6 58 58 58	Driveway		Line Cars Trucks Heavys Totals 41 2 0 43
West Peds:0TruWest Entering:72Heave	cks 0 Truc rys 0 Heav	ars 0 1 0 1 cks 0 0 0 0 rys <u>0 1 0</u> 1 als 0 2 0	Peds Cross: M South Peds: 0 South Entering: 2 South Leg Total: 14



Afternoon Pe	eak Dia	gram	From:	ed Period 16:00:00 19:00:00	One H From: To:	our Peak 16:00:00 17:00:00
Municipality: Oro-Med Site #: 1503300 Intersection: 7th Line TFR File #: 1 Count date: 2-Apr-15	0002 9 & Highway 1 5		Person Person	r conditions counted: prepared: checked: oad: 7th Lin	: e runs W/E	
North Entering: 56 North Peds: 0 Peds Cross: ► Heavys Trucks Cars Totals 1 1 38 40 Tth	Heavys 0 Trucks 0 Cars <u>11</u> Totals 11	0 2 2 1 42 4 1 44	N E		Cars Tru 41 2 22 1 1 1 64 4	Leg Total: 183 Entering: 69 Peds: 0 s Cross: X cks Heavys Totals 0 43 1 24 0 2 1
Heavys Trucks Cars Totals 1 3 62 66 0 0 63 63 0 0 0 0 1 3 125 125		Driveway	。 、 〜 (Cars Tru	cks Heavys Totals 0 114
	Cars 2 Trucks 1 Heavys 0 Totals 3	Tru Hea	Cars 5 4 icks 0 0 vys 0 0 tals 5 4	6 15 1 1 0 0 7	Sout Sout	s Cross: ► h Peds: 0 h Entering: 16 h Leg Total: 19



7:00:00 10:00:00 er conditions: n counted: n prepared: n checked:	One Hour Peak From: 7:30:00 To: 8:30:00
n counted: n prepared: n checked:	
Decade Utility of	
Road: Highway 1 Heavys 10 Trucks 3 Cars 51 Totals 64	East Leg Total: 159 East Entering: 62 East Peds: 0 Peds Cross: X
$ \begin{array}{c} 12 \\ 14 \\ 14 \\ 14 \\ 14 \\ 14 \\ 3 \\ 14 \\ 14 \\ 14 \\ 14 \\ 14 \\ 14 \\ 14 \\ 14$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
7th Lind Ca 92	ars Trucks Heavys Totals
32 3 47 2 0 2 0 0 0 34 3	Peds Cross:Image: Constraint of the second seco
	32 3 47 2 0 2 0 0 0



Afternoon Peak Diagram	Specified Period One Hour Peak From: 16:00:00 From: 16:00:00 To: 19:00:00 To: 17:00:00
Municipality:Oro-MedonteSite #:1503300001Intersection:Highway 11 & 7th LineTFR File #:1Count date:2-Apr-15	Weather conditions: Person counted: Person prepared: Person checked:
Heavys Trucks Cars Totals	Totals 58 Peds Cross: X ghway 11 overpass Cars Trucks Heavys Totals 27 0 0 27 4 1 0 5
	rs 7 12 0 19 Peds Cross: ₩ ks 0 1 0 1 South Peds: 0
	ls 7 13 0 South Leg Total: 64



Accu-Tr	affic Inc.
Morning Peak Diagram	Specified Period One Hour Peak From: 7:00:00 From: 7:45:00 To: 10:00:00 To: 8:45:00
Municipality:Oro-MedonteSite #:1503300003Intersection:7th Line & Ridge RoadTFR File #:1Count date:2-Apr-15	Weather conditions: Person counted: Person prepared: Person checked:
** Non-Signalized Intersection **	Major Road: 7th Line runs W/E
North Leg Total: 54Heavys0000North Entering:37Trucks000North Peds:0Cars1720037Peds Cross:Image: Second Secon	7 Cars <u>17</u> East Peds: 0 Totals 17 Peds Cross: Ⅹ
Heavys Trucks Cars Totals	idge Road Cars Trucks Heavys Totals 5 0 0 113 0 0 16 16 E
Heavys Trucks CarsTotals006111416 \Box 0347 \Box	7th Line
3 3 1 <th1< th=""> <th1< th=""> <th1< th=""> <th1< th=""></th1<></th1<></th1<></th1<>	
Peds Cross: X Cars 40 Cars West Peds: 0 Trucks 3 Truck West Entering: 29 Heavys 0 Heav	ars 6 6 10 22 Peds Cross: ▶ cks 0 0 2 2 South Peds: 0 ys 1 0 0 1 South Entering: 25 als 7 6 12 South Leg Total: 68
Comn	nents

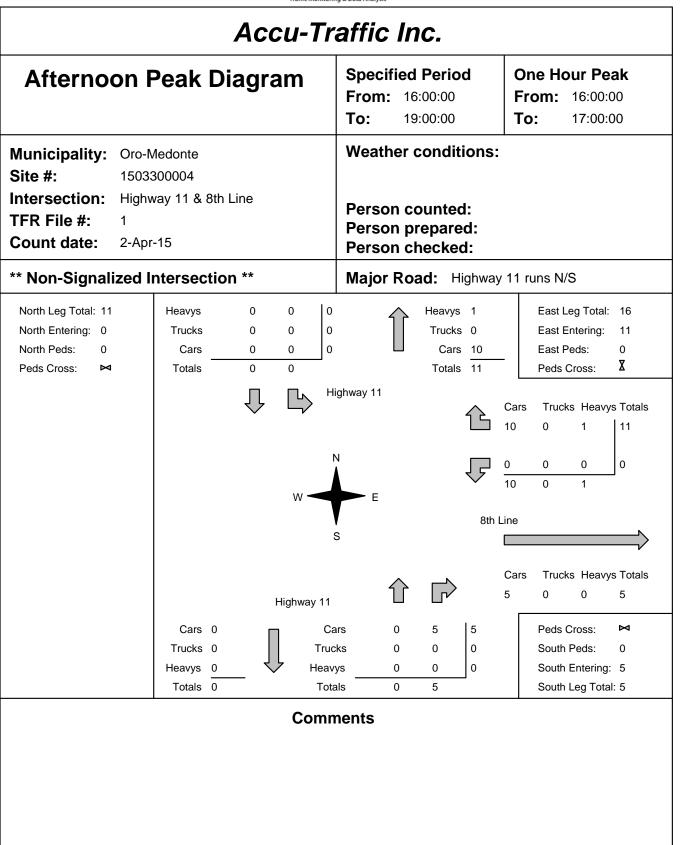


Afternoon F	Peak Diagra	am	Specif From: To:	ied Peric 16:00:00 19:00:00	od	One F From: To:	17:15:0 18:15:0	00
Site #:15033Intersection:7th LiTFR File #:1Count date:2-Apr			Persor Persor Persor	er condi n counte n prepare n checke	d: ed: d:			
** Non-Signalized In North Leg Total: 86 North Entering: 40 North Peds: 0 Peds Cross: ► Heavys Trucks Cars Tota 1 1 57 59 Heavys Trucks Cars Tota 0 0 15 15	Heavys 0 0 Trucks 0 0 Cars <u>8</u> 23 Totals 8 23 als	0 0 0 9 9 40 9 Ri	dge Road	Road: 7	$\begin{array}{c} & 0 \\ s & 0 \\ s & 46 \\ s & 46 \\ \hline \\ 46 \\ \hline \\ \hline \\ \hline \\ \hline \\ \hline \\ \\ \hline \\ \\ \hline \\ \\ \hline \\$	Eas Eas Ped Cars Tru 5 0 45 1 5 0 55 1	t Leg Total: t Entering: t Peds: s Cross: cks Heavy 0 1 0 1	57 0 X
0 0 15 15 1 0 85 86 0 0 8 8 1 0 108 Peds Cross: X West Peds: 0 West Entering: 109	Cars 36 Trucks 0 Heavys 0 Totals 36	Truck Heavy	rs 4 : ks 0 (ys <u>0 (</u>	$ \begin{array}{cccc} $		112 0 Ped Sou Sou	icks Heavy 1 s Cross: th Peds: th Entering: th Leg Tota	113 ► 0 48



Morning Peak Diagra	ım	-	00:00	One Hou From: 7	:15:00
Municipality:Oro-MedonteSite #:1503300004Intersection:Highway 11 & 8th LineTFR File #:1Count date:2-Apr-15		_	repared:	_	:15:00
** Non-Signalized Intersection ** North Leg Total: 4 Heavys 0 North Entering: 0 Trucks 0 North Peds: 0 Cars 0 Peds Cross: ► Totals 0		ghway 11	Heavys 2 Trucks 0 Cars 2 Totals 4	East Ent East Peo Peds Cro Cars Trucks 2 0 0 0	ls: 0
Cars 0 Trucks 0 Heavys 0 Totals 0	Highway 11 Car Truck Heavy Tota	ks 0 vs <u>0</u>		6 0 Peds Cro South Pe South Er	







Accu-T	raffic Inc.
Morning Peak Diagram	Specified Period One Hour Peak From: 7:00:00 From: 8:00:00 To: 10:00:00 To: 9:00:00
Municipality:Oro-MedonteSite #:1503300006Intersection:9th Line & Highway 11 overpassTFR File #:1Count date:2-Apr-15	Weather conditions: Person counted: Person prepared: Person checked:
** Non-Signalized Intersection **	Major Road: 9th Line runs W/E
	East Leg Total: 154 East Entering: 129 East Peds: 0 Peds Cross: X
Heavys Trucks Cars Totals 5 4 34 43 9th Line	Cars Trucks Heavys Totals $ \begin{array}{cccc} & & & \\ & $
Heavys Trucks Cars Totals $\begin{array}{c cccc} 0 & 1 & 12 \\ \hline 0 & 2 & 12 \\ \hline 0 & 3 & 24 \end{array}$ Highway 11 overpas	s S S S S S S S S S S S S S S S S S S S
West Peds: 0 Trucks 5 Trucks 5 West Entering: 27 Heavys 0 Heavys 1	Cars 6 9 15 Peds Cross: ▶ ucks 0 2 2 South Peds: 0 avys 0 1 1 South Entering: 18 otals 6 12 South Leg Total: 124
Con	ments



	Accu-Tr	affic Inc.		
Afternoon F	Peak Diagram	Specified Per From: 16:00:0 To: 19:00:0	00	One Hour PeakFrom:17:15:00To:18:15:00
Site #: 15033	ledonte 300006 ne & Highway 11 overpass 15	Weather con Person coun Person prepa Person chec	ted: ared:	
** Non-Signalized Ir	ntersection **	Major Road:	9th Line	runs W/E
				East Leg Total:133East Entering:53East Peds:0Peds Cross:X
	Is 9th Line W	N E	ج ا	Cars Trucks Heavys Totals 14 1 0 15 38 0 0 38 52 1 0
Heavys Trucks Cars Tota 0 0 49 49 0 0 21 21 0 0 70 1	Is	s G	\rightarrow	Cars Trucks Heavys Totals 78 2 0 80
Peds Cross: X West Peds: 0 West Entering: 70 West Leg Total: 99	Cars 59 C Trucks 0 Tru Heavys 0 Heav	ars 14 29 cks 0 2 rys <u>0 0</u> als 14 31	2 0	Peds Cross: M South Peds: 0 South Entering: 45 South Leg Total: 104
	Com	nents		



Morning Peak Diagram		
Norming Feak Diagram	Specified Period From: 7:00:00 To: 10:00:00	One Hour Peak From: 8:00:00 To: 9:00:00
Municipality:Oro-MedonteSite #:1503300005Intersection:9th Line & Highway 11 overpassTFR File #:1Count date:2-Apr-15	Weather conditions: Person counted: Person prepared: Person checked:	
** Non-Signalized Intersection **	Major Road: 9th Line	runs W/E
		East Leg Total: 129 East Entering: 15 East Peds: 0 Peds Cross: X
Heavys Trucks Cars Totals 0 4 8 12 9th Line		Cars Trucks Heavys Totals 0 0 0 0 0 13 1 1 15 13 1 1 1
Heavys Trucks Cars Totals	9th L	ine
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$		Cars Trucks Heavys Totals 110 3 1 114
West Peds: 0 Trucks 2 Truck West Entering: 27 Heavys 1 Heavys	ars 8 90 98 ks 4 1 5 ys <u>0 1</u> 1 als 12 92	Peds Cross: ► South Peds: 0 South Entering: 104 South Leg Total: 124
Comr	nents	



	Accu-T	raffic Ind				
Afternoon P	eak Diagram	Specified P From: 16:0 To: 19:0	0:00	One Fron To:	Hour Pe n: 17:15: 18:15:	00
Municipality:Oro-MeSite #:150330Intersection:9th LineTFR File #:1Count date:2-Apr-1	0005 e & Highway 11 overpass	Weather co Person cou Person pre Person che	inted: pared:			
** Non-Signalized Int	ersection **	Major Road	1: 9th Line	runs W	/E	
				E: E:	ast Leg Total: ast Entering: ast Peds: eds Cross:	97 42 0 X
Heavys Trucks Cars Totals 0 0 19 19	h Line	N E	Ŷŀ	6 (2 0	/s Totals 6 36
Heavys Trucks Cars Totals			9th	Line		•
0 0 9 9 0 0 8 8 0 0 17	Highway 11 overpas	s s			Frucks Heavy) 0	/s Totals 55
Peds Cross: X West Peds: 0 West Entering: 17 West Leg Total: 36	Trucks 2 Tru	rucks 0 avys <u>0</u>	46 59 0 0 0 0 46	S	eds Cross: outh Peds: outh Entering outh Leg Tota	
	Com	nments				

APPENDIX B: 2015 TRAFFIC OPERATIONS

	4	×	1	1	1	Ļ		
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	Y		4			र्भ		
Volume (veh/h)	190	45	25	80	10	45		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	207	49	27	87	11	49		
Pedestrians								
Lane Width (m)								
Walking Speed (m/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			None			None		
Median storage veh)								
Upstream signal (m)								
pX, platoon unblocked								
vC, conflicting volume	141	71			114			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	141	71			114			
tC, single (s)	6.4	6.2			4.1			
tC, 2 stage (s)								
tF (s)	3.5	3.3			2.2			
p0 queue free %	76	95			99			
cM capacity (veh/h)	845	992			1475			
Direction, Lane #	WB 1	NB 1	SB 1					
Volume Total	255	114	60					
Volume Left	207	0	11					
Volume Right	49	87	0					
cSH	870	1700	1475					
Volume to Capacity	0.29	0.07	0.01					
Queue Length 95th (m)	9.3	0.0	0.2					
Control Delay (s)	10.8	0.0	1.4					
Lane LOS	В		А					
Approach Delay (s)	10.8	0.0	1.4					
Approach LOS	В							
Intersection Summary								
Average Delay			6.7					
Intersection Capacity Utiliz	ation		29.5%	IC	U Level o	f Service		
Analysis Period (min)			15					

	1	*	1	1	1	ŧ
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W.		4Î			ર્સ
Volume (veh/h)	45	45	50	165	75	25
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	49	49	54	179	82	27
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	334	144			234	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	334	144			234	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	92	95			94	
cM capacity (veh/h)	621	903			1334	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	98	234	109			
Volume Left	49	234	82			
Volume Right	49	179	02			
cSH	736	1700	1334			
Volume to Capacity	0.13	0.14	0.06			
Queue Length 95th (m)	3.5	0.0	1.5			
Control Delay (s)	10.6	0.0	6.0			
Lane LOS	10.0 B	0.0	0.0 A			
Approach Delay (s)	10.6	0.0	6.0			
Approach LOS	10.0 B	0.0	0.0			
	U					
Intersection Summary			2.0			
Average Delay			3.9			
Intersection Capacity Utiliza	llion		33.5%	IC	CU Level o	Service
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 3: Line 7 & Ridge Rd

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		÷			\$			\$			\$	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	10	10	10	5	30	25	25	150	10	10	25	10
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	11	11	5	33	27	27	163	11	11	27	11
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	33	65	201	49								
Volume Left (vph)	11	5	27	11								
Volume Right (vph)	11	27	11	11								
Hadj (s)	-0.10	-0.20	0.03	-0.05								
Departure Headway (s)	4.4	4.3	4.2	4.3								
Degree Utilization, x	0.04	0.08	0.23	0.06								
Capacity (veh/h)	762	780	831	811								
Control Delay (s)	7.6	7.7	8.5	7.5								
Approach Delay (s)	7.6	7.7	8.5	7.5								
Approach LOS	А	А	А	А								
Intersection Summary												
Delay			8.1									
Level of Service			А									
Intersection Capacity Utiliza	tion		22.7%	IC	CU Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	4		Y	
Volume (veh/h)	10	20	50	10	10	10
Sign Control	10	Free	Free	10	Stop	10
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	22	54	11	11	11
Pedestrians		22	54			
Lane Width (m)						
• •						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)		Nama	Nama			
Median type		None	None			
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked					100	(0
vC, conflicting volume	65				103	60
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	65				103	60
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				99	99
cM capacity (veh/h)	1537				889	1006
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	33	65	22			
Volume Left	11	0	11			
Volume Right	0	11	11			
cSH	1537	1700	944			
Volume to Capacity	0.01	0.04	0.02			
Queue Length 95th (m)	0.01	0.04	0.02			
	2.5	0.0	8.9			
Control Delay (s)		0.0				
Lane LOS	A	0.0	A			
Approach Delay (s)	2.5	0.0	8.9			
Approach LOS			А			
Intersection Summary						
Average Delay			2.3			
Intersection Capacity Utilization	ation		18.3%	IC	CU Level o	of Service
Analysis Period (min)			15			

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्भ	¢Î,	
Volume (veh/h)	20	120	20	5	30	10
Sign Control	Stop	.20	20	Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	22	130	22	5	33	11
Pedestrians	22	130	22	5		11
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)				Mono	None	
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked	07	~~~	10			
vC, conflicting volume	87	38	43			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	87	38	43			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	98	87	99			
cM capacity (veh/h)	901	1034	1565			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	152	27	43			
Volume Left	22	22	0			
Volume Right	130	0	11			
cSH	1013	1565	1700			
Volume to Capacity	0.15	0.01	0.03			
Queue Length 95th (m)	4.0	0.3	0.0			
Control Delay (s)	9.2	5.9	0.0			
Lane LOS	7.2 A	Э.7 А	0.0			
Approach Delay (s)	9.2	5.9	0.0			
Approach LOS	7.2 A	5.7	0.0			
Intersection Summary						
Average Delay			7.0			
Intersection Capacity Utiliza	ation				CU Level o	fSonico
	1001		23.2%	IC	C Level 0	Service
Analysis Period (min)			15			

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			र्स	4Î	
Volume (veh/h)	10	20	120	50	20	20
Sign Control	Stop	20	120	Free	Free	20
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	22	130	54	22	22
Pedestrians		22	150	J4	22	22
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)				Mone	Mono	
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	348	33	43			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	348	33	43			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	98	98	92			
cM capacity (veh/h)	595	1041	1565			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	33	185	43			
Volume Left	11	130	0			
Volume Right	22	0	22			
cSH	833	1565	1700			
Volume to Capacity	0.04	0.08	0.03			
Queue Length 95th (m)	0.9	2.1	0.0			
Control Delay (s)	9.5	5.5	0.0			
Lane LOS	A	A	0.0			
Approach Delay (s)	9.5	5.5	0.0			
Approach LOS	A	0.0	0.0			
Intersection Summary						
Average Delay			5.1			
Intersection Capacity Utiliz	zation		25.9%	10	CU Level a	f Sarvica
Analysis Period (min)			25.9%			
Analysis Penou (min)			10			

HCM Unsignalized Intersection Capacity Analysis 7: Line 9 & Ridge Rd E

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Volume (veh/h)	10	10	15	5	30	20	20	150	10	10	20	10
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	11	16	5	33	22	22	163	11	11	22	11
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	54			27			117	106	19	188	103	43
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	54			27			117	106	19	188	103	43
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			100			97	79	99	98	97	99
cM capacity (veh/h)	1551			1587			826	776	1059	636	779	1027
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	38	60	196	43								
Volume Left	11	5	22	11								
Volume Right	16	22	11	11								
cSH	1551	1587	793	782								
Volume to Capacity	0.01	0.00	0.25	0.06								
Queue Length 95th (m)	0.01	0.00	7.4	1.3								
Control Delay (s)	2.1	0.7	11.0	9.9								
Lane LOS	Α	A	B	Α								
Approach Delay (s)	2.1	0.7	11.0	9.9								
Approach LOS	2.1	0.7	В	A								
Intersection Summary												
Average Delay			8.0									
Intersection Capacity Utiliza	ition		21.8%	IC	CU Level c	of Service			А			
Analysis Period (min)			15									

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Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	Υ		4			र्स	_	
Volume (veh/h)	100	50	20	55	25	25		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	109	54	22	60	27	27		
Pedestrians								
Lane Width (m)								
Walking Speed (m/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			None			None		
Median storage veh)								
Upstream signal (m)								
pX, platoon unblocked								
vC, conflicting volume	133	52			82			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	133	52			82			
tC, single (s)	6.4	6.2			4.1			
tC, 2 stage (s)								
tF (s)	3.5	3.3			2.2			
p0 queue free %	87	95			98			
cM capacity (veh/h)	845	1016			1516			
Direction, Lane #	WB 1	NB 1	SB 1					
Volume Total	163	82	54					
Volume Left	109	0	27					
Volume Right	54	60	0					
cSH	895	1700	1516					
Volume to Capacity	0.18	0.05	0.02					
Queue Length 95th (m)	5.0	0.0	0.4					
Control Delay (s)	9.9	0.0	3.8					
Lane LOS	А		А					
Approach Delay (s)	9.9	0.0	3.8					
Approach LOS	А							
Intersection Summary								
Average Delay			6.1					
Intersection Capacity Utiliza	ation		24.6%	IC	U Level o	f Service		
Analysis Period (min)			15					

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Movement	- WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Ý	HBR	<u>المار</u>	- NDR	UDL	<u>। वट</u>
Volume (veh/h)	6 0	15	40	65	90	*1 85
Sign Control	Stop	15	Free	05	70	Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
	0.92		0.92 43	0.92	0.92 98	0.92 92
Hourly flow rate (vph)	00	16	43	/1	98	92
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	367	79			114	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	367	79			114	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	89	98			93	
cM capacity (veh/h)	591	982			1475	
		NB 1	SB 1			
Direction, Lane #	WB 1					
Volume Total	82	114	190			
Volume Left	65	0	98			
Volume Right	16	71	0			
cSH	642	1700	1475			
Volume to Capacity	0.13	0.07	0.07			
Queue Length 95th (m)	3.3	0.0	1.6			
Control Delay (s)	11.4	0.0	4.2			
Lane LOS	В		А			
Approach Delay (s)	11.4	0.0	4.2			
Approach LOS	В					
Intersection Summary						
Average Delay			4.5			
Intersection Capacity Utiliz	zation		27.0%	IC	U Level o	f Service
Analysis Period (min)			15			
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HCM Unsignalized Intersection Capacity Analysis 3: Line 7 & Ridge Rd

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	10	35	25	15	30	15	10	65	10	20	115	15
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	38	27	16	33	16	11	71	11	22	125	16
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	76	65	92	163								
Volume Left (vph)	11	16	11	22								
Volume Right (vph)	27	16	11	16								
Hadj (s)	-0.15	-0.07	-0.01	0.00								
Departure Headway (s)	4.4	4.5	4.4	4.3								
Degree Utilization, x	0.09	0.08	0.11	0.20								
Capacity (veh/h)	758	742	780	792								
Control Delay (s)	7.9	7.9	8.0	8.4								
Approach Delay (s)	7.9	7.9	8.0	8.4								
Approach LOS	А	А	А	А								
Intersection Summary												
Delay			8.1									
Level of Service			А									
Intersection Capacity Utilizati	ion		23.4%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्भ	4		Y	
Volume (veh/h)	10	60	50	10	10	10
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	65	54	11	11	11
Pedestrians		05	JT			
Lane Width (m)						
. ,						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)		Mone	Marra			
Median type		None	None			
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	65				147	60
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	65				147	60
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				99	99
cM capacity (veh/h)	1537				840	1006
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	76	65	22			
Volume Left	11	0	11			
Volume Right	0	11	11			
cSH	1537	1700	915			
Volume to Capacity	0.01	0.04	0.02			
Queue Length 95th (m)	0.2	0.0	0.6			
Control Delay (s)	1.1	0.0	9.0			
Lane LOS	A	0.0	A			
Approach Delay (s)	1.1	0.0	9.0			
Approach LOS	1.1	0.0	9.0 A			
· · · · · · · · · · · · · · · · · · ·			A			
Intersection Summary						
Average Delay			1.7			
Intersection Capacity Utiliz	ation		20.4%	IC	CU Level o	of Service
Analysis Period (min)			15			

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			र्स	4Î	
Volume (veh/h)	20	60	50	10	15	15
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	22	65	54	11	16	16
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				None	None	
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	144	24	33			
vC1, stage 1 conf vol	144	24	- 33			
vC2, stage 2 conf vol						
vCu, unblocked vol	144	24	33			
tC, single (s)	6.4	6.2	4.1			
	0.4	0.2	4.1			
tC, 2 stage (s) tF (s)	3.5	3.3	2.2			
	3.5 97	3.3 94	97			
p0 queue free %		94 1052	1579			
cM capacity (veh/h)	819	1052	1579			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	87	65	33			
Volume Left	22	54	0			
Volume Right	65	0	16			
cSH	982	1579	1700			
Volume to Capacity	0.09	0.03	0.02			
Queue Length 95th (m)	2.2	0.8	0.0			
Control Delay (s)	9.0	6.2	0.0			
Lane LOS	А	А				
Approach Delay (s)	9.0	6.2	0.0			
Approach LOS	А					
Intersection Summary						
Average Delay			6.4			
Intersection Capacity Utiliz	ation		21.4%	IC	CU Level o	of Service
Analysis Period (min)			15			
			10			

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्भ	4	
Volume (veh/h)	20	45	50	20	65	30
Sign Control	Stop	10	00	Free	Free	50
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	22	49	54	22	71	33
Pedestrians	22	47	J4	22	/ 1	55
Lane Width (m)						
• •						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)				Mono	None	
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked	04-	07	100			
vC, conflicting volume	217	87	103			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	217	87	103			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	97	95	96			
cM capacity (veh/h)	743	972	1489			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	71	76	103			
Volume Left	22	54	0			
Volume Right	49	0	33			
cSH	887	1489	1700			
Volume to Capacity	0.08	0.04	0.06			
Queue Length 95th (m)	2.0	0.9	0.0			
Control Delay (s)	9.4	5.4	0.0			
Lane LOS	A	A	0.0			
Approach Delay (s)	9.4	5.4	0.0			
Approach LOS	A	0.1	0.0			
Intersection Summary						
Average Delay			4.3			
Intersection Capacity Utiliz	ation		4.3 21.0%	10	CU Level of	F Sonvico
Analysis Period (min)	allUIT				JU Level 0	Service
Analysis Penuu (min)			15			

HCM Unsignalized Intersection Capacity Analysis 7: Line 9 & Ridge Rd E

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			÷			÷	
Volume (veh/h)	10	35	20	15	30	10	10	60	10	10	95	10
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	38	22	16	33	11	11	65	11	11	103	11
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	43			60			204	147	49	185	152	38
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	43			60			204	147	49	185	152	38
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			98	91	99	98	86	99
cM capacity (veh/h)	1565			1544			656	732	1020	706	727	1034
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	71	60	87	125								
Volume Left	11	16	11	11								
Volume Right	22	11	11	11								
cSH	1565	1544	747	744								
Volume to Capacity	0.01	0.01	0.12	0.17								
Queue Length 95th (m)	0.2	0.2	3.0	4.6								
Control Delay (s)	1.2	2.1	10.5	10.8								
Lane LOS	А	А	В	В								
Approach Delay (s)	1.2	2.1	10.5	10.8								
Approach LOS			В	В								
Intersection Summary												
Average Delay			7.2									
Intersection Capacity Utiliza	tion		19.3%	IC	CU Level o	of Service			А			
Analysis Period (min)			15									

APPENDIX C: 2025 TRAFFIC OPERATIONS

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Y		4Î			र्स	
Volume (veh/h)	235	55	35	100	15	55	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	255	60	38	109	16	60	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type			None			None	
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	185	92			147		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	185	92			147		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)							
tF (s)	3.5	3.3			2.2		
p0 queue free %	68	94			99		
cM capacity (veh/h)	795	965			1435		
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total	315	147	76				
Volume Left	255	0	16				
Volume Right	60	109	0				
cSH	823	1700	1435				
Volume to Capacity	0.38	0.09	0.01				
Queue Length 95th (m)	13.8	0.0	0.3				
Control Delay (s)	12.1	0.0	1.7				
Lane LOS	В		А				
Approach Delay (s)	12.1	0.0	1.7				
Approach LOS	В						
Intersection Summary							
Average Delay			7.3				
Intersection Capacity Utilization	ation		38.1%	IC	U Level of	Service	
Analysis Period (min)			15				

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	l	
Lane Configurations	Y		4Î			स	1	
Volume (veh/h)	55	55	65	205	75	35		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	60	60	71	223	82	38		
Pedestrians								
Lane Width (m)								
Walking Speed (m/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			None			None		
Median storage veh)								
Upstream signal (m)								
pX, platoon unblocked								
vC, conflicting volume	383	182			293			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	383	182			293			
tC, single (s)	6.4	6.2			4.1			
tC, 2 stage (s)								
tF (s)	3.5	3.3			2.2			
p0 queue free %	90	93			94			
cM capacity (veh/h)	580	860			1268			
Direction, Lane #	WB 1	NB 1	SB 1					
Volume Total	120	293	120					
Volume Left	60	0	82					
Volume Right	60	223	0					
cSH	693	1700	1268					
Volume to Capacity	0.17	0.17	0.06					
Queue Length 95th (m)	4.7	0.0	1.6					
Control Delay (s)	11.3	0.0	5.6					
Lane LOS	В		А					
Approach Delay (s)	11.3	0.0	5.6					
Approach LOS	В							
Intersection Summary								
Average Delay			3.8					
Intersection Capacity Utiliza	ation		38.5%	IC	U Level o	f Service		
Analysis Period (min)	-		15					
			.5					

HCM Unsignalized Intersection Capacity Analysis 3: Line 7 & Ridge Rd

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		÷			\$			\$			\$	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	16	15	25	10	40	30	35	170	15	15	30	15
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	17	16	27	11	43	33	38	185	16	16	33	16
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	61	87	239	65								
Volume Left (vph)	17	11	38	16								
Volume Right (vph)	27	33	16	16								
Hadj (s)	-0.18	-0.17	0.02	-0.07								
Departure Headway (s)	4.5	4.5	4.4	4.4								
Degree Utilization, x	0.08	0.11	0.29	0.08								
Capacity (veh/h)	730	738	799	762								
Control Delay (s)	7.9	8.1	9.1	7.8								
Approach Delay (s)	7.9	8.1	9.1	7.8								
Approach LOS	А	А	А	А								
Intersection Summary												
Delay			8.6									
Level of Service			А									
Intersection Capacity Utiliza	tion		26.7%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		स	4		¥	
Volume (veh/h)	15	25	65	15	15	15
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	27	71	16	16	16
Pedestrians	10	21	/ 1	10	10	10
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)		Mono	Mono			
Median type		None	None			
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked					4.5.5	
vC, conflicting volume	87				139	79
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	87				139	79
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				98	98
cM capacity (veh/h)	1509				845	982
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	43	87	33			
Volume Left	16	0	16			
Volume Right	0	16	16			
cSH	1509	1700	908			
Volume to Capacity	0.01	0.05	0.04			
Queue Length 95th (m)	0.01	0.05	0.04			
	2.8					
Control Delay (s)		0.0	9.1			
Lane LOS	A	0.0	A			
Approach Delay (s)	2.8	0.0	9.1			
Approach LOS			A			
Intersection Summary						
Average Delay			2.6			
Intersection Capacity Utilization	ation		18.8%	IC	CU Level o	of Service
Analysis Period (min)			15			

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			र्स	¢Î,	
Volume (veh/h)	25	150	25	10	40	15
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	27	163	27	11	43	16
Pedestrians					10	
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				NULLE	NULL	
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	117	52	60			
vC1, stage 1 conf vol	11/	52	00			
vC2, stage 2 conf vol vCu, unblocked vol	117	52	60			
		6.2	4.1			
tC, single (s)	6.4	0.2	4.1			
tC, 2 stage (s)	2 5	2.2	2.2			
tF (s)	3.5	3.3	2.2			
p0 queue free %	97	84	98			
cM capacity (veh/h)	864	1016	1544			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	190	38	60			
Volume Left	27	27	0			
Volume Right	163	0	16			
cSH	991	1544	1700			
Volume to Capacity	0.19	0.02	0.04			
Queue Length 95th (m)	5.4	0.4	0.0			
Control Delay (s)	9.5	5.3	0.0			
Lane LOS	А	А				
Approach Delay (s)	9.5	5.3	0.0			
Approach LOS	А					
Intersection Summary						
Average Delay			7.0			
Intersection Capacity Utiliza	ation		25.9%	IC	CU Level o	f Service
Analysis Period (min)			15		5 201010	
			10			

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	4	
Volume (veh/h)	15	25	150	65	25	25
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	27	163	71	27	27
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				NULLE	NULLE	
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	438	41	54			
	430	41	54			
vC1, stage 1 conf vol vC2, stage 2 conf vol						
vCu, unblocked vol	438	41	54			
	430 6.4	6.2	4.1			
tC, single (s)	0.4	0.2	4.1			
tC, 2 stage (s)	ЭΓ	2.2	2.2			
tF (s)	3.5	3.3	2.2			
p0 queue free %	97	97	89			
cM capacity (veh/h)	516	1030	1551			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	43	234	54			
Volume Left	16	163	0			
Volume Right	27	0	27			
cSH	750	1551	1700			
Volume to Capacity	0.06	0.11	0.03			
Queue Length 95th (m)	1.4	2.7	0.0			
Control Delay (s)	10.1	5.5	0.0			
Lane LOS	В	А				
Approach Delay (s)	10.1	5.5	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			5.2			
Intersection Capacity Utiliza	ation		28.4%	IC	CU Level o	f Service
Analysis Period (min)	-		15			
			10			

HCM Unsignalized Intersection Capacity Analysis 7: Line 9 & Ridge Rd E

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		÷			\$			\$			\$	
Volume (veh/h)	15	15	20	10	40	25	25	170	15	15	25	15
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	16	22	11	43	27	27	185	16	16	27	16
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	71			38			168	152	27	247	149	57
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	71			38			168	152	27	247	149	57
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			96	75	98	97	96	98
cM capacity (veh/h)	1530			1572			750	727	1048	552	729	1009
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	54	82	228	60								
Volume Left	16	11	27	16								
Volume Right	22	27	16	16								
cSH	1530	1572	746	720								
Volume to Capacity	0.01	0.01	0.31	0.08								
Queue Length 95th (m)	0.2	0.2	9.9	2.1								
Control Delay (s)	2.3	1.0	11.9	10.4								
Lane LOS	А	А	В	В								
Approach Delay (s)	2.3	1.0	11.9	10.4								
Approach LOS			В	В								
Intersection Summary												
Average Delay			8.4									
Intersection Capacity Utilizat	tion		24.5%	IC	CU Level c	of Service			А			
Analysis Period (min)			15									

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Movement	- WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y	WBR(¢,	H BR	ODL	4
Volume (veh/h)	125	65	25	70	35	35
Sign Control	Stop	05	Free	70	55	Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.02	0.92	0.02	0.92	0.92
	136	0.92 71	0.92	0.92 76	0.92	
Hourly flow rate (vph)	130	/1	21	/0	38	38
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	179	65			103	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	179	65			103	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	83	93			97	
cM capacity (veh/h)	790	999			1489	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	207	103	76			
Volume Left	136	0	38			
Volume Right	71	76	0			
cSH	850	1700	1489			
Volume to Capacity	0.24	0.06	0.03			
Queue Length 95th (m)	7.2	0.0	0.6			
Control Delay (s)	10.6	0.0	3.8			
Lane LOS	В		А			
Approach Delay (s)	10.6	0.0	3.8			
Approach LOS	В					
Intersection Summary						
Average Delay			6.4			
Intersection Capacity Utiliz	zation		28.0%	IC	U Level o	f Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Y		<u>المار</u>	NER		<u>्व</u>	
Volume (veh/h)	75	20	50	80	110	105	
Sign Control	Stop	20	Free	00	110	Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
		0.92	0.92 54				
Hourly flow rate (vph)	82	22	54	87	120	114	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type			None			None	
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	451	98			141		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	451	98			141		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)							
tF (s)	3.5	3.3			2.2		
p0 queue free %	84	98			92		
cM capacity (veh/h)	519	958			1442		
			CD 1				
Direction, Lane #	WB 1	NB 1	SB 1				_
Volume Total	103	141	234				
Volume Left	82	0	120				
Volume Right	22	87	0				
cSH	575	1700	1442				
Volume to Capacity	0.18	0.08	0.08				
Queue Length 95th (m)	4.9	0.0	2.1				
Control Delay (s)	12.6	0.0	4.3				
Lane LOS	В		А				
Approach Delay (s)	12.6	0.0	4.3				
Approach LOS	В						
Intersection Summary							
Average Delay			4.8				
Intersection Capacity Utiliz	ation		34.5%	IC	U Level o	f Service	
Analysis Period (min)			15				
			10				

HCM Unsignalized Intersection Capacity Analysis 3: Line 7 & Ridge Rd

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	15	15	25	20	40	20	15	75	10	25	130	20
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	16	27	22	43	22	16	82	11	27	141	22
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	60	87	109	190								
Volume Left (vph)	16	22	16	27								
Volume Right (vph)	27	22	11	22								
Hadj (s)	-0.18	-0.07	0.00	-0.01								
Departure Headway (s)	4.5	4.6	4.5	4.4								
Degree Utilization, x	0.07	0.11	0.13	0.23								
Capacity (veh/h)	733	725	767	786								
Control Delay (s)	7.9	8.2	8.2	8.7								
Approach Delay (s)	7.9	8.2	8.2	8.7								
Approach LOS	А	А	А	А								
Intersection Summary												
Delay			8.3									
Level of Service			А									
Intersection Capacity Utiliza	tion		24.8%	IC	U Level o	of Service	t.		А			
Analysis Period (min)			15									

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Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		র্শ	4Î		Y		
Volume (veh/h)	15	75	65	15	15	15	
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	16	82	71	16	16	16	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type		None	None				
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	87				193	79	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	87				193	79	
tC, single (s)	4.1				6.4	6.2	
tC, 2 stage (s)							
tF (s)	2.2				3.5	3.3	
p0 queue free %	99				98	98	
cM capacity (veh/h)	1509				787	982	
Direction, Lane #	EB 1	WB 1	SB 1				
Volume Total	98	87	33				
Volume Left	90 16	0	16				
Volume Right	0	16	16				
cSH	1509	1700	874				
Volume to Capacity	0.01	0.05	0.04				
Queue Length 95th (m)	0.01	0.05	0.04				
Control Delay (s)	1.3	0.0	9.3				
Lane LOS	1.3 A	0.0	9.3 A				
Approach Delay (s)	1.3	0.0	9.3				
Approach LOS	1.0	0.0	9.3 A				
Intersection Summary							
Average Delay			2.0				
Intersection Capacity Utilizati	on		21.4%	10	CU Level d	of Service	
Analysis Period (min)			15	I.			
niaiysis r chuu (11111)			10				

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्भ	f,	
Volume (veh/h)	25	75	65	15	20	20
Sign Control	Stop		00	Free	Free	20
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	27	82	71	16	22	22
Pedestrians	21	02	/ 1	10	22	22
Lane Width (m)						
· ·						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)				Nama	Mana	
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked			10			
vC, conflicting volume	190	33	43			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	190	33	43			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	96	92	95			
cM capacity (veh/h)	763	1041	1565			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	109	87	43			
Volume Left	27	71	0			
Volume Right	82	0	22			
cSH	954	1565	1700			
Volume to Capacity	0.11	0.05	0.03			
Queue Length 95th (m)	2.9	1.1	0.0			
Control Delay (s)	9.3	6.1	0.0			
Lane LOS	A	A	0.0			
Approach Delay (s)	9.3	6.1	0.0			
Approach LOS	A	0.1	0.0			
Intersection Summary						
Average Delay			6.4			
Intersection Capacity Utiliza	ation		23.7%	10	CU Level o	fSorvice
Analysis Period (min)				IC	O Level 0	Service
Analysis Penda (MIN)			15			

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	¢Î,	
Volume (veh/h)	25	55	65	25	80	40
Sign Control	Stop	00	00	Free	Free	10
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	27	60	71	27	87	43
Pedestrians	21	00	/ 1	21	07	73
Lane Width (m)						
• •						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)				Mono	None	
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked		4.5.5				
vC, conflicting volume	277	109	130			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	277	109	130			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	96	94	95			
cM capacity (veh/h)	678	945	1455			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	87	98	130			
Volume Left	27	71	0			
Volume Right	60	0	43			
cSH	841	1455	1700			
Volume to Capacity	0.10	0.05	0.08			
Queue Length 95th (m)	2.6	1.2	0.0			
Control Delay (s)	9.8	5.6	0.0			
Lane LOS	A	A	0.0			
Approach Delay (s)	9.8	5.6	0.0			
Approach LOS	A	0.0	0.0			
Intersection Summary						
Average Delay			4.4			
Intersection Capacity Utiliza	ation		23.0%	IC	CU Level o	f Service
Analysis Period (min)			15			Scivice
			15			

HCM Unsignalized Intersection Capacity Analysis 7: Line 9 & Ridge Rd E

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Volume (veh/h)	15	45	25	20	40	15	15	70	15	15	105	15
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	49	27	22	43	16	16	76	16	16	114	16
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	60			76			264	198	62	245	204	52
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	60			76			264	198	62	245	204	52
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			97	89	98	97	83	98
cM capacity (veh/h)	1544			1523			579	680	1002	626	675	1016
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	92	82	109	147								
Volume Left	16	22	16	16								
Volume Right	27	16	16	16								
cSH	1544	1523	696	695								
Volume to Capacity	0.01	0.01	0.16	0.21								
Queue Length 95th (m)	0.2	0.3	4.2	6.0								
Control Delay (s)	1.4	2.1	11.1	11.6								
Lane LOS	А	А	В	В								
Approach Delay (s)	1.4	2.1	11.1	11.6								
Approach LOS			В	В								
Intersection Summary												
Average Delay			7.5									
Intersection Capacity Utiliza	ition		22.2%	IC	CU Level o	of Service			А			
Analysis Period (min)			15									

APPENDIX D: 2015 BURL'S CREEK EVENT CALENDAR

FARMER'S MARKET	FESTIVAL	SINGLE DAY EVENT	FLEA MARKET	COMMUNITY
UNCONFIRMED	FESTIVAL SET UP/TEAR DOWN	EVENT SET UP/TEAR DOWN	MARKET SET UP/TEAR DOWN	SOCCER

May 2015

Mon	Tue	Wed	Thu	Fri	Sat	Sun
				1	2	3
4	5	6	7	8	9	10
		SOCCER				
11	12	13	14	15	16	17
		SOCCER				
18	19	20	21	22	23	24
		SOCCER				
25	26	27	28	29	30	31
		SOCCER				

June 2015

Mon	Tue	Wed	Thu	Fri	Sat	Sun
1	2	3	4	5	6	7
			BARRIE AUTOMOTIVE FLEA MARKET	BARRIE AUTOMOTIVE FLEA MARKET	BARRIE AUTOMOTIVE FLEA MARKET	BARRIE AUTOMOTIVE FLEA MARKET
8	9	10	11	12	13	14
		SOCCER		FARMER'S MARKET		
15	16	17	18	19	20	21
		SOCCER		FARMER'S MARKET		
22	23	24	25	26	27	28
		SOCCER		FARMER'S MARKET	CONTEMORARY MUSIC CONCERT	
29	30					

July 2015

Mon	Tue	Wed	Thu	Fri	Sat	Sun
		1	2	3	4	5
				FARMER'S MARKET		
6	7	8	9	10	11	12
		SOCCER		FARMER'S MARKET		
13	14	15	16	17	18	19
		SOCCER		FARMER'S MARKET		
20	21	22	23	24	25	26
				WAYHOME	WAYHOME	WAYHOME
27	28	29	30	31		
		SOCCER		FARMER'S MARKET		

August 2015

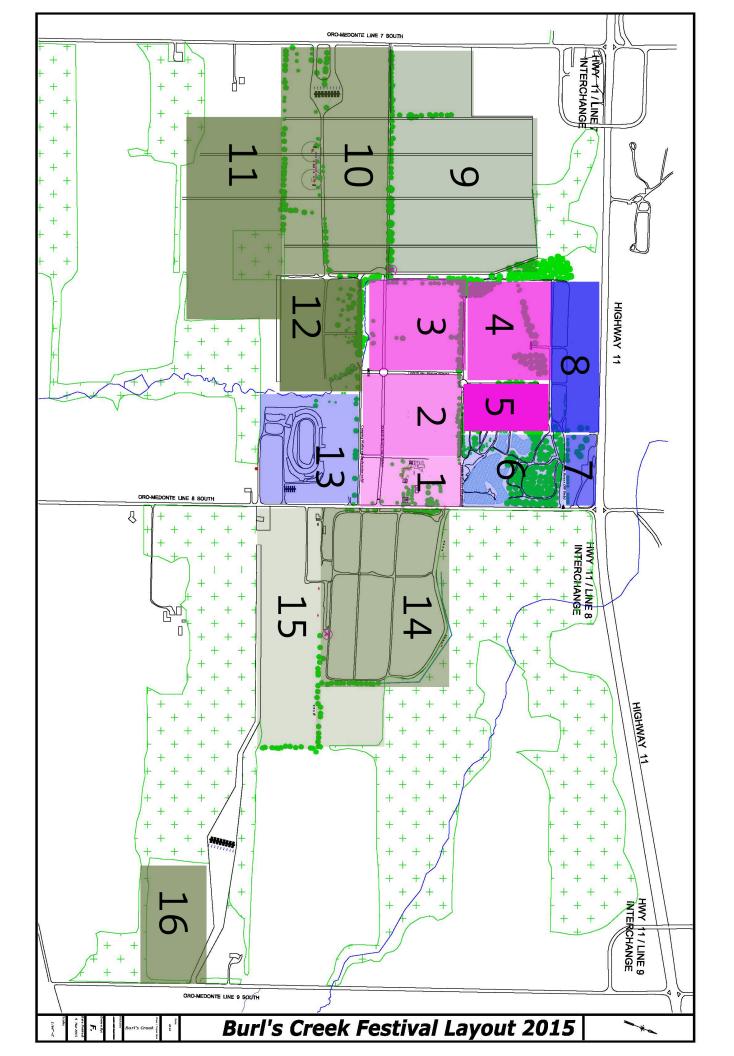
Mon	Tue	Wed	Thu	Fri	Sat	Sun
					1	2
3	4	5	6	7	8	9
			BOOTS & HEARTS	BOOTS & HEARTS	BOOTS & HEARTS	BOOTS & HEARTS
10	11	12	13	14	15	16
		SOCCER		FARMER'S MARKET	TOUGH MUDDER PARKING	
17	18	19	20	21	22	23
		SOCCER		FARMER'S MARKET		
24	25	26	27	28	29	30
		SOCCER		FARMER'S MARKET		
31						

September 2015

Mon	Tue	Wed	Thu	Fri	Sat	Sun
	1	2	3	4	5	6
				FARMER'S MARKET		
7	8	9	10	11	12	13
			BARRIE AUTOMOTIVE FLEA MARKET	BARRIE AUTOMOTIVE FLEA MARKET	BARRIE AUTOMOTIVE FLEA MARKET	BARRIE AUTOMOTIVE FLEA MARKET
14	15	16	17	18	19	20
				FARMER'S MARKET		
21	22	23	24	25	26	27
				FARMER'S MARKET		HURONIA FUR & FEATHERS
28	29	30				

October 2015

	2 ARMER'S	3 4
	MARKET	
8	9	10 11
15	16	17 18
	8 F	FARMER'S MARKET



APPENDIX E: BURL'S CREEK TRAFFIC VOLUMES

Huronia Fur & Feathers - Peak Hour Volumes (Sunday mid-morning)

2	2015	Bacl Al	k ground T i M	affic Volu Pl			Site Traffi M		s PM	T A		c Volumes Pl		A	Volume/C		ΡM	Total
	.015	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	Capacity
Hwy 11	Fri	2,540	1,260	2,820	1,525					2,540	1,260	2,820	1,525					3,270
	Sat	2,520	1,585	2,475	1,640					2,520	1,585	2,475	1,640					3,270
	Sun	1,755	2,215	1,715	2,605	67	67			1,822	2,282	1,715	2,605	0.56	0.70			3,270
	Weekday	1,520	1,225	2,005	1,420					1,520	1,225	2,005	1,420					3,270
SR 20	Fri	35	55	55	45					35	55	55	45					800
Line 3-7	Sat	40	65	70	55					40	65	70	55					800
	Sun	35	55	60	50	7	7			42	62	60	50	0.05	0.08			800
	Weekday	35	50	55	40					35	50	55	40					800
SR 20	Fri	30	50	60	45					30	50	60	45					800
Line7-11	Sat	30	60	65	60					30	60	65	60					800
	Sun	30	50	60	55	7	7			37	57	60	55	0.05	0.07			800
	Weekday	25	45	50	45					25	45	50	45					800
Line 7	Fri	60	175	120	85					60	175	120	85					600
	Sat	70	215	145	105					70	215	145	105					600
	Sun	65	190	130	90	120	120			185	310	130	90	0.31	0.52			600
	Weekday	55	165	110	80					55	165	110	80					600
Line 8	Fri	10	5	10	15					10	5	10	15					600
	Sat	10	10	10	15					10	10	10	15					600
	Sun	10	5	10	15					10	5	10	15					600
	Weekday	10	5	5	15					10	5	5	15					600
Line 9	Fri	30	140	90	60					30	140	90	60					600
	Sat	40	170	110	70					40	170	110	70					600
	Sun	30	155	100	65					30	155	100	65					600
	Weekday	30	135	85	55					30	135	85	55					600

		Bacl	kground Tr	affic Volur	mes		Site Traffic	c Volumes	5		T	otal Traffic	c Volumes	i i		Volume/	Capacity		Total
	2020	A	M	PI	N	A	М	F	PM		AM	Л	PI	N	A	N	F	PM	Capacity
		NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB		NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	Capacity
Hwy 11	Fri	2,685	1,335	2,980	1,610						2,685	1,335	2,980	1,610					3,270
	Sat	2,660	1,675	2,615	1,730						2,660	1,675	2,615	1,730					3,270
	Sun	1,855	2,340	1,815	2,745	67	67				1,922	2,407	1,815	2,745	0.59	0.74			3,270
	Weekday	1,695	1,295	2,240	1,580						1,695	1,295	2,240	1,580					3,270
SR 20	Fri	50	70	70	60						50	70	70	60					800
Line 3-7	Sat	55	80	85	70						55	80	85	70					800
		5.0	=0			_	_												
	Sun	50	70	75	65	7	7				57	77	75	65	0.07	0.10			800
	Weekday	50	65	70	55						50	65	70	55					800
SR 20	Fri	45	65	75	60						45	65	75	60					800
Line7-11	Sat	45	75	80	75						45	75	80	75					800
	Sun	45	65	75	70	7	7				52	72	75	70	0.06	0.09			800
	Weekday	40	60	65	60						40	60	65	60					800
Line 7	Fri	70	195	140	100						70	195	140	100					600
	Sat	80	245	165	120						80	245	165	120					600
	Sun	75	215	150	105	120	120				195	335	150	105	0.33	0.56			600
	Weekday	65	185	125	95						65	185	125	95					600
Line 8	Fri	15	10	15	20						15	10	15	20					600
	Sat	15	15	15	20						15	15	15	20					600
	Sun	15	10	15	20						15	10	15	20					600
	Weekday	15	10	10	20					_	15	10	10	20					600
Line 9	Fri	40	160	105	70					_	40	160	105	70					600
	Sat	50	195	125	85						50	195	125	85					600
	Sun	40	175	115	75						40	175	115	75					600
	Weekday	40	150	100	65						40	150	100	65					600
_																			

		Bacl	kground Ti	raffic Volu	mes		Site Traffi	c Volumes	6	1	Total Traffi	c Volumes	i		Volume/	Capacity		Total
1	2025	A	M	Р	M	A	M	F	PM	A	N	PI	N	A	M	I	PM	
		NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	Capacity
Hwy 11	Fri	2,820	1,410	3,145	1,700					2,820	1,410	3,145	1,700					3,270
-	Sat	2,805	1,770	2,760	1,825					2,805	1,770	2,760	1,825					3,270
	Sun	1,955	2,465	1,920	2,890	67	67			2,022	2,532	1,920	2,890	0.62	0.77			3,270
	Weekday	1,695	1,370	2,240	1,580					1,695	1,370	2,240	1,580					3,270
SR 20	Fri	50	75	75	65					50	75	75	65					800
Line 3-7	Sat	55	90	95	75					55	90	95	75					800
	Sun	50	75	85	70	7	7			57	82	85	70	0.07	0.10			800
	Weekday	50	65	75	60					50	65	75	60					800
SR 20	Fri	45	70	80	65					45	70	80	65					800
Line7-11	Sat	45	80	85	80					45	80	85	80					800
	Sun	45	70	80	75	7	7			52	77	80	75	0.06	0.10			800
	Weekday	40	60	70	65					40	60	70	65					800
Line 7	Fri	75	215	155	110					75	215	155	110					600

		10	210	100	110			70	210	100	110			000
	Sat	90	270	180	130			90	270	180	130			600
	Sun	85	235	165	115	120	120	205	355	165	115	0.34	0.59	600
	Weekday	70	205	135	105			70	205	135	105			600
Line 8	Fri	15	10	15	20			15	10	15	20			 600
	Sat	15	15	15	20			15	15	15	20			600
	Sun	15	10	15	20			15	10	15	20			600
	Weekday	15	10	10	20			15	10	10	20			600
Line 9	Fri	40	175	115	75			40	175	115	75			600
	Sat	50	215	135	90			50	215	135	90			600
	Sun	40	190	125	80			40	190	125	80			600
	Weekday	40	170	110	70			40	170	110	70			 600
					_									

Farmers Market - Peak Hour Volumes (Friday evening)

			kground Tr					c Volumes				c Volumes				Capacity		Total
2	2015	A		PI			M		М	A		PI			M	PI		Capacity
		NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	oupdeity
Hwy 11	Fri	2,540	1,260	2,820	1,525			75	50	2,540	1,260	2,895	1,575			0.89	0.48	3,270
	Sat	2,520	1,585	2,475	1,640					2,520	1,585	2,475	1,640					3,270
	Sun	1,755	2,215	1,715	2,605					1,755	2,215	1,715	2,605					3,270
	Weekday	1,520	1,225	2,005	1,420					1,520	1,225	2,005	1,420					3,270
SR 20	Fri	35	55	55	45			4	4	35	55	59	49			0.07	0.06	800
Line 3-7	Sat	40	65	70	55					40	65	70	55					800
	Sun	35	55	60	50					35	55	60	50					800
	Weekday	35	50	55	40					35	50	55	40					800
SR 20	Fri	30	50	60	45			4	4	30	50	64	49			0.08	0.06	800
Line7-11	Sat	30	60	65	60					30	60	65	60					800
	Sun	30	50	60	55					30	50	60	55					800
	Weekday	25	45	50	45					25	45	50	45					800
Line 7	Fri	60	175	120	85					60	175	120	85					600
	Sat	70	215	145	105					70	215	145	105					600
	Sun	65	190	130	90					65	190	130	90					600
	Weekday	55	165	110	80					55	165	110	80					600
Line 8	Fri	10	5	10	15			75	75	10	5	85	90			0.14	0.15	600
	Sat	10	10	10	15					10	10	10	15					600
	Sun	10	5	10	15					10	5	10	15					600
	Weekday	10	5	5	15					10	5	5	15					600
Line 9	Fri	30	140	90	60			50		30	140	140	60			0.23		600
	Sat	40	170	110	70					40	170	110	70					600
	Sun	30	155	100	65					30	155	100	65					600
	Weekday	30	135	85	55					30	135	85	55					600

	0000		ground Tr	affic Volui Pl				c Volumes	M		otal Traffi	c Volumes Pl		,		Capacity PN	٨	Total
	2020	An NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	AM SB/WB	NB/EB	SB/WB	Al NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	AM SB/WB	NB/EB	SB/WB	Capacity
Hwy 11	Fri	2,685	1,335	2,980	1,610	ND/LD	30/110	75	50	2,685	1,335	3,055	1,660	ND/LD	30/00	0.93	0.51	3,270
1100 y 11	Sat	2,660	1,675	2,900	1,010			75	50	2,660	1,675	2,615	1,730			0.75	0.51	3,270
	Sun	1,855	2,340	1,815	2,745					1,855	2,340	1,815	2,745					3,270
	Weekday	1,695	1,295	2,240	1,580					1,695	1,295	2,240	1,580					3,270
SR 20	Fri	50	70	70	60			4	4	50	70	74	64			0.09	0.08	800
517.20		50	10	10	00				1	50	70	7-1	01			0.07	0.00	000
Line 3-7	Sat	55	80	85	70					55	80	85	70					800
	Sun	50	70	75	65					50	70	75	65					800
	Weekday	50	65	70	55					50	65	70	55					800
SR 20	Fri	45	65	75	60			4	4	45	65	79	64			0.10	0.08	800
Line7-11	Sat	45	75	80	75					45	75	80	75					800
	Sun	45	65	75	70					45	65	75	70					800
	Weekday	40	60	65	60					40	60	65	60					800
Line 7	Fri	70	195	140	100					70	195	140	100					600
	Sat	80	245	165	120					80	245	165	120					600
	Sun	75	215	150	105					75	215	150	105					600
	Weekday	65	185	125	95					65	185	125	95					600
Line 8	Fri	15	10	15	20			75	75	15	10	90	95			0.15	0.16	600
	Sat	15	15	15	20					15	15	15	20					600
	Sun	15	10	15	20					15	10	15	20					600
	Weekday	15	10	10	20					15	10	10	20					600
Line 9	Fri	40	160	105	70			50		40	160	155	70			0.26		600
	Sat	50	195	125	85					50	195	125	85					600
	Sun	40	175	115	75					40	175	115	75					600
	Weekday	40	150	100	65					40	150	100	65					600

		Back	kground Tr	affic Volu	nes		Site Traffi	c Volumes			Total Traffi	c Volumes	;		Volume	Capacity		Total
9	2025	A	M	PI	N		AM	Р	M	A	M	PI	M	ŀ	M	P	M	
		NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	Capacity
Hwy 11	Fri	2,820	1,410	3,145	1,700			75	50	2,820	1,410	3,220	1,750			0.98	0.54	3,270
	Sat	2,805	1,770	2,760	1,825					2,805	1,770	2,760	1,825					3,270
	Sun	1,955	2,465	1,920	2,890					1,955	2,465	1,920	2,890					3,270
	Weekday	1,695	1,370	2,240	1,580					1,695	1,370	2,240	1,580					3,270
SR 20	Fri	50	75	75	65			4	4	50	75	79	69			0.10	0.09	800
Line 3-7	Sat	55	90	95	75					55	90	95	75					800
	Sun	50	75	85	70					50	75	85	70					800
	Weekday	50	65	75	60					50	65	75	60					800
SR 20	Fri	45	70	80	65			4	4	45	70	84	69			0.11	0.09	800
Line7-11	Sat	45	80	85	80					45	80	85	80					800
	Sun	45	70	80	75					45	70	80	75					800
	Weekday	40	60	70	65					40	60	70	65					800
Line 7	Fri	75	215	155	110					75	215	155	110					600

2010 1			210	100					2.10					000
	Sat	90	270	180	130			90	270	180	130			600
	Sun	85	235	165	115			85	235	165	115			600
	Weekday	70	205	135	105			70	205	135	105			600
Line 8	Fri	15	10	15	20	75	75	15	10	90	95	0.1	5 0.16	600
	Sat	15	15	15	20			15	15	15	20			600
	Sun	15	10	15	20			15	10	15	20			600
	Weekday	15	10	10	20			15	10	10	20			600
Line 9	Fri	40	175	115	75	50		40	175	165	75	0.2	8	600
	Sat	50	215	135	90			50	215	135	90			600
	Sun	40	190	125	80			40	190	125	80			600
	Weekday	40	170	110	70			40	170	110	70			600

Minor Soccer - Peak Hour Volumes (Wednesday evening)

Future Traffic Volumes

			kground Tr					c Volumes				c Volumes				Capacity		Total
2	2015	AI		PI			M	P		Al ND/FD		PI			M	P		Capacity
		NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	
Hwy 11	Fri	2,540	1,260	2,820	1,525					2,540	1,260	2,820	1,525					3,270
	Sat	2,520	1,585	2,475	1,640					2,520	1,585		1,640					3,270
	Sun	1,755	2,215	1,715	2,605					1,755	2,215		2,605					3,270
	Weekday	1,520	1,225	2,005	1,420			150	150	1,520	1,225		1,570			0.66	0.48	3,270
SR 20	Fri	35	55	55	45					35	55	55	45					800
Line 3-7	Sat	40	65	70	55					40	65	70	55					800
	Sun	35	55	60	50					35	55	60	50					800
	Weekday	35	50	55	40			25	25	35	50	80	65			0.10	0.08	800
SR 20	Fri	30	50	60	45					30	50	60	45					800
Line7-11	Sat	30	60	65	60					30	60	65	60					800
	Sun	30	50	60	55					30	50	60	55					800
	Weekday	25	45	50	45			(25)	25	25	45	75	70			0.09	0.09	800
Line 7	Fri	60	175	120	85					60	175	120	85					600
	Sat	70	215	145	105					70	215	145	105					600
	Sun	65	190	130	90					65	190	130	90					600
	Weekday	55	165	110	80			450	450	55	165	560	530			0.93	0.88	600
Line 8	Fri	10	5	10	15					10	5	10	15					600
	Sat	10	10	10	15					10	10	10	15					600
	Sun	10	5	10	15					10	5	10	15					600
	Weekday	10	5	5	15					10	5	5	15					600
Line 9	Fri	30	140	90	60					30	140	90	60					600
	Sat	40	170	110	70					40	170	110	70					600
	Sun	30	155	100	65					30	155	100	65					600
	Weekday	30	135	85	55					30	135	85	55					600
	vicenuay		133	00							133	00	55					000

9	2020	Back AM	kground Tr	affic Volur Pl		l	Site Traffi	c Volum es Pl		۲ AI	otal Traffi	c Volumes Pl		A	Volume/	Capacity Pl	M	Total
	.020	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	Capacity
Hwy 11	Fri	2,685	1,335	2,980	1,610					2,685	1,335	2,980	1,610					3,270
,	Sat	2,660	1,675	2,615	1,730					2,660	1,675	2,615	1,730					3,270
	Sun	1,855	2,340	1,815	2,745					1,855	2,340	1,815	2,745					3,270
	Weekday	1,695	1,295	2,240	1,580			150	150	1,695	1,295	2,390	1,730			0.73	0.53	3,270
SR 20	Fri	50	70	70	60					50	70	70	60					800
Line 3-7	Sat	55	80	85	70					55	80	85	70					800
	0	50	70	75						50	70							
	Sun	50	70	75	65			05	05	50	70	75	65			0.10	0.10	800
<u> </u>	Weekday	50	65	70	55			25	25	50	65	95	80			0.12	0.10	800
SR 20	Fri	45	65	75	60					45	65	75	60					800
Line7-11	Sat	45	75	80	75					45	75	80	75					800
	Sun	45	65	75	70			25	25	45	65	75	70			0.11	0.11	800
11	Weekday	40	60	65	60			25	25	40	60	90	85			0.11	0.11	800
Line 7	Fri	70	195	140	100					70	195	140	100					600
	Sat	80	245	165	120					80	245	165	120					600
	Sun	75	215	150	105			450	450	75	215	150	105			0.07	0.01	600
Line 0	Weekday	65	185 10	125 15	<u>95</u> 20			450	450	65	185 10	575 15	545			0.96	0.91	<u> </u>
Line 8	Fri	15								15			20					
	Sat Sun	15 15	15 10	15 15	20 20					15 15	15 10	15 15	20 20					600 600
		15	10	15	20 20					15	10							600
Line 9	Weekday Fri	40	160	105	70	. <u> </u>				40	160	10 105	<u>20</u> 70					600
	Sat	40 50	100	105	70 85					40 50	100	105	70 85					600
	Sun	50 40	195	125	85 75					50 40	195	125	85 75					600
	Weekday	40 40	175	115	65					40 40	175	115	65					600
	weekudy	40	100	100	00					40	100	100	00					000

		Back	ground Tr	raffic Volur			Site Traffi				T	otal Traffic	c Volumes			Volume/	Capacity		Total
2	2025	AN	Л	PN	N	1	AM	P	M		AN		PN	Л	ł	AM	P	M	Capacity
		NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB		NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	Capacity
Hwy 11	Fri	2,820	1,410	3,145	1,700						2,820	1,410	3,145	1,700					3,270
	Sat	2,805	1,770	2,760	1,825						2,805	1,770	2,760	1,825					3,270
	Sun	1,955	2,465	1,920	2,890						1,955	2,465	1,920	2,890					3,270
	Weekday	1,695	1,370	2,240	1,580			150	150		1,695	1,370	2,390	1,730			0.73	0.53	3,270
SR 20	Fri	50	75	75	65						50	75	75	65					800
Line 3-7	Sat	55	90	95	75						55	90	95	75					800
	Sun	50	75	85	70						50	75	85	70					800
	Weekday	50	65	75	60			25	25	_	50	65	100	85			0.13	0.11	800
SR 20	Fri	45	70	80	65						45	70	80	65					800
Line7-11	Sat	45	80	85	80						45	80	85	80					800
	Sun	45	70	80	75						45	70	80	75					800
	Weekday	40	60	70	65			25	25		40	60	95	90			0.12	0.11	800
Line 7	Fri	75	215	155	110						75	215	155	110					600
	Sat	90	270	180	130						90	270	180	130					600
	Sun	85	235	165	115						85	235	165	115					600
	Weekday	70	205	135	105			450	450		70	205	585	555			0.98	0.93	600
Line 8	Fri	15	10	15	20						15	10	15	20					600
	Sat	15	15	15	20						15	15	15	20					600
	Sun	15	10	15	20						15	10	15	20					600
	Weekday	15	10	10	20						15	10	10	20					600
Line 9	Fri	40	175	115	75						40	175	115	75					600
	Sat	50	215	135	90						50	215	135	90					600
	Sun	40	190	125	80						40	190	125	80					600
	Weekday	40	170	110	70					_	40	170	110	70					600

Note: for site traffic, consider the peak directional volumes from both peak hours (ie. consider the maximum volumes)

Tough Mudder - Peak Hour Volumes (Saturday)

ZUID AM PM AM PM AM PM AM PM Marce SB/res SB/res				kground Ti				Site Traffic				Fotal Traffi				Volume/0			Total
Hwy 11 Fri 2,540 1,260 2,800 1,755 2,215 1,715 2,002 0,825 3,270	2	2015																	
Sat 2,520 1,585 2,475 1,640 500 275 250 400 3,020 1,860 2,725 2,040 0,92 0,57 0,83 0,62 3,270 Sn 1,725 2,215 1,715 2,605 1,725 2,215 1,715 2,605 3,270 3,270 SR 20 Fri 35 55 55 45 1,520 1,225 2,005 1,420 3,270 SR 20 Fri 35 55 55 45 35 55 45 800 SR 20 Fri 30 50 60 55 40 35 55 40 800 SR 20 Fri 30 50 60 55 40 30 50 60 55 40 800 Line 7. Fri 30 50 60 55 45 50 45 800 Line 7 Fri 60 175 145			NB/EB		NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB		NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	
Sun Weekday 1,755 2,215 1,715 2,005 1,225 2,005 1,420 3,270	Hwy 11																		
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$		Sat	2,520	1,585	2,475	1,640	500	275	250	400	3,020	1,860	2,725	2,040	0.92	0.57	0.83	0.62	
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		Sun	1,755	2,215	1,715						1,755	2,215	1,715						
Line 3-7 Sat 40 65 70 55 800 Sun 35 55 60 55 40 65 70 55 800 Weekday 35 50 60 40 65 70 55 800 SR 20 Fri 30 50 60 45 60 70 55 40 Line 7.11 Sat 30 60 65 60 55 40 30 50 60 45 800 Sun 30 50 60 55 40 30 50 60 45 60 800 Weekday 25 45 50 45 50 45 60 175 120 85 800 Line 7 Fri 60 175 120 85 60 175 120 85 600 Sun 65 190 130 90 55 165 110 80 600 600 600 600 600 600 600 60		,	1,520								1,520								
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $			35	55	55						35	55	55						
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	Line 3-7	Sat	40	65	70	55					40	65	70	55					800
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $		Sun		55		50						55		50					800
Line 7-11 Sat 30 60 65 60 55 800 Sun 30 50 60 55 30 50 60 55 800 Weekday 25 45 50 45 50 45 800 800 Line 7 Fri 60 175 120 85 60 175 120 85 800 Sun 65 190 130 90 65 100 105 600 175 145 105 600 Sun 65 190 130 90 65 100 130 90 65 100 80 Line 8 Fri 10 5 10 15 500 250 250 510 110 80 600<	_	Weekday	35	50	55	40					35	50	55	40					800
Sun 30 50 60 55 30 50 60 55 800	SR 20	Fri	30	50	60	45					30	50	60	45					800
Weekday 25 45 50 45 50 45 60 175 120 85 600 600 175 120 85 600	Line7-11	Sat	30	60	65						30	60	65						800
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $		Sun	30	50	60	55					30	50	60	55					800
Sat 70 215 145 105 600 Sun 65 190 130 90 65 190 130 90 665 190 130 90 600	_	Weekday	25	45	50	45					25	45	50						
Sun 65 190 130 90 130 90 55 165 110 80 600	Line 7	Fri	60	175	120	85					60	175	120						600
Weekday 55 165 110 80 Line 8 Fri 10 5 10 15 500 250 500 510 15 10 15 600		Sat	70	215	145	105					70	215	145	105					600
Line 8 Fri 10 5 10 15 500 250 500 510 16 15 600 Sat 10 10 10 15 500 250 250 500 510 260 260 515 0.85 0.43 0.43 0.86 600 Sun 10 5 15 10 15 10 5 10 15 600 Weekday 10 5 5 15 10 5 10 15 600 Line 9 Fri 30 140 90 60 30 140 90 60 600 Sat 40 170 110 70 200 200 240 170 310 70 0.40 0.52 600 Sun 30 155 100 65 30 155 100 65 600		Sun	65	190	130	90					65	190	130	90					600
Sat 10 10 10 15 500 250 250 500 260 515 0.85 0.43 0.43 0.86 600 Sun 10 5 10 15 10 15 10 5 10 10 5 10 15 600 Weekday 10 5 5 15 10 5 5 16 600 Line 9 Fri 30 140 90 60 30 140 90 600 <		Weekday	55	165	110	80					55	165	110	80					600
Sun Weekday 10 5 10 15 10 5 10 15 600	Line 8	Fri	10	5	10	15					10	5	10	15					600
Weekday 10 5 5 15 10 5 5 15 600		Sat	10	10	10	15	500	250	250	500	510	260	260	515	0.85	0.43	0.43	0.86	600
Line 9 Fri 30 140 90 60 30 140 90 60 600 Sat 40 170 110 70 200 200 240 170 310 70 0.40 0.52 600 Sun 30 155 100 65 30 155 100 65 600		Sun	10	5	10	15					10	5	10	15					600
Sat4017011070200200240170310700.400.52600Sun30155100653015510065600		Weekday	10	5	5	15					10	5	5	15					600
Sun 30 155 100 65 30 155 100 65 600	Line 9	Fri	30	140	90	60					30	140	90	60					600
		Sat	40	170	110	70	200		200		240	170	310	70	0.40		0.52		600
Weekday 30 135 85 55 30 135 85 55 600		Sun	30	155	100	65					30	155	100	65					600
<u>vvcckudy</u> 30 133 03 33 00 00 00 00 00 00 00 00 00 00 0		Weekday	30	135	85	55					30	135	85	55					600

		Back	kground Tr	affic Volu	mes		Site Traffic	c Volumes			Т	otal Traffi	c Volumes			Volume/0	Capacity		Total
	2020	A	M	PI	M	1	١M	Р	M		AN	1	PI	N	A	N	PI	N	
		NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/	/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	Capacity
Hwy 11	Fri	2,685	1,335	2,980	1,610					2	2,685	1,335	2,980	1,610					3,270
	Sat	2,660	1,675	2,615	1,730	500) 275	250	400	3	8,160	1,950	2,865	2,130	0.97	0.60	0.88	0.65	3,270
	Sun	1,855	2,340	1,815	2,745					1	,855	2,340	1,815	2,745					3,270
	Weekday	1,695	1,295	2,240	1,580					1	,695	1,295	2,240	1,580					3,270
SR 20	Fri	50	70	70	60						50	70	70	60					800
Line 3-7	Sat	55	80	85	70						55	80	85	70					800
	Sun	50	70	75	65						50	70	75	65					800
	Weekday	50	65	70	55						50	65	70	55					800
SR 20	Fri	45	65	75	60						45	65	75	60					800
Line7-11	Sat	45	75	80	75						45	75	80	75					800
	Sun	45	65	75	70						45	65	75	70					800
	Weekday	40	60	65	60						40	60	65	60					800
Line 7	Fri	70	195	140	100						70	195	140	100					600
	Sat	80	245	165	120						80	245	165	120					600
	Sun	75	215	150	105						75	215	150	105					600
	Weekday	65	185	125	95						65	185	125	95					600
Line 8	Fri	15	10	15	20						15	10	15	20					600
	Sat	15	15	15	20	500) 250	250	500		515	265	265	520	0.86	0.44	0.44	0.87	600
	Sun	15	10	15	20						15	10	15	20					600
	Weekday	15	10	10	20						15	10	10	20					600
Line 9	Fri	40	160	105	70						40	160	105	70					600
	Sat	50	195	125	85	200)	200			250	195	325	85	0.42		0.54		600
	Sun	40	175	115	75						40	175	115	75					600
	Weekday	40	150	100	65						40	150	100	65					600

	Background Traffic Volumes				mes		Site Traffic	c Volumes		1	otal Traffi	c Volumes	;		Volume/0	Capacity		Tetel
9	2025	A	M	PI	N	A	М	Р	M	A	N	P	M	A	М	P	M	Total
		NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	Capacity
Hwy 11	Fri	2,820	1,410	3,145	1,700					2,820	1,410	3,145	1,700					3,270
	Sat	2,805	1,770	2,760	1,825	500	275	250	400	3,305	2,045	3,010	2,225	1.01	0.63	0.92	0.68	3,270
	Sun	1,955	2,465	1,920	2,890					1,955	2,465	1,920	2,890					3,270
	Weekday	1,695	1,370	2,240	1,580					 1,695	1,370	2,240	1,580					3,270
SR 20	Fri	50	75	75	65					 50	75	75	65					800
Line 3-7	Sat	55	90	95	75					55	90	95	75					800
	Sun	50	75	85	70					50	75	85	70					800
	Weekday	50	65	75	60					 50	65	75	60					800
SR 20	Fri	45	70	80	65					45	70	80	65					800
Line7-11	Sat	45	80	85	80					45	80	85	80					800
	Sun	45	70	80	75					45	70	80	75					800
	Weekday	40	60	70	65					 40	60	70	65					800
Line 7	Fri	75	215	155	110					75	215	155	110					600

Ento			2.0								2.0							000
	Sat	90	270	180	130					90	270	180	130					600
	Sun	85	235	165	115					85	235	165	115					600
	Weekday	70	205	135	105					70	205	135	105					600
Line 8	Fri	15	10	15	20					15	10	15	20					600
	Sat	15	15	15	20	500	250	250	500	515	265	265	520	0.86	0.44	0.44	0.87	600
	Sun	15	10	15	20					15	10	15	20					600
	Weekday	15	10	10	20					15	10	10	20					600
Line 9	Fri	40	175	115	75					40	175	115	75					600
	Sat	50	215	135	90	200		200		250	215	335	90	0.42		0.56		600
	Sun	40	190	125	80					40	190	125	80					600
	Weekday	40	170	110	70					40	170	110	70					600

Contemporary Music & Camping Festival - Peak Hour Volumes (Sat/Sun mid-day)

			kground Ti					c Volumes				c Volumes				Capacity		Total
	2015	A		PI			M	PI		A		PI			١M	PI		Capacity
		NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	
Hwy 11	Fri	2,540	1,260	2,820	1,525					2,540	1,260	2,820	1,525					3,270
	Sat	2,520	1,585	2,475	1,640			200	120	2,520	1,585	2,675	1,760			0.82	0.54	3,270
	Sun	1,755	2,215	1,715	2,605			200	120	1,755	2,215	1,915	2,725			0.59	0.83	3,270
	Weekday	1,520	1,225	2,005	1,420					1,520	1,225	2,005	1,420					3,270
SR 20	Fri	35	55	55	45					35	55	55	45					800
Line 3-7	Sat	40	65	70	55			20		40	65	90	55			0.11		800
	Sun	35	55	60	50			20		35	55	80	50			0.10		800
	Weekday	35	50	55	40					35	50	55	40					800
SR 20	Fri	30	50	60	45					30	50	60	45					800
Line7-11	Sat	30	60	65	60			20	20	30	60	85	80			0.11	0.10	800
	Sun	30	50	60	55			20	20	30	50	80	75			0.10	0.09	800
	Weekday	25	45	50	45					25	45	50	45					800
Line 7	Fri	60	175	120	85					60	175	120	85					600
	Sat	70	215	145	105					70	215	145	105					600
	Sun	65	190	130	90					65	190	130	90					600
	Weekday	55	165	110	80					55	165	110	80					600
Line 8	Fri	10	5	10	15					10	5	10	15					600
	Sat	10	10	10	15			360	40	10	10	370	55			0.62	0.09	600
	Sun	10	5	10	15			360	40	10	5	370	55			0.62	0.09	600
	Weekday	10	5	5	15					10	5	5	15					600
Line 9	Fri	30	140	90	60					30	140	90	60					600
	Sat	40	170	110	70					40	170	110	70					600
	Sun	30	155	100	65					30	155	100	65					600
	Weekday	30	135	85	55					30	135	85	55					600

g	2020	Back Al	kground Tr	affic Volur Pl			Site Traffi	c Volumes Pl		r IA	otal Traffi	c Volumes Pl		1	Volume/	Capacity Pl	\A	Total
	2020	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	Capacity
Hwy 11	Fri	2,685	1,335	2,980	1,610	10,20	08/118	110/20	00,110	2,685	1,335	2,980	1,610	110/20	00,110	110/20	00,110	3,270
	Sat	2,660	1,675	2,615	1,730			200	120	2,660	1,675	2,815	1,850			0.86	0.57	3,270
	Sun	1,855	2,340	1,815	2,745			200	120	1,855	2,340	2,015	2,865			0.62	0.88	3,270
	Weekday	1,695	1,295	2,240	1,580					1,695	1,295	2,240	1,580					3,270
SR 20	Fri	50	70	70	60					50	70	70	60					800
Line 3-7	Sat	55	80	85	70			20		55	80	105	70			0.13		800
	0	50	70	75						50	70	05				0.40		
	Sun	50	70	75	65			20		50	70	95	65			0.12		800
<u> </u>	Weekday	50	65	70	55					50	65	70	55					800
SR 20	Fri	45	65	75	60			00	00	45	65	75	60			0.10	0.10	800
Line7-11	Sat	45	75	80	75			20	20	45	75	100	95			0.13	0.12	800
	Sun	45	65	75	70			20	20	45	65	95	90			0.12	0.11	800
	Weekday	40	60	65	60					40	60	65	60					800
Line 7	Fri	70	195	140	100					70	195	140	100					600
	Sat	80	245	165	120					80	245	165	120					600
	Sun	75	215	150	105					75	215	150	105					600
	Weekday	65	185	125	95					65	185	125	95					600
Line 8	Fri	15	10	15	20					15	10	15	20					600
	Sat	15	15	15	20			360	40	15	15	375	60			0.63	0.10	600
	Sun	15	10	15	20			360	40	15	10	375	60			0.63	0.10	600
	Weekday	15	10	10	20					15	10	10	20					600
Line 9	Fri	40	160	105	70					40	160	105	70					600
	Sat	50	195	125	85					50	195	125	85					600
	Sun	40	175	115	75					40	175	115	75					600
	Weekday	40	150	100	65					40	150	100	65					600

9	025	Back AN	-	affic Volur PN			Site Traffi AM	c Volumes P	M	T AN		c Volumes PN		A	Volume/	Capacity PN	Л	Total
	.023	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	Capacity
Hwy 11	Fri	2,820	1,410	3,145	1,700					2,820	1,410	3,145	1,700					3,270
	Sat	2,805	1,770	2,760	1,825			200	120	2,805	1,770	2,960	1,945			0.91	0.59	3,270
	Sun	1,955	2,465	1,920	2,890			200	120	1,955	2,465	2,120	3,010			0.65	0.92	3,270
	Weekday	1,695	1,370	2,240	1,580					1,695	1,370	2,240	1,580					3,270
SR 20	Fri	50	75	75	65					50	75	75	65					800
Line 3-7	Sat	55	90	95	75			20		55	90	115	75			0.14		800
	Sun	50	75	85	70			20		50	75	105	70			0.13		800
	Weekday	50	65	75	60					50	65	75	60					800
SR 20	Fri	45	70	80	65					45	70	80	65					800
Line7-11	Sat	45	80	85	80			20	20	45	80	105	100			0.13	0.13	800
	Sun	45	70	80	75			20	20	45	70	100	95			0.13	0.12	800
	Weekday	40	60	70	65					40	60	70	65					800
Line 7	Fri	75	215	155	110					75	215	155	110					600
	Sat	90	270	180	130					90	270	180	130					600
	Sun	85	235	165	115					85	235	165	115					600
	Weekday	70	205	135	105					70	205	135	105					600
Line 8	Fri	15	10	15	20					15	10	15	20					600
	Sat	15	15	15	20			360	40	15	15	375	60			0.63	0.10	600
	Sun	15	10	15	20			360	40	15	10	375	60			0.63	0.10	600
	Weekday	15	10	10	20					15	10	10	20					600
Line 9	Fri	40	175	115	75					40	175	115	75					600
	Sat	50	215	135	90					50	215	135	90					600
	Sun	40	190	125	80					40	190	125	80					600
	Weekday	40	170	110	70					40	170	110	70					600

Barrie Automotive Flea Market - Peak Hour Volumes (Sat/Sun mid-day)

	2015	Bacl A	kground Ti	r affic Volu Pl		A	Site Traffic	: Volumes Pl		T A	Total Traffi	c Volumes Pl		A	Volume/0	Capacity Pl	N A	Total
	2015	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	Capacity
Hwy 11	Fri	2,540	1,260	2,820	1,525	NBIED	00,110	NUILD	OBITE	2,540	1,260	2,820	1,525	NDIED	00/110	NUILU	00/110	3,270
	Sat	2,520	1,585	2,475	1,640	300	300	300	300	2,820	1,885	2,775	1,940	0.86	0.58	0.85	0.59	3,270
	Sun	1,755	2,215	1,715	2,605	300	300	300	300	2,055	2,515	2,015	2,905	0.63	0.77	0.62	0.89	3,270
	Weekday	1,520	1,225	2,005	1,420					1,520	1,225	2,005	1,420					3,270
SR 20	Fri	35	55	55	45					35	55	55	45					800
Line 3-7	Sat	40	65	70	55	30	30	30	30	70	95	100	85	0.09	0.12	0.13	0.11	800
	Sun	35	55	60	50	30	30	30	30	65	85	90	80	0.08	0.11	0.11	0.10	800
	Weekday	35	50	55	40					35	50	55	40					800
SR 20	Fri	30	50	60	45					30	50	60	45					800
Line7-11	Sat	30	60	65	60	30	30	30	30	60	90	95	90	0.08	0.11	0.12	0.11	800
	Sun	30	50	60	55	30	30	30	30	60	80	90	85	0.08	0.10	0.11	0.11	800
	Weekday	25	45	50	45					25	45	50	45					800
Line 7	Fri	60	175	120	85					60	175	120	85					600
	Sat	70	215	145	105	225	270	225	270	295	485	370	375	0.49	0.81	0.62	0.63	600
	Sun	65	190	130	90	225	270	225	270	290	460	355	360	0.48	0.77	0.59	0.60	600
	Weekday	55	165	110	80					55	165	110	80					600
Line 8	Fri	10	5	10	15					10	5	10	15					600
	Sat	10	10	10	15	225	270	225	270	235	280	235	285	0.39	0.47	0.39	0.48	600
	Sun	10	5	10	15	225	270	225	270	235	275	235	285	0.39	0.46	0.39	0.48	600
	Weekday	10	5	5	15					10	5	5	15					600
Line 9	Fri	30	140	90	60					30	140	90	60					600
	Sat	40	170	110	70	180		180		220	170	290	70	0.37		0.48		600
	Sun	30	155	100	65	180		180		210	155	280	65	0.35		0.47		600
	Weekday	30	135	85	55					30	135	85	55					600

			kground Ti				Site Traffic				otal Traffi				Volume/0			Total
	2020	A		PI		A		PI		A		PI		A		PI		Capacity
		NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	
Hwy 11	Fri	2,685	1,335	2,980	1,610					2,685	1,335	2,980	1,610					3,270
	Sat	2,660	1,675	2,615	1,730	300	300	300	300	2,960	1,975	2,915	2,030	0.91	0.60	0.89	0.62	3,270
	Sun	1,855	2,340	1,815	2,745	300	300	300	300	2,155	2,640	2,115	3,045	0.66	0.81	0.65	0.93	3,270
	Weekday	1,695	1,295	2,240	1,580					1,695	1,295	2,240	1,580					3,270
SR 20	Fri	50	70	70	60					50	70	70	60					800
Line 3-7	Sat	55	80	85	70	30	30	30	30	85	110	115	100	0.11	0.14	0.14	0.13	800
	Sun	50	70	75	65	30	30	30	30	80	100	105	95	0.10	0.13	0.13	0.12	800
	Weekday	50	65	70	55					50	65	70	55					800
SR 20	Fri	45	65	75	60					45	65	75	60					800
Line7-11	Sat	45	75	80	75	30	30	30	30	75	105	110	105	0.09	0.13	0.14	0.13	800
	Sun	45	65	75	70	30	30	30	30	75	95	105	100	0.09	0.12	0.13	0.13	800
	Weekday	40	60	65	60					40	60	65	60					800
Line 7	Fri	70	195	140	100					70	195	140	100					600
	Sat	80	245	165	120	225	270	225	270	305	515	390	390	0.51	0.86	0.65	0.65	600
	Sun	75	215	150	105	225	270	225	270	300	485	375	375	0.50	0.81	0.63	0.63	600
	Weekday	65	185	125	95					65	185	125	95					600
Line 8	Fri	15	10	15	20					15	10	15	20					600
	Sat	15	15	15	20	225	270	225	270	240	285	240	290	0.40	0.48	0.40	0.48	600
	Sun	15	10	15	20	225	270	225	270	240	280	240	290	0.40	0.47	0.40	0.48	600
	Weekday	15	10	10	20					15	10	10	20					600
Line 9	Fri	40	160	105	70					40	160	105	70					600
	Sat	50	195	125	85	180		180		230	195	305	85	0.38		0.51		600
	Sun	40	175	115	75	180		180		220	175	295	75	0.37		0.49		600
	Weekday	40	150	100	65					40	150	100	65					600

1	2025	Back Al	k ground T i M	affic Volu Pl		A	Site Traffic	: Volum es Pl		۲ IA	T otal Traffi M	c Volumes Pl		A	Volume/0	Capacity Pl	M	Total
		NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	NB/EB	SB/WB	Capacity
Hwy 11	Fri	2,820	1,410	3,145	1,700					2,820	1,410	3,145	1,700					3,270
	Sat	2,805	1,770	2,760	1,825	300	300	300	300	3,105	2,070	3,060	2,125	0.95	0.63	0.94	0.65	3,270
	Sun	1,955	2,465	1,920	2,890	300	300	300	300	2,255	2,765	2,220	3,190	0.69	0.85	0.68	0.98	3,270
	Weekday	1,695	1,370	2,240	1,580					1,695	1,370	2,240	1,580					3,270
SR 20	Fri	50	75	75	65					50	75	75	65					800
Line 3-7	Sat	55	90	95	75	30	30	30	30	85	120	125	105	0.11	0.15	0.16	0.13	800
	Sun	50	75	85	70	30	30	30	30	80	105	115	100	0.10	0.13	0.14	0.13	800
	Weekday	50	65	75	60					50	65	75	60					800
SR 20	Fri	45	70	80	65					45	70	80	65					800
Line7-11	Sat	45	80	85	80	30	30	30	30	75	110	115	110	0.09	0.14	0.14	0.14	800
	Sun	45	70	80	75	30	30	30	30	75	100	110	105	0.09	0.13	0.14	0.13	800
	Weekday	40	60	70	65					40	60	70	65					800
Line 7	Fri	75	215	155	110					75	215	155	110					600
	Sat	90	270	180	130	225	270	225	270	315	540	405	400	0.53	0.90	0.68	0.67	600
	Sun	85	235	165	115	225	270	225	270	310	505	390	385	0.52	0.84	0.65	0.64	600
	Weekday	70	205	135	105					70	205	135	105					600
Line 8	Fri	15	10	15	20					15	10	15	20					600
	Sat	15	15	15	20	225	270	225	270	240	285	240	290	0.40	0.48	0.40		600
	Sun	15	10	15	20	225	270	225	270	240	280	240	290	0.40	0.47	0.40	0.48	600
	Weekday	15	10	10	20					15	10	10	20					600
Line 9	Fri	40	175	115	75					40	175	115	75					600
	Sat	50	215	135	90	180		180		230	215	315	90	0.38		0.53		600
	Sun	40	190	125	80	180		180		220	190	305	80	0.37		0.51		600
	Weekday	40	170	110	70					40	170	110	70					600

APPENDIX F: 2025 TRAFFIC OPERATIONS WITH BURL'S CREEK

	4	*	1	1	1	ŧ		
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	Y		4Î			र्भ	_	
Volume (veh/h)	280	65	25	130	30	50		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	304	71	27	141	33	54		
Pedestrians								
Lane Width (m)								
Walking Speed (m/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			None			None		
Median storage veh)								
Upstream signal (m)								
pX, platoon unblocked								
vC, conflicting volume	217	98			168			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	217	98			168			
tC, single (s)	6.4	6.2			4.1			
tC, 2 stage (s)								
tF (s)	3.5	3.3			2.2			
p0 queue free %	60	93			98			
cM capacity (veh/h)	753	958			1409			
Direction, Lane #	WB 1	NB 1	SB 1					
Volume Total	375	168	87					
Volume Left	304	0	33					
Volume Right	71	141	0					
cSH	785	1700	1409					
Volume to Capacity	0.48	0.10	0.02					
Queue Length 95th (m)	19.9	0.0	0.5					
Control Delay (s)	13.7	0.0	3.0					
Lane LOS	В		A					
Approach Delay (s)	13.7	0.0	3.0					
Approach LOS	В							
Intersection Summary								
Average Delay			8.6					
Intersection Capacity Utiliza	ation		43.1%	IC	U Level o	f Service		
Analysis Period (min)			15			2		
			10					

	4	*	1	۲	1	ŧ	
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Y		4Î			र्स	
Volume (veh/h)	105	50	95	260	80	185	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	114	54	103	283	87	201	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type			None			None	
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	620	245			386		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	620	245			386		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)							
tF (s)	3.5	3.3			2.2		
p0 queue free %	73	93			93		
cM capacity (veh/h)	418	794			1173		
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total	168	386	288				
Volume Left	114	0	87				
Volume Right	54	283	0				
cSH	494	1700	1173				
Volume to Capacity	0.34	0.23	0.07				
Queue Length 95th (m)	11.4	0.0	1.8				
Control Delay (s)	16.0	0.0	3.0				
Lane LOS	С		A				
Approach Delay (s)	16.0	0.0	3.0				
Approach LOS	С						
Intersection Summary							
Average Delay			4.2				
Intersection Capacity Utiliz	ation		54.0%	IC	U Level o	f Service	
Analysis Period (min)			15				
			10				

HCM Unsignalized Intersection Capacity Analysis 3: Line 7 & Ridge Rd

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EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
	\$			\$			\$			\$	
	Stop			Stop			Stop			Stop	
25	15	20	10	35	35	25	145	15	25	25	25
0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
27	16	22	11	38	38	27	158	16	27	27	27
EB 1	WB 1	NB 1	SB 1								
65	87	201	82								
27	11	27	27								
22	38	16	27								
-0.08	-0.20	0.01	-0.10								
4.6	4.4	4.4	4.4								
0.08	0.11	0.24	0.10								
727	753	793	774								
8.0	8.0	8.8	7.9								
8.0	8.0	8.8	7.9								
А	А	А	А								
		8.3									
		А									
on		25.8%	IC	U Level o	of Service			А			
		15									
	25 0.92 27 EB 1 65 27 22 -0.08 4.6 0.08 727 8.0 8.0 8.0 A	♣ Stop 25 15 0.92 0.92 27 16 EB 1 WB 1 65 87 27 11 22 38 -0.08 -0.20 4.6 4.4 0.08 0.11 727 753 8.0 8.0 8.0 8.0 A A	Stop 25 15 20 0.92 0.92 0.92 27 16 22 EB 1 WB 1 NB 1 65 87 201 27 11 27 22 38 16 -0.08 -0.20 0.01 4.6 4.4 4.4 0.08 0.11 0.24 727 753 793 8.0 8.0 8.8 A A A A A A A A A 707 25.8% 8.3	Stop 25 15 20 10 0.92 0.92 0.92 0.92 27 16 22 11 EB 1 WB 1 NB 1 SB 1 65 87 201 82 27 11 27 27 22 38 16 27 -0.08 -0.20 0.01 -0.10 4.6 4.4 4.4 4.4 0.08 0.11 0.24 0.10 727 753 793 774 8.0 8.0 8.8 7.9 8.0 8.0 8.8 7.9 A A A A 0.0 8.0 8.8 7.9 A A A A 8.3 A A A 0.0 25.8% IC 10	\clubsuit \bigstar Stop Stop 25 15 20 10 35 0.92 0.92 0.92 0.92 0.92 27 16 22 11 38 EB 1 WB 1 NB 1 SB 1 65 87 201 82 27 11 27 27 22 38 16 27 -0.08 -0.20 0.01 -0.10 4.6 4.4 4.4 4.4 0.08 0.11 0.24 0.10 727 753 793 774 8.0 8.0 8.8 7.9 8.0 8.0 8.8 7.9 A A A A A A A A 8.3 A A A A A A A	\bullet \bullet \bullet Stop Stop Stop 25 15 20 10 35 35 0.92 0.92 0.92 0.92 0.92 0.92 27 16 22 11 38 38 EB 1 WB 1 NB 1 SB 1 65 87 201 82 27 11 27 27 22 38 16 27 -0.08 -0.20 0.01 -0.10 4.6 4.4 4.4 4.4 0.08 0.11 0.24 0.10 727 753 793 774 8.0 8.0 8.8 7.9 8.0 8.0 8.8 7.9 A A A A A A A A M A A A A A A A A	\clubsuit \bigstar StopStop251520103535250.920.920.920.920.920.920.9227162211383827EB 1WB 1NB 1SB 16587201822711272722381627-0.08-0.200.01-0.104.64.44.44.40.080.110.240.107277537937748.08.08.87.98.08.08.87.9AA <t< td=""><td>\bullet \bullet \bullet \bullet \bullet \bullet Stop Stop Stop Stop Stop Stop 25 15 20 10 35 35 25 145 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 27 16 22 11 38 38 27 158 EB 1 WB 1 NB 1 SB 1</td><td>Stop Stop Stop Stop 25 15 20 10 35 35 25 145 15 0.92 0.92 0.92 0.92 0.92 0.92</td><td>Stop Stop Stop Stop 25 15 20 10 35 35 25 145 15 25 0.92</td><td>Stop Stop Stop Stop Stop Stop 25 15 20 10 35 35 25 145 15 25 25 0.92 <</td></t<>	\bullet \bullet \bullet \bullet \bullet \bullet Stop Stop Stop Stop Stop Stop 25 15 20 10 35 35 25 145 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 27 16 22 11 38 38 27 158 EB 1 WB 1 NB 1 SB 1	Stop Stop Stop Stop 25 15 20 10 35 35 25 145 15 0.92 0.92 0.92 0.92	Stop Stop Stop Stop 25 15 20 10 35 35 25 145 15 25 0.92	Stop Stop Stop Stop Stop Stop 25 15 20 10 35 35 25 145 15 25 25 0.92 <

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	4Î		Y	
Volume (veh/h)	15	35	65	15	15	15
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	38	71	16	16	16
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)		110110				
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	87				149	79
vC1, stage 1 conf vol	07				177	,,
vC2, stage 2 conf vol						
vCu, unblocked vol	87				149	79
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)	4.1				0.4	0.2
tF (s)	2.2				3.5	3.3
p0 queue free %	99				98	98
cM capacity (veh/h)	1509				833	982
					033	902
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	54	87	33			
Volume Left	16	0	16			
Volume Right	0	16	16			
cSH	1509	1700	901			
Volume to Capacity	0.01	0.05	0.04			
Queue Length 95th (m)	0.2	0.0	0.9			
Control Delay (s)	2.3	0.0	9.1			
Lane LOS	А		А			
Approach Delay (s)	2.3	0.0	9.1			
Approach LOS			А			
Intersection Summary						
Average Delay			2.4			
Intersection Capacity Utiliz	ation		19.3%	IC	U Level o	of Service
Analysis Period (min)			15	10	2 201010	
			10			

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	4Î	
Volume (veh/h)	20	135	25	10	40	15
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	22	147	27	11	43	16
Pedestrians			<u> </u>		10	10
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
				NULLE	NULLE	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked	117	F 2	(0			
vC, conflicting volume	117	52	60			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol	447	50	()			
vCu, unblocked vol	117	52	60			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	97	86	98			
cM capacity (veh/h)	864	1016	1544			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	168	38	60			
Volume Left	22	27	0			
Volume Right	147	0	16			
cSH	994	1544	1700			
Volume to Capacity	0.17	0.02	0.04			
Queue Length 95th (m)	4.6	0.4	0.0			
Control Delay (s)	9.4	5.3	0.0			
Lane LOS	А	A				
Approach Delay (s)	9.4	5.3	0.0			
Approach LOS	A	0.0	0.0			
Intersection Summary						
Average Delay			6.7			
Intersection Capacity Utiliz	ation		24.7%	IC	CU Level c	f Service
Analysis Period (min)			15			
			15			

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	4Î	
Volume (veh/h)	15	20	135	55	20	25
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	22	147	60	22	27
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				NONC	NOTIC	
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	389	35	49			
vC1, stage 1 conf vol	307	- 55	47			
vC2, stage 2 conf vol						
vC2, stage 2 control	389	35	49			
	509 6.4	6.2	49			
tC, single (s)	0.4	0.2	4.1			
tC, 2 stage (s)	3.5	3.3	2.2			
tF (s)			2.2 91			
p0 queue free %	97	98				
cM capacity (veh/h)	557	1037	1558			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	38	207	49			
Volume Left	16	147	0			
Volume Right	22	0	27			
cSH	758	1558	1700			
Volume to Capacity	0.05	0.09	0.03			
Queue Length 95th (m)	1.2	2.4	0.0			
Control Delay (s)	10.0	5.6	0.0			
Lane LOS	В	А				
Approach Delay (s)	10.0	5.6	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			5.2			
Intersection Capacity Utiliza	ation		27.0%	IC	CU Level o	f Service
Analysis Period (min)			15			
			10			

HCM Unsignalized Intersection Capacity Analysis 7: Line 9 & Ridge Rd E

Burls Creek Event Grounds 2025 Fur & Feathers (Sunday mid-morning)

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Volume (veh/h)	15	25	20	10	45	25	25	150	15	15	25	15
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	27	22	11	49	27	27	163	16	16	27	16
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	76			49			185	168	38	253	166	62
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	76			49			185	168	38	253	166	62
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			96	77	98	97	96	98
cM capacity (veh/h)	1523			1558			731	712	1034	560	714	1002
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	65	87	207	60								
Volume Left	16	11	27	16								
Volume Right	22	27	16	16								
cSH	1523	1558	732	717								
Volume to Capacity	0.01	0.01	0.28	0.08								
Queue Length 95th (m)	0.2	0.2	8.8	2.1								
Control Delay (s)	1.9	1.0	11.8	10.5								
Lane LOS	А	А	В	В								
Approach Delay (s)	1.9	1.0	11.8	10.5								
Approach LOS			В	В								
Intersection Summary												
Average Delay			7.8									
Intersection Capacity Utiliza	ation		24.1%	IC	CU Level o	of Service			А			
Analysis Period (min)			15									

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Y	WBR	4	NDR	ODL	4	
Volume (veh/h)	15	120	170	15	120	50	
Sign Control	Stop	120	Free	15	120	Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	16	130	185	16	130	54	
Pedestrians	10	130	100	10	130	04	
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)						N	
Median type			None			None	
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	508	193			201		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	508	193			201		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)							
tF (s)	3.5	3.3			2.2		
p0 queue free %	97	85			90		
cM capacity (veh/h)	475	849			1371		
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total	147	201	185				
Volume Left	147	201	130				
Volume Right	130	16	0				
cSH	780	1700	1371				
	0.19	0.12	0.10				
Volume to Capacity							
Queue Length 95th (m)	5.2	0.0	2.4				
Control Delay (s)	10.7	0.0	5.8				
Lane LOS	B	0.0	A				
Approach Delay (s)	10.7	0.0	5.8				
Approach LOS	В						
Intersection Summary							
Average Delay			5.0				
Intersection Capacity Utilization	ation		37.4%	IC	CU Level c	f Service	
Analysis Period (min)			15				

HCM Unsignalized Intersection Capacity Analysis 9: Line 8 S & Line 8 Access

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			- ↔			4			4	
Volume (veh/h)	0	0	0	0	0	0	0	10	0	0	15	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	0	0	0	0	11	0	0	16	0
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	27	27	16	27	27	11	16			11		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	27	27	16	27	27	11	16			11		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	100	100	100			100		
cM capacity (veh/h)	983	866	1063	983	866	1070	1601			1608		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	0	0	11	16								
Volume Left	0	0	0	0								
Volume Right	0	0	0	0								
cSH	1700	1700	1601	1608								
Volume to Capacity	0.00	0.00	0.00	0.00								
Queue Length 95th (m)	0.0	0.0	0.0	0.0								
Control Delay (s)	0.0	0.0	0.0	0.0								
Lane LOS	A	A	0.0	0.0								
Approach Delay (s)	0.0	0.0	0.0	0.0								
Approach LOS	A	A										
Intersection Summary												
Average Delay			0.0									
Intersection Capacity Utiliza	ition		6.7%	IC	U Level	of Service			А			
Analysis Period (min)			15									
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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y			र्स	eî.		
Volume (veh/h)	0	0	0	190	40	0	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	0	0	0	207	43	0	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	250	43	43				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	250	43	43				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	100	100	100				
cM capacity (veh/h)	739	1027	1565				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	0	207	43				
Volume Left	0	207	43				
	0	0	0				
Volume Right cSH	1700	1565	1700				
Volume to Capacity	0.00	0.00	0.03				
Queue Length 95th (m)	0.00	0.00	0.03				
Control Delay (s)	0.0	0.0	0.0				
Lane LOS	0.0 A	0.0	0.0				
Approach Delay (s)	0.0	0.0	0.0				
Approach LOS	0.0 A	0.0	0.0				
Intersection Summary							
Average Delay			0.0				
Intersection Capacity Utiliz	zation		13.3%	10	CU Level o	of Sorvico	
Analysis Period (min)	Lation		13.3%	IC	O Level (Service	
Analysis renou (IIIII)			10				

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Ý	THE R	î,	HDR.		् र्	
Volume (veh/h)	100	50	20	80	50	1 25	
Sign Control	Stop		Free	00		Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	109	54	22	87	54	27	
Pedestrians	109	54	22	07	54	21	
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)			Mare			Nore	
Median type			None			None	
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	201	65			109		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	201	65			109		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)							
tF (s)	3.5	3.3			2.2		
p0 queue free %	86	95			96		
cM capacity (veh/h)	759	999			1482		
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total	163	109	82				
Volume Left	109	0	54				
Volume Right	54	87	0				
cSH	825	1700	1482				
Volume to Capacity	0.20	0.06	0.04				
Queue Length 95th (m)	5.6	0.00	0.9				
Control Delay (s)	10.4	0.0	5.1				
Lane LOS	10.4 B	0.0	J.1 A				
	10.4	0.0	5.1				
Approach Delay (s) Approach LOS		0.0	0.1				
	В						
Intersection Summary							
Average Delay			6.0				
Intersection Capacity Utiliza	ation		26.0%	IC	CU Level of	Service	
Analysis Period (min)			15				

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Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	Y		¢Î			र्स		
Volume (veh/h)	65	130	45	65	90	9 0		
Sign Control	Stop	150	Free	00	70	Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	71	141	49	71	98	98		
Pedestrians	71	141	т <i>1</i>	/ 1	70	70		
Lane Width (m)								
Walking Speed (m/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			None			None		
Median storage veh)			None			NULLE		
Upstream signal (m)								
pX, platoon unblocked	070	0.4			100			
vC, conflicting volume	378	84			120			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol	070	0.4			100			
vCu, unblocked vol	378	84			120			
tC, single (s)	6.4	6.2			4.1			
tC, 2 stage (s)								
tF (s)	3.5	3.3			2.2			
p0 queue free %	88	86			93			
cM capacity (veh/h)	582	975			1468			
Direction, Lane #	WB 1	NB 1	SB 1					
Volume Total	212	120	196					
Volume Left	71	0	98					
Volume Right	141	71	0					
cSH	796	1700	1468					
Volume to Capacity	0.27	0.07	0.07					
Queue Length 95th (m)	8.2	0.0	1.6					
Control Delay (s)	11.2	0.0	4.1					
Lane LOS	В		A					
Approach Delay (s)	11.2	0.0	4.1					
Approach LOS	В							
Intersection Summary								
Average Delay			6.0					
Intersection Capacity Utiliza	ation		34.6%	IC	U Level of	Service	ý	
Analysis Period (min)	-		15					
			10					

HCM Unsignalized Intersection Capacity Analysis 3: Line 7 & Ridge Rd

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	10	45	25	15	40	15	15	60	15	25	105	15
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	49	27	16	43	16	16	65	16	27	114	16
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	87	76	98	158								
Volume Left (vph)	11	16	16	27								
Volume Right (vph)	27	16	16	16								
Hadj (s)	-0.13	-0.05	-0.03	0.01								
Departure Headway (s)	4.5	4.5	4.4	4.4								
Degree Utilization, x	0.11	0.10	0.12	0.19								
Capacity (veh/h)	750	737	771	777								
Control Delay (s)	8.0	8.0	8.0	8.5								
Approach Delay (s)	8.0	8.0	8.0	8.5								
Approach LOS	А	А	А	А								
Intersection Summary												
Delay			8.2									
Level of Service			А									
Intersection Capacity Utilizati	on		23.6%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	4		¥	
Volume (veh/h)	20	65	50	15	20	20
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	22	71	54	16	22	22
Pedestrians			01	10		
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)		NULL	NULL			
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	71				177	62
vC1, stage 1 conf vol	11				1//	UΖ
vC2, stage 2 conf vol						
vCu, unblocked vol	71				177	62
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)	4.1				0.4	0.2
tF (s)	2.2				3.5	3.3
p0 queue free %	2.2 99				97	98
cM capacity (veh/h)	1530				802	98 1002
civi capacity (veri/ii)	1030				002	1002
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	92	71	43			
Volume Left	22	0	22			
Volume Right	0	16	22			
cSH	1530	1700	891			
Volume to Capacity	0.01	0.04	0.05			
Queue Length 95th (m)	0.3	0.0	1.2			
Control Delay (s)	1.8	0.0	9.2			
Lane LOS	А		А			
Approach Delay (s)	1.8	0.0	9.2			
Approach LOS			А			
Intersection Summary						
Average Delay			2.8			
Intersection Capacity Utilization	ation		21.2%	IC	U Level o	of Service
Analysis Period (min)			15			
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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	4Î	
Volume (veh/h)	45	90	50	15	15	15
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	49	98	54	16	16	16
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				110110		
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	149	24	33			
vC1, stage 1 conf vol	177	27	00			
vC2, stage 2 conf vol						
vCu, unblocked vol	149	24	33			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.4	0.2	7.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	94	91	97			
cM capacity (veh/h)	814	1052	1579			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	147	71	33			
Volume Left	49	54	0			
Volume Right	98	0	16			
cSH	958	1579	1700			
Volume to Capacity	0.15	0.03	0.02			
Queue Length 95th (m)	4.1	0.8	0.0			
Control Delay (s)	9.4	5.7	0.0			
Lane LOS	А	А				
Approach Delay (s)	9.4	5.7	0.0			
Approach LOS	А					
Intersection Summary						
Average Delay			7.2			
Intersection Capacity Utiliza	ation		24.9%	IC	CU Level d	of Service
Analysis Period (min)			15			
			10			

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y			4	4Î		
Volume (veh/h)	20	45	50	25	70	85	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	22	49	54	27	76	92	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	258	122	168				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	258	122	168				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	97	95	96				
cM capacity (veh/h)	702	929	1409				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	71	82	168				
Volume Left	22	54	0				
Volume Right	49	0	92				
cSH	845	1409	1700				
Volume to Capacity	0.08	0.04	0.10				
Queue Length 95th (m)	2.1	0.9	0.0				
Control Delay (s)	9.6	5.2	0.0				
Lane LOS	A	0.2 A	0.0				
Approach Delay (s)	9.6	5.2	0.0				
Approach LOS	A	0.2	0.0				
Intersection Summary							
Average Delay			3.4				
Intersection Capacity Utilization	tion		26.8%	IC	CU Level d	of Service	А
Analysis Period (min)			15				

Movement EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT Lane Configurations		•	•	+	∢	\mathbf{r}	-	٦	
Volume (veh/h) 15 45 25 20 40 15 15 60 15 15 85 Sign Control Free Free Stop Stop % 0%	WBR NBL NBT NBR SBL SBT	NBL	WBR	WBT	WBL	EBR	EBT	EBL	Movement
Volume (veh/h) 15 45 25 20 40 15 15 60 15 15 85 Sign Control Free Free Stop Stop % 0%	÷ ÷			÷			\$		Lane Configurations
Grade 0% 0% 0% 0% Peak Hour Factor 0.92 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.92 0.92 0.92 <		15	15		20	25		15	Volume (veh/h)
Peak Hour Factor 0.92 0.9	Stop Stop			Free			Free		Sign Control
Hourly flow rate (vph) 16 49 27 22 43 16 16 65 16 16 92 Pedestrians Lane Width (m) Walking Speed (m/s) Percent Blockage Right turn flare (veh) Median storage veh) Upstream signal (m) pX, platoon unblocked vC, conflicting volume 60 76 253 198 62 239 204 vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC2, stage 3 conf vol vC2, stage 4 conf vol vC2, stage 5 conf vol vC2, stage 6 conf vol vC2, stage 7 conf vol vC2, stage				0%			0%		
Pedestrians Image: Second	0.92 0.92 0.92 0.92 0.92 0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	Peak Hour Factor
Lane Width (m) Walking Speed (m/s) Percent Blockage Right Lm flare (veh) Median storage veh) Upstream signal (m) pX, platoon unblocked VC2, stage 1 conf vol VC2, stage 2 conf v	16 16 65 16 16 92	16	16	43	22	27	49	16	Hourly flow rate (vph)
Walking Speed (m/s) Percent Blockage Right turn flare (veh) Median storage veh) Upstream signal (m) pX, platoon unblocked vC2, conflicting volume 60 76 253 198 62 239 204 vC1, stage 1 conf vol									Pedestrians
Percent Biockage Right turn flare (veh) Median storage veh) Upstream signal (m) pX, platoon unblocked VC, conflicting volume 60 76 253 198 62 239 204 vC1, stage 1 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC2, stage 3 TF (s) 2.2 2.2 3.5 4.0 3.3 3.5 4.0 p0 queue free % 99 99 97 90 97 90 98 97 86 cM capacity (veh/h) 1544 1523 606 680 1002 640 675 Direction, Lane # EB 1 WB 1 NB 1 SB 1 Volume Total 92 82 98 125 Volume Left 16 22 16 16 Volume Right 27 16 16 16 C5H 1544 1523 703 701 Volume to Capacity 0.01 0.01 0.14 0.18 Queue Length 95th (m) 0.2 0.3 3.7 4.9 Control Delay (s) 1.4 2.1 10.9 11.2 Approach LOS A A B B B Intersection Summary Average Delay 7.0									Lane Width (m)
Percent Biockage Right turn flare (veh) Median storage veh) Upstream signal (m) pX, platoon unblocked VC, conflicting volume 60 76 253 198 62 239 204 vC1, stage 1 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC2, stage 3 TF (s) 2.2 2.2 3.5 4.0 3.3 3.5 4.0 p0 queue free % 99 99 97 90 97 90 98 97 86 cM capacity (veh/h) 1544 1523 606 680 1002 640 675 Direction, Lane # EB 1 WB 1 NB 1 SB 1 Volume Total 92 82 98 125 Volume Left 16 22 16 16 Volume Right 27 16 16 16 C5H 1544 1523 703 701 Volume to Capacity 0.01 0.01 0.14 0.18 Queue Length 95th (m) 0.2 0.3 3.7 4.9 Control Delay (s) 1.4 2.1 10.9 11.2 Approach LOS A A B B B Intersection Summary Average Delay 7.0									Walking Speed (m/s)
Median type None None Median storage veh) Upstream signal (m) PX. pX, platoon unblocked VC, conflicting volume 60 76 253 198 62 239 204 vC1, stage 1 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC1, unblocked vol 60 76 253 198 62 239 204 IC, single (s) 4.1 4.1 7.1 6.5 6.2 7.1 6.5 IF (s) 2.2 2.2 3.5 4.0 3.3 3.5 4.0 p0 queue free % 99 99 97 90 98 97 86 CM capacity (veh/h) 1544 1523 606 680 1002 640 675 Direction, Lane # EB1 WB1 NB1 SB1 VD VD 640 675 Volume Total 92 82 98 125 VD VD 640 675 Volume Right 27 16 16 16 16 16 16 16 CSH 1544 1523 703 701 VD 11 2 10 11 Unue Le									
Median type None None Median storage veh) Upstream signal (m) PX. pX, platoon unblocked VC, conflicting volume 60 76 253 198 62 239 204 vC1, stage 1 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC1, unblocked vol 60 76 253 198 62 239 204 IC, single (s) 4.1 4.1 7.1 6.5 6.2 7.1 6.5 IF (s) 2.2 2.2 3.5 4.0 3.3 3.5 4.0 p0 queue free % 99 99 97 90 98 97 86 CM capacity (veh/h) 1544 1523 606 680 1002 640 675 Direction, Lane # EB1 WB1 NB1 SB1 VD VD 640 675 Volume Total 92 82 98 125 VD VD 640 675 Volume Right 27 16 16 16 16 16 16 16 CSH 1544 1523 703 701 VD 11 2 10 11 Unue Le									Right turn flare (veh)
Median storage veh) Upstream signal (m) pX, platoon unblocked 76 253 198 62 239 204 vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC2, stage 1 7.1 6.5 6.2 7.1 6.5 vC2, stage (s) 4.1 4.1 7.1 6.5 6.2 7.1 6.5 UC 2 stage (s) 5. 4.0 3.3 3.5 4.0 6.0 675 Do queue free % 99 99 97 90 98 97 86 CM Capacity (veh/h) 1544 1523 606 680 1002 640 675 Direction, Lane # EB 1 WB 1 NB 1 SB 1 V2				None			None		
Upstream signal (m) pX, platoon unblocked VC, conflicting volume 60 76 253 198 62 239 204 vC1, stage 1 conf vol vC2, stage 2 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC2, stage (s) 4.1 4.1 7.1 6.5 6.2 7.1 6.5 TC, 2 stage (s) Ff (s) 2.2 2.2 3.5 4.0 3.3 3.5 4.0 p0 queue free % 99 99 97 90 98 97 86 CM capacity (veh/h) 1544 1523 606 680 1002 640 675 Direction, Lane # EB 1 WB 1 NB 1 SB 1 Volume Total 92 82 98 125 Volume Left 16 22 16 16 Volume Left 16 22 16 16 Volume Left 16 22 16 16 Volume to Capacity 0.01 0.01 0.14 0.18 Queue Length 95h (m) 0.2 0.3 3.7 4.9 Control Delay (s) 1.4 2.1 10.9 11.2 Lane LOS A A B B Approach LOS B B Intersection Summary Average Delay 7.0									
pX, platoon unblocked vC, conflicting volume 60 76 253 198 62 239 204 vC1, stage 1 conf vol vc2, stage 2 conf vol vc2, stage 2 conf vol vc2, unblocked vol 60 76 253 198 62 239 204 Vc1, stage 1 conf vol vc2, stage 2 conf vol vc1 7.1 6.5 6.2 7.1 6.5 Vc2, stage (s) 4.1 4.1 7.1 6.5 6.2 7.1 6.5 tF (s) 2.2 2.2 3.5 4.0 3.3 3.5 4.0 p0 queue free % 99 99 97 90 98 97 86 cM capacity (veh/h) 1544 1523 606 680 1002 640 675 Direction, Lane # EB 1 WB 1 NB 1 SB 1 Vc1 Vc1 1523 606 680 1002 640 675 Direction, Lane # EB 1 WB 1 NB 1 SB 1 Vc1 Vc1 Vc1 Vc1 Vc1 Vc1 Vc1 Vc1 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>									
vC, conflicting volume 60 76 253 198 62 239 204 vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol 60 76 253 198 62 239 204 vC, single (s) 4.1 7.1 6.5 6.2 7.1 6.5 C, 2 stage (s) 1 4.1 7.1 6.5 6.2 7.1 6.5 C, 2 stage (s) 1 4.1 7.1 6.5 6.2 7.1 6.5 C, 2 stage (s) 1 4.1 7.1 6.5 6.2 7.1 6.5 DP queue free % 99 99 97 90 98 97 86 CM capacity (veh/h) 1544 1523 606 680 1002 640 675 Direction, Lane # EB 1 WB 1 NB 1 SB 1 Volume Total 92 82 98 125 Volume Left 16 22 16 16 66 680 1002 640 675 Volume Right 27 16 16									
vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol 60 76 253 198 62 239 204 tC, single (s) 4.1 4.1 7.1 6.5 6.2 7.1 6.5 tC, 2 stage (s) tF (s) 2.2 2.2 3.5 4.0 3.3 3.5 4.0 p0 queue free % 99 99 97 90 98 97 86 cM capacity (veh/h) 1544 1523 606 680 1002 640 675 Direction, Lane # EB 1 WB 1 NB 1 SB 1 Volume Total 92 82 98 125 Volume Left 16 22 16 16 Volume Right 27 16 16 16 cSH 1544 1523 703 701 Volume to Capacity 0.01 0.01 0.14 0.18 Queue Length 95th (m) 0.2 0.3 3.7 4.9 Control Delay (s) 1.4 2.1 10.9 11.2 Lane LOS A A B B Approach LOS B B Intersection Summary Average Delay 7.0	253 198 62 239 204	253			76			60	
vC2, stage 2 conf vol vCu, unblocked vol 60 76 253 198 62 239 204 tC, single (s) 4.1 4.1 7.1 6.5 6.2 7.1 6.5 tC, 2 stage (s) 79 99 97 90 98 97 86 p0 queue free % 99 99 97 90 98 97 86 CM capacity (veh/h) 1544 1523 606 680 1002 640 675 Direction, Lane # EB1 WB 1 NB 1 SB 1 675 675 675 675 675 675									
vCu, unblocked vol 60 76 253 198 62 239 204 tC, single (s) 4.1 4.1 7.1 6.5 6.2 7.1 6.5 tC, 2 stage (s) 6.5 6.2 7.1 6.5 tF (s) 2.2 2.2 3.5 4.0 3.3 3.5 4.0 p0 queue free % 99 99 97 90 98 97 86 cM capacity (veh/h) 1544 1523 606 680 1002 640 675 Direction, Lane # EB 1 WB 1 NB 1 SB 1 . <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>									
tC, 2 stage (s) tF (s) 2.2 2.2 3.5 4.0 3.3 3.5 4.0 p0 queue free % 99 99 97 90 98 97 86 cM capacity (veh/h) 1544 1523 606 680 1002 640 675 Direction, Lane # EB 1 WB 1 NB 1 SB 1 Volume Total 92 82 98 125 Volume Total 92 82 98 125 Volume Left 16 22 16 16 Volume Right 27 16 16 16 22 203 3.7 701 Volume to Capacity 0.01 0.14 0.18 20 20 3.3 4.9 20 20 3.3 7.4.9 20 20 3.3 4.9 20 20 3.3 7.1 20 20 20 3.3 4.9 20 20 20 3.3 7.1 20 20 20 20 20 20 20 20 20 20 20 20 20 <td>253 198 62 239 204</td> <td>253</td> <td></td> <td></td> <td>76</td> <td></td> <td></td> <td>60</td> <td></td>	253 198 62 239 204	253			76			60	
tC, 2 stage (s) tF (s) 2.2 2.2 3.5 4.0 3.3 3.5 4.0 p0 queue free % 99 99 97 90 98 97 86 cM capacity (veh/h) 1544 1523 606 680 1002 640 675 Direction, Lane # EB 1 WB 1 NB 1 SB 1 Volume Total 92 82 98 125 Volume Left 16 22 16 16 Volume Left 16 16 Volume Right 27 16 16 16 Volume to Capacity 0.01 0.01 0.14 0.18 Volume to Capacity 0.01 0.01 0.14 0.18 Volume Length 95th (m) 0.2 0.3 3.7 4.9 Volume Logs X<	7.1 6.5 6.2 7.1 6.5	7.1			4.1			4.1	tC, single (s)
IF (s) 2.2 3.5 4.0 3.3 3.5 4.0 p0 queue free % 99 99 97 90 98 97 86 cM capacity (veh/h) 1544 1523 606 680 1002 640 675 Direction, Lane # EB 1 WB 1 NB 1 SB 1 SB 1 Volume Total 92 82 98 125 Volume Total 92 82 98 125									
CM capacity (veh/h) 1544 1523 606 680 1002 640 675 Direction, Lane # EB 1 WB 1 NB 1 SB 1 SB 1 Volume Total 92 82 98 125 Volume Left 16 22 16 16 Volume Right 27 16 16 16 Volume to Capacity 0.01 0.01 0.14 0.18 Queue Length 95th (m) 0.2 0.3 3.7 4.9 Control Delay (s) 1.4 2.1 10.9 11.2 Lane LOS A A B B Approach Delay (s) 1.4 2.1 10.9 11.2 Approach LOS B B B Intersection Summary 7.0 7.0	3.5 4.0 3.3 3.5 4.0	3.5			2.2			2.2	
Direction, Lane # EB 1 WB 1 NB 1 SB 1 Volume Total 92 82 98 125 Volume Left 16 22 16 16 Volume Right 27 16 16 16 cSH 1544 1523 703 701 Volume to Capacity 0.01 0.14 0.18 Queue Length 95th (m) 0.2 0.3 3.7 4.9 Control Delay (s) 1.4 2.1 10.9 11.2 Lane LOS A A B B Approach Delay (s) 1.4 2.1 10.9 11.2 Approach LOS B B B Intersection Summary 7.0 7.0	97 90 98 97 86	97			99			99	p0 queue free %
Volume Total 92 82 98 125 Volume Left 16 22 16 16 Volume Right 27 16 16 16 cSH 1544 1523 703 701 Volume to Capacity 0.01 0.01 0.14 0.18 Queue Length 95th (m) 0.2 0.3 3.7 4.9 Control Delay (s) 1.4 2.1 10.9 11.2 Lane LOS A A B B Approach Delay (s) 1.4 2.1 10.9 11.2 Approach LOS B B B B Approach LOS 7.0 7.0 7.0	606 680 1002 640 675 1	606			1523			1544	cM capacity (veh/h)
Volume Left 16 22 16 16 Volume Right 27 16 16 16 cSH 1544 1523 703 701 Volume to Capacity 0.01 0.01 0.14 0.18 Queue Length 95th (m) 0.2 0.3 3.7 4.9 Control Delay (s) 1.4 2.1 10.9 11.2 Lane LOS A A B B Approach Delay (s) 1.4 2.1 10.9 11.2 Approach LOS B B B Intersection Summary 7.0 7.0 7.0					SB 1	NB 1	WB 1	EB 1	Direction, Lane #
Volume Right 27 16 16 16 cSH 1544 1523 703 701 Volume to Capacity 0.01 0.01 0.14 0.18 Queue Length 95th (m) 0.2 0.3 3.7 4.9 Control Delay (s) 1.4 2.1 10.9 11.2 Lane LOS A A B B Approach Delay (s) 1.4 2.1 10.9 11.2 Approach Delay (s) 1.4 2.1 10.9 11.2 Approach Delay (s) 1.4 2.1 10.9 11.2 Approach LOS B B B Intersection Summary 7.0 7.0					125	98	82	92	Volume Total
cSH 1544 1523 703 701 Volume to Capacity 0.01 0.01 0.14 0.18 Queue Length 95th (m) 0.2 0.3 3.7 4.9 Control Delay (s) 1.4 2.1 10.9 11.2 Lane LOS A A B B Approach Delay (s) 1.4 2.1 10.9 11.2 Approach LOS B B B Intersection Summary 7.0 7.0					16	16	22	16	Volume Left
Volume to Capacity 0.01 0.01 0.14 0.18 Queue Length 95th (m) 0.2 0.3 3.7 4.9 Control Delay (s) 1.4 2.1 10.9 11.2 Lane LOS A A B B Approach Delay (s) 1.4 2.1 10.9 11.2 Approach Delay (s) 1.4 2.1 10.9 11.2 Approach Delay (s) 1.4 2.1 10.9 11.2 Approach LOS B B B Intersection Summary 7.0 7.0					16	16	16	27	Volume Right
Queue Length 95th (m) 0.2 0.3 3.7 4.9 Control Delay (s) 1.4 2.1 10.9 11.2 Lane LOS A A B B Approach Delay (s) 1.4 2.1 10.9 11.2 Approach Dolay (s) 1.4 2.1 10.9 11.2 Approach LOS B B B Intersection Summary 7.0 7.0					701	703	1523	1544	cSH
Queue Length 95th (m) 0.2 0.3 3.7 4.9 Control Delay (s) 1.4 2.1 10.9 11.2 Lane LOS A A B B Approach Delay (s) 1.4 2.1 10.9 11.2 Approach Delay (s) 1.4 2.1 10.9 11.2 Approach LOS B B B Intersection Summary 7.0 7.0					0.18	0.14	0.01	0.01	Volume to Capacity
Control Delay (s) 1.4 2.1 10.9 11.2 Lane LOS A A B B Approach Delay (s) 1.4 2.1 10.9 11.2 Approach Delay (s) 1.4 2.1 10.9 11.2 Approach LOS B B B Intersection Summary 7.0 7.0					4.9	3.7	0.3	0.2	
Lane LOSAABBApproach Delay (s)1.42.110.911.2Approach LOSBBIntersection SummaryAverage Delay7.0					11.2	10.9	2.1	1.4	
Approach LOS B B Intersection Summary 7.0					В	В	А	А	Lane LOS
Approach LOS B B Intersection Summary 7.0					11.2	10.9	2.1	1.4	Approach Delay (s)
Average Delay 7.0									
									Intersection Summary
						7.0			Average Delay
Intersection Capacity Utilization 21.0% ICU Level of Service A	Service A		of Service	U Level o	IC	21.0%		ation	Intersection Capacity Utiliza
Analysis Period (min) 15									

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Y		¢î			र्भ	
Volume (veh/h)	0	0	85	0	0	145	
Sign Control	Stop	Ŭ	Free	Ū	Ū	Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	0.72	0.72	92	0.72	0.72	158	
Pedestrians	Ū	Ū	12	0	Ū	100	
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type			None			None	
Median storage veh)			None			None	
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	250	92			92		
vC1, stage 1 conf vol	200	72			12		
vC2, stage 2 conf vol							
vCu, unblocked vol	250	92			92		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)	0.1	0.2					
tF (s)	3.5	3.3			2.2		
p0 queue free %	100	100			100		
cM capacity (veh/h)	739	965			1502		
					1002		
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total	0	92	158				
Volume Left	0	0	0				
Volume Right	0	0	0				
cSH	1700	1700	1502				
Volume to Capacity	0.19	0.05	0.00				
Queue Length 95th (m)	0.0	0.0	0.0				
Control Delay (s)	0.0	0.0	0.0				
Lane LOS	А						
Approach Delay (s)	0.0	0.0	0.0				
Approach LOS	А						
Intersection Summary							
Average Delay			0.0				
Intersection Capacity Utilization	ation		11.0%	IC	CU Level of S	Service	
Analysis Period (min)			15				

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			\$			\$			\$	
Volume (veh/h)	75	0	10	0	0	0	10	20	0	0	15	75
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	82	0	11	0	0	0	11	22	0	0	16	82
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	101	101	57	111	141	22	98			22		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	101	101	57	111	141	22	98			22		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	91	100	99	100	100	100	99			100		
cM capacity (veh/h)	876	784	1009	852	744	1055	1495			1594		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	92	0	33	98								
Volume Left	82	0	11	0								
Volume Right	11	0	0	82								
cSH	890	1700	1495	1594								
Volume to Capacity	0.10	0.00	0.01	0.00								
Queue Length 95th (m)	2.6	0.0	0.2	0.0								
Control Delay (s)	9.5	0.0	2.5	0.0								
Lane LOS	А	А	А									
Approach Delay (s)	9.5	0.0	2.5	0.0								
Approach LOS	А	А										
Intersection Summary												
Average Delay			4.3									
Intersection Capacity Utiliza	ation		19.7%	IC	CU Level	of Service			А			
Analysis Period (min)			15									
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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	4Î	
Volume (veh/h)	0	0	0	75	115	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	82	125	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	207	125	125			
vC1, stage 1 conf vol	201	.20	.20			
vC2, stage 2 conf vol						
vCu, unblocked vol	207	125	125			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.1	3.2				
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	782	926	1462			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	0	82	125			
Volume Left	0	0	0			
Volume Right	0	0	0			
cSH	1700	1462	1700			
Volume to Capacity	0.00	0.00	0.07			
Queue Length 95th (m)	0.0	0.0	0.0			
Control Delay (s)	0.0	0.0	0.0			
Lane LOS	А					
Approach Delay (s)	0.0	0.0	0.0			
Approach LOS	А					
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliz	zation		9.4%	IC	CU Level c	f Service
Analysis Period (min)			15			
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Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	Υ		4			र्स		
Volume (veh/h)	115	65	20	200	175	25		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	125	71	22	217	190	27		
Pedestrians								
Lane Width (m)								
Walking Speed (m/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			None			Vone		
Median storage veh)								
Upstream signal (m)								
pX, platoon unblocked								
vC, conflicting volume	538	130			239			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	538	130			239			
tC, single (s)	6.4	6.2			4.1			
tC, 2 stage (s)								
tF (s)	3.5	3.3			2.2			
p0 queue free %	71	92			86			
cM capacity (veh/h)	432	919			1328			
Direction, Lane #	WB 1	NB 1	SB 1					
Volume Total	196	239	217					
Volume Left	125	0	190					
Volume Right	71	217	0					
cSH	534	1700	1328					
Volume to Capacity	0.37	0.14	0.14					
Queue Length 95th (m)	12.7	0.0	3.8					
Control Delay (s)	15.6	0.0	7.3					
Lane LOS	C	0.0	A					
Approach Delay (s)	15.6	0.0	7.3					
Approach LOS	С							
Intersection Summary								
Average Delay			7.1					
Intersection Capacity Utiliza	ation		44.8%	IC	CU Level of S	Service	د د	
Analysis Period (min)			15			501 1100		
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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Υ		eî 🗧			र्स
Volume (veh/h)	335	20	55	95	90	320
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	364	22	60	103	98	348
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	655	111			163	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	655	111			163	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	9	98			93	
cM capacity (veh/h)	401	942			1416	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	386	163	446			
Volume Left	364	0	98			
Volume Right	22	103	0			
cSH	415	1700	1416			
Volume to Capacity	0.93	0.10	0.07			
Queue Length 95th (m)	78.8	0.0	1.7			
Control Delay (s)	60.4	0.0	2.2			
Lane LOS	F		A			
Approach Delay (s)	60.4	0.0	2.2			
Approach LOS	F					
Intersection Summary						
Average Delay			24.4			
Intersection Capacity Utiliza	ation		60.3%	IC	U Level of	Service
Analysis Period (min)			15			
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	35	40	25	15	35	40	10	60	10	25	100	20
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	38	43	27	16	38	43	11	65	11	27	109	22
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	109	98	87	158								
Volume Left (vph)	38	16	11	27								
Volume Right (vph)	27	43	11	22								
Hadj (s)	-0.05	-0.20	-0.02	-0.01								
Departure Headway (s)	4.5	4.4	4.6	4.5								
Degree Utilization, x	0.14	0.12	0.11	0.20								
Capacity (veh/h)	738	760	744	759								
Control Delay (s)	8.3	8.0	8.1	8.6								
Approach Delay (s)	8.3	8.0	8.1	8.6								
Approach LOS	А	А	А	А								
Intersection Summary												
Delay			8.3									
Level of Service			А									
Intersection Capacity Utiliza	ition		28.8%	IC	CU Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	4		¥	
Volume (veh/h)	10	60	70	10	10	10
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	65	76	11	11	11
Pedestrians		00	10			
Lane Width (m)						
.,						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)		Mene	Maria			
Median type		None	None			
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	87				168	82
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	87				168	82
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				99	99
cM capacity (veh/h)	1509				816	978
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	76	87	22			
Volume Left	11	0	11			
Volume Right	0	11	11			
cSH	1509	1700	890			
Volume to Capacity	0.01	0.05	0.02			
	0.01	0.05	0.02			
Queue Length 95th (m)	1.1		9.1			
Control Delay (s)		0.0				
Lane LOS	A	0.0	A			
Approach Delay (s)	1.1	0.0	9.1			
Approach LOS			А			
Intersection Summary						
Average Delay			1.5			
Intersection Capacity Utilization	ation		20.4%	IC	U Level o	of Service
Analysis Period (min)			15			

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्भ	4Î	
Volume (veh/h)	20	65	50	15	15	15
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	22	71	54	16	16	16
Pedestrians			01	10	10	10
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				None	None	
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	149	24	33			
vC1, stage 1 conf vol	147	24	55			
vC2, stage 2 conf vol						
vCu, unblocked vol	149	24	33			
tC, single (s)	6.4	6.2	33 4.1			
	0.4	0.2	4.1			
tC, 2 stage (s)	3.5	3.3	2.2			
tF (s)	3.5 97	3.3 93				
p0 queue free %			97			
cM capacity (veh/h)	814	1052	1579			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	92	71	33			
Volume Left	22	54	0			
Volume Right	71	0	16			
cSH	984	1579	1700			
Volume to Capacity	0.09	0.03	0.02			
Queue Length 95th (m)	2.4	0.8	0.0			
Control Delay (s)	9.0	5.7	0.0			
Lane LOS	А	А				
Approach Delay (s)	9.0	5.7	0.0			
Approach LOS	А					
Intersection Summary						
Average Delay			6.3			
Intersection Capacity Utiliza	ation		22.0%	10	CU Level o	of Service
Analysis Period (min)			15			
			10			

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y			र्च	4Î		
Volume (veh/h)	20	45	50	20	65	35	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	22	49	54	22	71	38	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	220	90	109				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	220	90	109				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	97	95	96				
cM capacity (veh/h)	740	968	1482				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	71	76	109				
Volume Left	22	54	0				
Volume Right	49	0	38				
cSH	884	1482	1700				
Volume to Capacity	0.08	0.04	0.06				
Queue Length 95th (m)	2.0	0.9	0.0				
Control Delay (s)	9.4	5.5	0.0				
Lane LOS	А	А					
Approach Delay (s)	9.4	5.5	0.0				
Approach LOS	А						
Intersection Summary							
Average Delay			4.2				
Intersection Capacity Utiliza	ation		21.0%	IC	CU Level o	of Service	А
Analysis Period (min)			15				

Burls Creek Event Grounds 2025 Soccer Inbound (Wednesday evening)

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			.			- 4 >			- 4 >	
Volume (veh/h)	10	40	20	15	50	10	10	50	10	10	80	10
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	43	22	16	54	11	11	54	11	11	87	11
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	65			65			223	174	54	207	179	60
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	65			65			223	174	54	207	179	60
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			98	92	99	98	88	99
cM capacity (veh/h)	1537			1537			648	707	1013	690	702	1006
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	76	82	76	109								
Volume Left	11	16	11	11								
Volume Right	22	11	11	11								
cSH	1537	1537	729	722								
Volume to Capacity	0.01	0.01	0.10	0.15								
Queue Length 95th (m)	0.2	0.2	2.6	4.0								
Control Delay (s)	1.1	1.5	10.5	10.9								
Lane LOS	А	А	В	В								
Approach Delay (s)	1.1	1.5	10.5	10.9								
Approach LOS			В	В								
Intersection Summary												
Average Delay			6.4									
Intersection Capacity Utiliza	ation		19.4%	IC	CU Level o	of Service			А			
Analysis Period (min)			15									

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		¢			र्भ
Volume (veh/h)	5	45	85	50	450	135
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	5	49	92	54	489	147
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1245	120			147	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1245	120			147	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	96	95			66	
cM capacity (veh/h)	127	932			1435	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	54	147	636			
Volume Left	5	0	489			
Volume Right	49	54	0			
cSH	570	1700	1435			
Volume to Capacity	0.10	0.09	0.34			
Queue Length 95th (m)	2.4	0.0	11.6			
Control Delay (s)	12.0	0.0	7.5			
Lane LOS	В		А			
Approach Delay (s)	12.0	0.0	7.5			
Approach LOS	В					
Intersection Summary						
Average Delay			6.5			
Intersection Capacity Utiliz	ation		52.9%	IC	U Level o	f Service
Analysis Period (min)			15			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			÷			÷			\$	
Volume (veh/h)	0	0	0	0	0	0	0	20	0	0	10	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	0	0	0	0	22	0	0	11	0
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	33	33	11	33	33	22	11			22		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	33	33	11	33	33	22	11			22		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	100	100	100			100		
cM capacity (veh/h)	975	860	1070	975	860	1055	1608			1594		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	0	0	22	11								
Volume Left	0	0	0	0								
Volume Right	0	0	0	0								
cSH	1700	1700	1608	1594								
Volume to Capacity	0.10	0.00	0.00	0.00								
Queue Length 95th (m)	0.0	0.0	0.0	0.0								
Control Delay (s)	0.0	0.0	0.0	0.0								
Lane LOS	А	А										
Approach Delay (s)	0.0	0.0	0.0	0.0								
Approach LOS	А	А										
Intersection Summary												
Average Delay			0.0									
Intersection Capacity Utiliza	ition		6.7%	IC	CU Level	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	¥			ب ا	4Î		
Volume (veh/h)	0	0	0	70	110	0	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	0	0	0	76	120	0	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	196	120	120				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	196	120	120				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	100	100	100				
cM capacity (veh/h)	793	932	1468				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	0	76	120				
Volume Left	0	0	0				
Volume Right	0	0	0				
cSH	1700	1468	1700				
Volume to Capacity	0.00	0.00	0.07				
Queue Length 95th (m)	0.0	0.0	0.0				
Control Delay (s)	0.0	0.0	0.0				
Lane LOS	А						
Approach Delay (s)	0.0	0.0	0.0				
Approach LOS	А						
Intersection Summary							
Average Delay			0.0				
Intersection Capacity Utilizat	tion		9.1%	IC	CU Level o	f Service	А
Analysis Period (min)			15				

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		4			ન
Volume (veh/h)	250	200	20	65	40	25
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	272	217	22	71	43	27
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	171	57			92	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	171	57			92	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	66	78			97	
cM capacity (veh/h)	795	1009			1502	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	489	92	71			
Volume Left	272	0	43			
Volume Right	217	71				
cSH	878	1700	1502			
Volume to Capacity	0.56	0.05	0.03			
Queue Length 95th (m)	26.7	0.00	0.7			
Control Delay (s)	14.1	0.0	4.7			
Lane LOS	В	0.0	A			
Approach Delay (s)	14.1	0.0	4.7			
Approach LOS	B	0.0	1.7			
Intersection Summary						
Average Delay			11.1			
Intersection Capacity Utiliza	ation		43.0%	10	U Level of	Sorvico
Analysis Period (min)	allon			IC.		Service
Analysis renou (IIIIII)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		4Î			با
Volume (veh/h)	85	20	190	365	90	185
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	92	22	207	397	98	201
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	802	405			603	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	802	405			603	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	71	97			90	
cM capacity (veh/h)	318	646			974	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	114	603	299			
Volume Left	92	003	299 98			
Volume Right	92	397	98			
cSH	352	397 1700	974			
Volume to Capacity	0.32	0.35	0.10			
Queue Length 95th (m)	10.52	0.35	2.5			
Control Delay (s)	20.1	0.0	2.5			
Lane LOS	20.1 C	0.0	3.7 A			
Approach Delay (s)	20.1	0.0	3.7			
Approach LOS	20.1 C	0.0	3.7			
· · ·	U					
Intersection Summary						
Average Delay			3.3			_
Intersection Capacity Utiliz	zation		63.1%	IC	CU Level of	Service
Analysis Period (min)			15			

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			÷			\$			\$	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	15	40	25	15	35	20	10	60	10	45	100	40
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	43	27	16	38	22	11	65	11	49	109	43
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	87	76	87	201								
Volume Left (vph)	16	16	11	49								
Volume Right (vph)	27	22	11	43								
Hadj (s)	-0.12	-0.09	-0.02	-0.05								
Departure Headway (s)	4.5	4.6	4.5	4.3								
Degree Utilization, x	0.11	0.10	0.11	0.24								
Capacity (veh/h)	733	727	758	790								
Control Delay (s)	8.1	8.1	8.0	8.7								
Approach Delay (s)	8.1	8.1	8.0	8.7								
Approach LOS	А	А	А	А								
Intersection Summary												
Delay			8.4									
Level of Service			А									
Intersection Capacity Utilizat	tion		29.2%	IC	CU Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्भ	4		Y	
Volume (veh/h)	10	80	50	10	10	10
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	87	54	11	11	11
Pedestrians		0,	0.			
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)		None	None			
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	65				168	60
vC1, stage 1 conf vol					100	00
vC2, stage 2 conf vol						
vCu, unblocked vol	65				168	60
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)					0.1	0.2
tF (s)	2.2				3.5	3.3
p0 queue free %	99				99	99
cM capacity (veh/h)	1537				816	1006
					010	1000
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	98	65	22			
Volume Left	11	0	11			
Volume Right	0	11	11			
cSH	1537	1700	901			
Volume to Capacity	0.01	0.04	0.02			
Queue Length 95th (m)	0.2	0.0	0.6			
Control Delay (s)	0.9	0.0	9.1			
Lane LOS	А		А			
Approach Delay (s)	0.9	0.0	9.1			
Approach LOS			А			
Intersection Summary						
Average Delay			1.5			
Intersection Capacity Utiliza	ation		21.4%	IC	U Level o	of Service
Analysis Period (min)			15			
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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	¢Î,	
Volume (veh/h)	20	65	50	15	15	15
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	22	71	54	16	16	16
Pedestrians			0.			
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				NULLE	NULLE	
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	149	24	33			
	149	24	33			
vC1, stage 1 conf vol vC2, stage 2 conf vol						
vCu, unblocked vol	140	24	33			
	149	6.2	33 4.1			
tC, single (s)	6.4	0.2	4.1			
tC, 2 stage (s)	0.5	2.2	0.0			
tF (s)	3.5	3.3	2.2			
p0 queue free %	97	93	97			
cM capacity (veh/h)	814	1052	1579			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	92	71	33			
Volume Left	22	54	0			
Volume Right	71	0	16			
cSH	984	1579	1700			
Volume to Capacity	0.09	0.03	0.02			
Queue Length 95th (m)	2.4	0.8	0.0			
Control Delay (s)	9.0	5.7	0.0			
Lane LOS	А	А				
Approach Delay (s)	9.0	5.7	0.0			
Approach LOS	А					
Intersection Summary						
Average Delay			6.3			
Intersection Capacity Utiliza	ation		22.0%	IC	CU Level o	f Service
Analysis Period (min)			15			20.1100
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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	4Î	
Volume (veh/h)	20	45	50	20	65	35
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	22	49	54	22	71	38
Pedestrians		.,	01			00
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
				NULLE	NULLE	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked	220	00	100			
vC, conflicting volume	220	90	109			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol			100			
vCu, unblocked vol	220	90	109			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	97	95	96			
cM capacity (veh/h)	740	968	1482			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	71	76	109			
Volume Left	22	54	0			
Volume Right	49	0	38			
cSH	884	1482	1700			
Volume to Capacity	0.08	0.04	0.06			
Queue Length 95th (m)	2.0	0.9	0.0			
Control Delay (s)	9.4	5.5	0.0			
Lane LOS	А	А				
Approach Delay (s)	9.4	5.5	0.0			
Approach LOS	A	010	0.0			
Intersection Summary						
Average Delay			4.2			
Intersection Capacity Utiliza	ition		21.0%	10	CU Level c	f Service
Analysis Period (min)			15			
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			÷	
Volume (veh/h)	10	60	20	15	30	10	10	50	10	10	80	10
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	65	22	16	33	11	11	54	11	11	87	11
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	43			87			223	174	76	207	179	38
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	43			87			223	174	76	207	179	38
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			98	92	99	98	88	99
cM capacity (veh/h)	1565			1509			648	707	985	690	702	1034
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	98	60	76	109								
Volume Left	11	16	11	11								
Volume Right	22	11	11	11								
cSH	1565	1509	727	724								
Volume to Capacity	0.01	0.01	0.10	0.15								
Queue Length 95th (m)	0.2	0.2	2.7	4.0								
Control Delay (s)	0.9	2.1	10.5	10.9								
Lane LOS	А	А	В	В								
Approach Delay (s)	0.9	2.1	10.5	10.9								
Approach LOS			В	В								
Intersection Summary												
Average Delay			6.4									
Intersection Capacity Utiliza	ation		19.1%	IC	CU Level o	of Service			А			
Analysis Period (min)			15									

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		¢,			र्स
Volume (veh/h)	50	450	85	5	45	135
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	54	489	92	5	49	147
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	340	95			98	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	340	95			98	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	91	49			97	
cM capacity (veh/h)	635	962			1495	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	543	98	196			
Volume Left	54	0	49			
Volume Right	489	5	0			
cSH	914	1700	1495			
Volume to Capacity	0.59	0.06	0.03			
Queue Length 95th (m)	30.7	0.0	0.8			
Control Delay (s)	14.5	0.0	2.1			
Lane LOS	В		А			
Approach Delay (s)	14.5	0.0	2.1			
Approach LOS	В					
Intersection Summary						
Average Delay			9.9			
Intersection Capacity Utiliz	ation		53.5%	IC	U Level of	Service
Analysis Period (min)			15			
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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			÷			÷			÷	
Volume (veh/h)	0	0	0	0	0	0	0	20	0	0	10	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	0	0	0	0	22	0	0	11	0
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	33	33	11	33	33	22	11			22		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	33	33	11	33	33	22	11			22		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	100	100	100			100		
cM capacity (veh/h)	975	860	1070	975	860	1055	1608			1594		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	0	0	22	11								
Volume Left	0	0	0	0								
Volume Right	0	0	0	0								
cSH	1700	1700	1608	1594								
Volume to Capacity	0.10	0.00	0.00	0.00								
Queue Length 95th (m)	0.0	0.0	0.0	0.0								
Control Delay (s)	0.0	0.0	0.0	0.0								
Lane LOS	А	А										
Approach Delay (s)	0.0	0.0	0.0	0.0								
Approach LOS	A	А										
Intersection Summary												
Average Delay			0.0									
Intersection Capacity Utilizati	on		6.7%	IC	CU Level of	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y			र्स	4Î		
Volume (veh/h)	0	0	0	70	110	0	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	0	0	0	76	120	0	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	196	120	120				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	196	120	120				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	100	100	100				
cM capacity (veh/h)	793	932	1468				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	0	76	120				
Volume Left	0	0	0				
Volume Right	0	0	0				
cSH	1700	1468	1700				
Volume to Capacity	0.00	0.00	0.07				
Queue Length 95th (m)	0.00	0.00	0.07				
Control Delay (s)	0.0	0.0	0.0				
Lane LOS	0.0 A	0.0	0.0				
Approach Delay (s)	0.0	0.0	0.0				
Approach LOS	0.0 A	0.0	0.0				
Intersection Summary							
Average Delay			0.0				
Intersection Capacity Utiliza	ation		9.1%		CU Level o	Service	А
Analysis Period (min)			15			0011100	
			15				

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Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	¥		4Î			र्स		
Volume (veh/h)	210	50	30	190	65	50		
Sign Control	Stop	50	Free	170	00	Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	228	54	33	207	71	54		
Pedestrians	220	54	33	207	/ 1	-04		
ane Width (m)								
Valking Speed (m/s)								
Percent Blockage								
Right turn flare (veh)			None			None		
Median type			None			None		
Vedian storage veh)								
Jpstream signal (m)								
X, platoon unblocked	222	10/			220			
C, conflicting volume	332	136			239			
C1, stage 1 conf vol								
C2, stage 2 conf vol	222	10/			220			
/Cu, unblocked vol	332	136			239			
C, single (s)	6.4	6.2			4.1			
C, 2 stage (s)								
F (s)	3.5	3.3			2.2			
0 queue free %	64	94			95			
M capacity (veh/h)	628	913			1328			
irection, Lane #	WB 1	NB 1	SB 1					
olume Total	283	239	125					
olume Left	228	0	71					
olume Right	54	207	0					
SH	668	1700	1328					
olume to Capacity	0.42	0.14	0.05					
Queue Length 95th (m)	16.0	0.0	1.3					
Control Delay (s)	14.3	0.0	4.6					
ane LOS	В		А					
pproach Delay (s)	14.3	0.0	4.6					
Approach LOS	В							
ntersection Summary								
verage Delay			7.1					
ntersection Capacity Utilization	ation		44.2%	IC	CU Level of	Service	А	
Analysis Period (min)			15					

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WBL	WBR	NBT	NBR	SBL	SBT	
	200		185	85		
	200		100	00		
	0.92		0.92	0.92		
01	217	00	201	12	00	
		None			None	
		NONC			NOTE	
283	166			266		
303	100			200		
202	166			266		
0.4	0.2			4.1		
25	2.2					
575	0/9			1290		
WB 1	NB 1	SB 1				
	0.0	6.1				
В		А				
11.9	0.0	6.1				
В						
		6.0				
tion		45.9%	IC	CU Level o	f Service	А
		15				
	272 54 217 795 0.34 11.6 11.9 B 11.9 B	Y 200 50 200 Stop 0% 0.92 0.92 54 217 54 217 383 166 6.4 6.2 3.5 3.3 91 75 575 879 WB 1 NB 1 272 266 54 0 217 201 795 1700 0.34 0.16 11.6 0.0 11.9 0.0 B 11.9 0.0 B	Y Jo 50 200 60 Stop Free 0% 0% 0.92 0.92 0.92 0.92 54 217 65 383 166 6.4 6.2 3.5 3.3 91 75 575 879 VB 1 NB 1 SB 1 272 266 125 54 0 92 217 201 0 795 1700 1298 0.34 0.16 0.07 11.6 0.0 1.7 11.9 0.0 6.1 B A 11.9 0.0 6.1 B A 11.9 0.0 6.1 B A	50 200 60 185 Stop Free $0%$ $0%$ 0.92 0.92 0.92 0.92 54 217 65 201 833 166 6.4 6.2 3.5 3.3 91 75 575 879 879 $WB 1$ $NB 1$ $SB 1$ 272 266 125 54 0 92 217 201 0 795 1700 1298 0.34 0.16 0.07 11.6 0.0 1.7 11.9 0.0 6.1 B A 11.9 6.0 6.0 6.0	50 200 60 185 85 Stop Free 0% 0% 0	None None 383 166 266 383 166 266 383 166 266 383 166 266 383 166 266 383 166 266 383 166 266 383 166 266 383 166 266 383 166 266 383 166 266 383 166 266 34 0.22 91 75 93 575 575 879 1298 WB 1 NB 1 SB 1 272 266 125 54 0 92 217 201 0 795 1700 1298 0.34 0.16 0.07 11.6 0.0 1.7 11.9 0.0 6.1 B A 11.9

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	15	15	25	10	35	30	30	160	15	15	30	15
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	16	27	11	38	33	33	174	16	16	33	16
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	60	82	223	65								
Volume Left (vph)	16	11	33	16								
Volume Right (vph)	27	33	16	16								
Hadj (s)	-0.18	-0.18	0.02	-0.07								
Departure Headway (s)	4.5	4.5	4.3	4.4								
Degree Utilization, x	0.07	0.10	0.27	0.08								
Capacity (veh/h)	741	749	803	770								
Control Delay (s)	7.8	7.9	8.9	7.8								
Approach Delay (s)	7.8	7.9	8.9	7.8								
Approach LOS	А	А	А	А								
Intersection Summary												
Delay			8.4									
Level of Service			А									
Intersection Capacity Utilization	tion		25.1%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्भ	4		¥	
Volume (veh/h)	15	25	60	15	15	15
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	27	65	16	16	16
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	82				133	73
vC1, stage 1 conf vol						. •
vC2, stage 2 conf vol						
vCu, unblocked vol	82				133	73
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				98	98
cM capacity (veh/h)	1516				851	988
			CD 1			, 30
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	43	82	33			
Volume Left	16	0	16			
Volume Right	0	16	16			
cSH	1516	1700	915			
Volume to Capacity	0.01	0.05	0.04			
Queue Length 95th (m)	0.2	0.0	0.8			
Control Delay (s)	2.8	0.0	9.1			
Lane LOS	A	~ ~	A			
Approach Delay (s)	2.8	0.0	9.1			
Approach LOS			А			
Intersection Summary						
Average Delay			2.7			
Intersection Capacity Utilization	ation		18.8%	IC	U Level o	of Service
Analysis Period (min)			15			

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y			र्स	eî		
Volume (veh/h)	50	310	25	10	35	15	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	54	337	27	11	38	16	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)				None	None		
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	111	46	54				
vC1, stage 1 conf vol		70	54				
vC2, stage 2 conf vol							
vCu, unblocked vol	111	46	54				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)	0.4	0.2	7.1				
tF (s)	3.5	3.3	2.2				
p0 queue free %	94	67	98				
cM capacity (veh/h)	870	1023	1551				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	391	38	54				
Volume Left	54	27	0				
Volume Right	337	0	16				
cSH	999	1551	1700				
Volume to Capacity	0.39	0.02	0.03				
Queue Length 95th (m)	14.3	0.4	0.0				
Control Delay (s)	10.9	5.3	0.0				
Lane LOS	В	A					
Approach Delay (s)	10.9	5.3	0.0				
Approach LOS	В						
Intersection Summary							
Average Delay			9.2				
Intersection Capacity Utilizati	on		37.2%	IC	CU Level o	f Service	А
Analysis Period (min)			15				

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			با	4	
Volume (veh/h)	15	25	135	60	25	360
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	27	147	65	27	391
Pedestrians	10	21		00	21	071
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked	500	000	410			
vC, conflicting volume	582	223	418			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol	=					
vCu, unblocked vol	582	223	418			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	96	97	87			
cM capacity (veh/h)	414	817	1141			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	43	212	418			
Volume Left	16	147	0			
Volume Right	27	0	391			
cSH	599	1141	1700			
Volume to Capacity	0.07	0.13	0.25			
Queue Length 95th (m)	1.8	3.4	0.0			
Control Delay (s)	11.5	6.3	0.0			
Lane LOS	В	A	0.0			
Approach Delay (s)	11.5	6.3	0.0			
Approach LOS	В	0.0	0.0			
Intersection Summary						
Average Delay			2.7			
Intersection Capacity Utiliza	ation		47.5%	10	CU Level of	f Servico
Analysis Period (min)	ation		47.5%	IC IC		Scivice
Analysis Peniuu (min)			10			

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Movement	EBL	EBT	▼ EBR	▼ WBL	WBT	WBR	NBL	NBT	r NBR	SBL	▼ SBT	SBR
Lane Configurations		4	LDI	WDL	4	WBR	NDL	4	NDR	ODL	4	
Volume (veh/h)	15	15	20	10	35	25	25	160	15	15	25	15
Sign Control	10	Free	20	10	Free	20	20	Stop	10	15	Stop	10
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	16	22	11	38	27	27	174	16	16	27	16
Pedestrians	10	10	22		50	21	21	17 4	10	10	21	10
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh)		1 tono										
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	65			38			163	147	27	236	144	52
vC1, stage 1 conf vol	00									200		02
vC2, stage 2 conf vol												
vCu, unblocked vol	65			38			163	147	27	236	144	52
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			96	76	98	97	96	98
cM capacity (veh/h)	1537			1572			757	732	1048	570	734	1016
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	54	76	217	60								
Volume Left	16	11	27	16								
Volume Right	22	27	16	16								
cSH	1537	1572	752	732								
Volume to Capacity	0.01	0.01	0.29	0.08								
Queue Length 95th (m)	0.2	0.2	9.1	2.0								
Control Delay (s)	2.3	1.1	11.7	10.4								
Lane LOS	А	А	В	В								
Approach Delay (s)	2.3	1.1	11.7	10.4								
Approach LOS			В	В								
Intersection Summary												
Average Delay			8.3									
Intersection Capacity Utiliza	ation		23.9%	IC	CU Level o	of Service			А			
Analysis Period (min)			15									

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Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	Y		4Î			र्भ		
Volume (veh/h)	0	0	190	0	0	60		
Sign Control	Stop	-	Free	-	-	Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	0.72	0.72	207	0.72	0.72	65		
Pedestrians	Ū	Ŭ	207	Ū	Ū	00		
Lane Width (m)								
Walking Speed (m/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			None			None		
Median storage veh)			None			None		
Upstream signal (m)								
pX, platoon unblocked								
vC, conflicting volume	272	207			207			
vC1, stage 1 conf vol	212	207			207			
vC2, stage 2 conf vol								
vCu, unblocked vol	272	207			207			
tC, single (s)	6.4	6.2			4.1			
tC, 2 stage (s)	0.4	0.2			7.1			
tF (s)	3.5	3.3			2.2			
p0 queue free %	100	100			100			
cM capacity (veh/h)	718	834			1365			
					1303			
Direction, Lane #	WB 1	NB 1	SB 1					
Volume Total	0	207	65					
Volume Left	0	0	0					
Volume Right	0	0	0					
cSH	1700	1700	1365					
Volume to Capacity	0.00	0.12	0.00					
Queue Length 95th (m)	0.0	0.0	0.0					
Control Delay (s)	0.0	0.0	0.0					
Lane LOS	A							
Approach Delay (s)	0.0	0.0	0.0					
Approach LOS	А							
Intersection Summary								
Average Delay			0.0					
Intersection Capacity Utilizat	tion		13.3%	IC	CU Level c	of Service	А	
Analysis Period (min)			15					

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Volume (veh/h)	0	0	0	0	0	250	0	15	0	500	15	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	0	0	272	0	16	0	543	16	0
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1391	1120	16	1120	1120	16	16			16		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1391	1120	16	1120	1120	16	16			16		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	100	74	100			66		
cM capacity (veh/h)	65	136	1063	135	136	1063	1601			1601		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	0	272	16	560								
Volume Left	0	0	0	543								
Volume Right	0	272	0	0								
cSH	1700	1063	1601	1601								
Volume to Capacity	0.00	0.26	0.00	0.34								
Queue Length 95th (m)	0.0	7.8	0.0	11.6								
Control Delay (s)	0.0	9.5	0.0	8.2								
Lane LOS	A	A	0.0	A								
Approach Delay (s)	0.0	9.5	0.0	8.2								
Approach LOS	A	A	0.0	0.2								
Intersection Summary												
Average Delay			8.5									
Intersection Capacity Utiliza	ition		57.3%	IC	CU Level (of Service			В			
Analysis Period (min)			15									
			10									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	4	
Volume (veh/h)	0	0	0	185	50	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	201	54	0
Pedestrians	-	-	-			-
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				None	None	
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	255	54	54			
vC1, stage 1 conf vol	200	54	JT			
vC2, stage 2 conf vol						
vCu, unblocked vol	255	54	54			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.4	0.2	4.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	733	1013	1551			
	100	1013				
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	0	201	54			
Volume Left	0	0	0			
Volume Right	0	0	0			
cSH	1700	1551	1700			
Volume to Capacity	0.00	0.00	0.03			
Queue Length 95th (m)	0.0	0.0	0.0			
Control Delay (s)	0.0	0.0	0.0			
Lane LOS	А					
Approach Delay (s)	0.0	0.0	0.0			
Approach LOS	А					
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliz	ation		13.1%	IC	CU Level o	of Service
Analysis Period (min)			15			
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Movement	WBL	WBR	NBT	NBR	SBL	SBT	ļ	
Lane Configurations	Y		4			र्स		
Volume (veh/h)	115	60	25	115	55	30		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	125	65	27	125	60	33		
Pedestrians								
Lane Width (m)								
Walking Speed (m/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			None			Vone		
Median storage veh)								
Upstream signal (m)								
pX, platoon unblocked								
vC, conflicting volume	242	90			152			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	242	90			152			
tC, single (s)	6.4	6.2			4.1			
tC, 2 stage (s)								
tF (s)	3.5	3.3			2.2			
p0 queue free %	83	93			96			
cM capacity (veh/h)	715	968			1429			
Direction, Lane #	WB 1	NB 1	SB 1					
Volume Total	190	152	92					
Volume Left	125	0	60					
Volume Right	65	125	0					
cSH	786	1700	1429					
Volume to Capacity	0.24	0.09	0.04					
Queue Length 95th (m)	7.2	0.0	1.0					
Control Delay (s)	11.0	0.0	5.1					
Lane LOS	В		А					
Approach Delay (s)	11.0	0.0	5.1					
Approach LOS	В							
Intersection Summary								
Average Delay			5.9					
Intersection Capacity Utiliza	ation		33.1%	IC	CU Level of S	Service		
Analysis Period (min)			15					

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Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	Υ		eî.			با		
Volume (veh/h)	70	95	45	75	100	95		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	76	103	49	82	109	103		
Pedestrians								
Lane Width (m)								
Walking Speed (m/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			None			None		
Median storage veh)								
Upstream signal (m)								
pX, platoon unblocked								
vC, conflicting volume	410	90			130			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	410	90			130			
tC, single (s)	6.4	6.2			4.1			
tC, 2 stage (s)								
tF (s)	3.5	3.3			2.2			
p0 queue free %	86	89			93			
cM capacity (veh/h)	553	968			1455			
Direction, Lane #	WB 1	NB 1	SB 1					
Volume Total	179	130	212					
Volume Left	76	0	109					
Volume Right	103	82	0					
cSH	734	1700	1455					
Volume to Capacity	0.24	0.08	0.07					
Queue Length 95th (m)	7.3	0.0	1.8					
Control Delay (s)	11.5	0.0	4.2					
Lane LOS	В		A					
Approach Delay (s)	11.5	0.0	4.2					
Approach LOS	В							
Intersection Summary								
Average Delay			5.7					
Intersection Capacity Utiliza	ation		33.6%	IC	CU Level of	Service		
Analysis Period (min)			15					

HCM Unsignalized Intersection Capacity Analysis 3: Line 7 & Ridge Rd

Burls Creek Event Grounds 2025 Tough Mudder (Saturday mid-afternoon)

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		÷			\$			\$			\$	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	15	40	30	20	35	20	15	70	15	25	125	20
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	43	33	22	38	22	16	76	16	27	136	22
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	92	82	109	185								
Volume Left (vph)	16	22	16	27								
Volume Right (vph)	33	22	16	22								
Hadj (s)	-0.14	-0.07	-0.03	-0.01								
Departure Headway (s)	4.5	4.6	4.5	4.4								
Degree Utilization, x	0.12	0.10	0.14	0.23								
Capacity (veh/h)	732	719	757	771								
Control Delay (s)	8.1	8.2	8.2	8.7								
Approach Delay (s)	8.1	8.2	8.2	8.7								
Approach LOS	А	А	А	А								
Intersection Summary												
Delay			8.4									
Level of Service			А									
Intersection Capacity Utilizatio	n		25.4%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		र्स	ef 👘		Υ		
Volume (veh/h)	15	70	60	15	15	15	
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	16	76	65	16	16	16	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type		None	None				
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	82				182	73	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	82				182	73	
tC, single (s)	4.1				6.4	6.2	
tC, 2 stage (s)							
tF (s)	2.2				3.5	3.3	
p0 queue free %	99				98	98	
cM capacity (veh/h)	1516				799	988	
Direction, Lane #	EB 1	WB 1	SB 1				
Volume Total	92		33				
	92 16	82	33 16				
Volume Left	0	0 16	16				
Volume Right cSH	1516	1700	883				
Volume to Capacity	0.01	0.05	883 0.04				
1 3	0.01	0.05	0.04				
Queue Length 95th (m) Control Delay (s)	0.2 1.4	0.0	9.2				
Lane LOS	1.4 A	0.0	9.2 A				
Approach Delay (s)	1.4	0.0	9.2				
Approach LOS	1.4	0.0	9.2 A				
Intersection Summary							
			2.1				
Average Delay Intersection Capacity Utiliz	ation		2.1	10		of Service	А
Analysis Period (min)	ิลแบบ		21.2% 15	IC.	O Level (JI SEIVICE	A
Analysis renou (min)			10				

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्भ	f,	
Volume (veh/h)	75	420	60	15	20	20
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	82	457	65	16	22	22
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	179	33	43			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	179	33	43			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	90	56	96			
cM capacity (veh/h)	776	1041	1565			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	538	82	43			
Volume Left	82	65	45 0			
Volume Right	457	0	22			
cSH	990	1565	1700			
Volume to Capacity	0.54	0.04	0.03			
Queue Length 95th (m)	25.6	1.0	0.03			
Control Delay (s)	12.9	6.0	0.0			
Lane LOS	12.9 B	A	0.0			
Approach Delay (s)	12.9	6.0	0.0			
Approach LOS	В	0.0	0.0			
Intersection Summary						
Average Delay			11.2			
	ation		47.5%	10	CU Level o	fSonico
Intersection Capacity Utiliza Analysis Period (min)	auun			IC	O Level 0	Service
Analysis Penou (IIIII)			15			

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y			र्च	et 🗧		
Volume (veh/h)	25	50	60	25	375	495	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	27	54	65	27	408	538	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	834	677	946				
vC1, stage 1 conf vol	001	0	, 10				
vC2, stage 2 conf vol							
vCu, unblocked vol	834	677	946				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	91	88	91				
cM capacity (veh/h)	308	453	726				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	82	92	946				
Volume Left	27	65	0				
Volume Right	54	0	538				
cSH	391	726	1700				
Volume to Capacity	0.21	0.09	0.56				
Queue Length 95th (m)	5.9	2.2	0.0				
Control Delay (s)	16.6	7.7	0.0				
Lane LOS	С	А					
Approach Delay (s)	16.6	7.7	0.0				
Approach LOS	С						
Intersection Summary							
Average Delay			1.8				
Intersection Capacity Utilizati	ion		65.2%	IC	CU Level o	of Service	С
Analysis Period (min)			15				

HCM Unsignalized Intersection Capacity Analysis 7: Line 9 & Ridge Rd E

Burls Creek Event Grounds 2025 Tough Mudder (Saturday mid-afternoon)

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (veh/h)	15	40	25	20	35	15	15	65	15	15	100	15
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	43	27	22	38	16	16	71	16	16	109	16
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	54			71			250	188	57	231	193	46
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	54			71			250	188	57	231	193	46
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			97	90	98	97	84	98
cM capacity (veh/h)	1551			1530			597	690	1009	644	685	1023
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	87	76	103	141								
Volume Left	16	22	16	16								
Volume Right	27	16	16	16								
cSH	1551	1530	708	707								
Volume to Capacity	0.01	0.01	0.15	0.20								
Queue Length 95th (m)	0.2	0.3	3.9	5.6								
Control Delay (s)	1.4	2.2	11.0	11.4								
Lane LOS	А	А	В	В								
Approach Delay (s)	1.4	2.2	11.0	11.4								
Approach LOS			В	В								
Intersection Summary												
Average Delay			7.4									
Intersection Capacity Utilization	ition		21.4%	IC	CU Level o	of Service			А			
Analysis Period (min)			15									

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		4			र्च
Volume (veh/h)	0	0	105	0	0	170
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	114	0	0	185
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None		1	Vone
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	299	114			114	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	299	114			114	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	100			100	
cM capacity (veh/h)	692	938			1475	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	0	114	185			
Volume Left	0	0	0			
Volume Right	0	0	0			
cSH	1700	1700	1475			
Volume to Capacity	0.00	0.07	0.00			
Queue Length 95th (m)	0.0	0.0	0.0			
Control Delay (s)	0.0	0.0	0.0			
Lane LOS	A	0.0	0.0			
Approach Delay (s)	0.0	0.0	0.0			
Approach LOS	A	010	0.0			
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliza	ation		12.3%	IC	CU Level of S	Service
Analysis Period (min)	auon		12.370	IC IC		
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HCM Unsignalized Intersection Capacity Analysis 9: Line 8 S & Line 8 Access

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Volume (veh/h)	0	0	0	0	0	500	0	20	0	250	15	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	0	0	543	0	22	0	272	16	0
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1125	582	16	582	582	22	16			22		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1125	582	16	582	582	22	16			22		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	100	49	100			83		
cM capacity (veh/h)	77	352	1063	369	352	1055	1601			1594		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	0	543	22	288								
Volume Left	0	0	0	272								
Volume Right	0	543	0	0								
cSH	1700	1055	1601	1594								
Volume to Capacity	0.00	0.51	0.00	0.17								
Queue Length 95th (m)	0.0	23.1	0.0	4.7								
Control Delay (s)	0.0	12.0	0.0	7.4								
Lane LOS	А	В		А								
Approach Delay (s)	0.0	12.0	0.0	7.4								
Approach LOS	А	В										
Intersection Summary												
Average Delay			10.1									
Intersection Capacity Utiliza	ation		58.9%	IC	CU Level	of Service			В			
Analysis Period (min)			15									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y			र्स	4Î		
Volume (veh/h)	0	0	0	85	125	0	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	0	0	0	92	136	0	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	228	136	136				
vC1, stage 1 conf vol	220	100	100				
vC2, stage 2 conf vol							
vCu, unblocked vol	228	136	136				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)	0.11	0.2					
tF (s)	3.5	3.3	2.2				
p0 queue free %	100	100	100				
cM capacity (veh/h)	760	913	1448				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total							
	0	92	136				
Volume Left	0	0	0				
Volume Right cSH	0 1700	0	0 1700				
		1448					
Volume to Capacity	0.00 0.0	0.00 0.0	0.08 0.0				
Queue Length 95th (m)		0.0	0.0				
Control Delay (s) Lane LOS	0.0	0.0	0.0				
	A	0.0	0.0				
Approach Delay (s) Approach LOS	0.0 A	0.0	0.0				
Intersection Summary							
			0.0				
Average Delay	tion		0.0	10		Conulas	٨
Intersection Capacity Utilization	UUT		9.9% 15	IC	CU Level o	Service	А
Analysis Period (min)			15				

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		4Î			र्स
Volume (veh/h)	125	65	25	190	75	35
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	136	71	27	207	82	38
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)			None			None
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	332	130			234	
vC1, stage 1 conf vol	JJZ	130			204	
vC2, stage 2 conf vol						
vCu, unblocked vol	332	130			234	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	0.4	0.2			4.1	
tF (s)	3.5	3.3			2.2	
p0 queue free %	78	92			94	
	623	92 919			1334	
cM capacity (veh/h)	023	919			1334	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	207	234	120			
Volume Left	136	0	82			
Volume Right	71	207	0			
cSH	700	1700	1334			
Volume to Capacity	0.30	0.14	0.06			
Queue Length 95th (m)	9.4	0.0	1.5			
Control Delay (s)	12.3	0.0	5.5			
Lane LOS	В		А			
Approach Delay (s)	12.3	0.0	5.5			
Approach LOS	В					
Intersection Summary						
Average Delay			5.7			
Intersection Capacity Utiliz	ation		39.9%	IC	U Level o	f Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Y		4Î			र्स	
Volume (veh/h)	75	180	50	80	110	105	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	82	196	54	87	120	114	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type			None			None	
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	451	98			141		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	451	98			141		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)							
tF (s)	3.5	3.3			2.2		
p0 queue free %	84	80			92		
cM capacity (veh/h)	519	958			1442		
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total	277	141	234				
Volume Left	82	0	120				
Volume Right	196	87	0				
cSH	767	1700	1442				
Volume to Capacity	0.36	0.08	0.08				
Queue Length 95th (m)	12.6	0.00	2.1				
Control Delay (s)	12.0	0.0	4.3				
Lane LOS	12.J B	0.0	4.3 A				
Approach Delay (s)	12.3	0.0	4.3				
Approach LOS	12.J B	0.0	4.5				
Intersection Summary	-						
Average Delay			6.8				
Intersection Capacity Utiliza	tion		0.8 44.4%	10	U Level of	Sonvico	
Analysis Period (min)			44.4%		O Level OI	Service	
			CI				

HCM Unsignalized Intersection Capacity Analysis 3: Line 7 & Ridge Rd

Burls Creek Event Grounds 2025 Contemporary Music Festival (Saturday mid-afternoon)

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		÷			\$			\$			\$	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	15	65	35	20	40	20	15	75	15	25	130	20
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	71	38	22	43	22	16	82	16	27	141	22
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	125	87	114	190								
Volume Left (vph)	16	22	16	27								
Volume Right (vph)	38	22	16	22								
Hadj (s)	-0.12	-0.07	-0.02	-0.01								
Departure Headway (s)	4.6	4.7	4.6	4.5								
Degree Utilization, x	0.16	0.11	0.15	0.24								
Capacity (veh/h)	722	703	733	747								
Control Delay (s)	8.5	8.3	8.4	9.0								
Approach Delay (s)	8.5	8.3	8.4	9.0								
Approach LOS	А	А	А	А								
Intersection Summary												
Delay			8.6									
Level of Service			А									
Intersection Capacity Utilization	n		27.0%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्भ	101 1	1101	Y	
Volume (veh/h)	35	75	65	35	15	15
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	38	82	71	38	16	16
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	109				247	90
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	109				247	90
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	97				98	98
cM capacity (veh/h)	1482				722	968
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	120	109	33			
Volume Left	38	0	16			
Volume Right	0	38	16			
cSH	1482	1700	827			
Volume to Capacity	0.03	0.06	0.04			
Queue Length 95th (m)	0.6	0.0	0.9			
Control Delay (s)	2.5	0.0	9.5			
Lane LOS	А		А			
Approach Delay (s)	2.5	0.0	9.5			
Approach LOS			А			
Intersection Summary						
Average Delay			2.3			
Intersection Capacity Utilization	ation		22.5%	IC	CU Level c	of Service
Analysis Period (min)			15			
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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	eî.	
Volume (veh/h)	25	75	65	15	20	20
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	27	82	71	16	22	22
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	190	33	43			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	190	33	43			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	96	92	95			
cM capacity (veh/h)	763	1041	1565			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	109	87	43			
Volume Left	27	71	0			
Volume Right	82	0	22			
cSH	954	1565	1700			
Volume to Capacity	0.11	0.05	0.03			
Queue Length 95th (m)	2.9	1.1	0.0			
Control Delay (s)	9.3	6.1	0.0			
Lane LOS	А	А				
Approach Delay (s)	9.3	6.1	0.0			
Approach LOS	А					
Intersection Summary						
Average Delay			6.4			
Intersection Capacity Utiliza	ition		23.7%	IC	CU Level c	f Service
Analysis Period (min)			15			

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Υ			र्स	4Î	
Volume (veh/h)	25	55	65	25	80	40
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	27	60	71	27	87	43
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	277	109	130			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	277	109	130			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	96	94	95			
cM capacity (veh/h)	678	945	1455			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	87	98	130			
Volume Left	27	70	0			
Volume Right	60	0	43			
cSH	841	1455	1700			
Volume to Capacity	0.10	0.05	0.08			
Queue Length 95th (m)	2.6	1.2	0.00			
Control Delay (s)	9.8	5.6	0.0			
Lane LOS	7.0 A	5.0 A	0.0			
Approach Delay (s)	9.8	5.6	0.0			
Approach LOS	7.0 A	5.0	0.0			
	/ \					
Intersection Summary			4.4			
Average Delay	ation			10		fCondes
Intersection Capacity Utiliz	.ali011		23.0%	IC	CU Level o	Service
Analysis Period (min)			15			

HCM Unsignalized Intersection Capacity Analysis 7: Line 9 & Ridge Rd E

Burls Creek Event Grounds 2025 Contemporary Music Festival (Saturday mid-afternoon)

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (veh/h)	15	45	25	20	60	15	15	70	15	15	100	15
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	49	27	22	65	16	16	76	16	16	109	16
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	82			76			283	220	62	266	226	73
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	82			76			283	220	62	266	226	73
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			97	88	98	97	83	98
cM capacity (veh/h)	1516			1523			564	661	1002	604	657	988
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	92	103	109	141								
Volume Left	16	22	16	16								
Volume Right	27	16	16	16								
cSH	1516	1523	679	676								
Volume to Capacity	0.01	0.01	0.16	0.21								
Queue Length 95th (m)	0.2	0.3	4.3	6.0								
Control Delay (s)	1.4	1.6	11.3	11.7								
Lane LOS	А	А	В	В								
Approach Delay (s)	1.4	1.6	11.3	11.7								
Approach LOS			В	В								
Intersection Summary												
Average Delay			7.1									
Intersection Capacity Utiliza	ation		22.8%	IC	CU Level o	of Service			А			
Analysis Period (min)			15									

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		4Î			र्स
Volume (veh/h)	0	0	110	0	0	175
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	120	0	0	190
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	310	120			120	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	310	120			120	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	100	100			100	
cM capacity (veh/h)	683	932			1468	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	0	120	190			
Volume Left	0	0	0			
Volume Right	0	0	0			
cSH Valuma ta Canaaitu	1700	1700	1468			
Volume to Capacity	0.00	0.07	0.00			
Queue Length 95th (m)	0.0	0.0	0.0			
Control Delay (s)	0.0	0.0	0.0			
Lane LOS	A	0.0	0.0			
Approach Delay (s)	0.0	0.0	0.0			
Approach LOS	A					
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliz	ation		12.5%	IC	U Level of	of Service
Analysis Period (min)			15			
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HCM Unsignalized Intersection Capacity Analysis 9: Line 8 S & Line 8 Access

Burls Creek Event Grounds 2025 Contemporary Music Festival (Saturday mid-afternoon)

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			÷			÷	
Volume (veh/h)	0	0	0	0	0	0	40	20	0	0	15	360
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	0	0	0	43	22	0	0	16	391
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	321	321	212	321	516	22	408			22		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	321	321	212	321	516	22	408			22		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	100	100	96			100		
cM capacity (veh/h)	614	574	828	614	445	1055	1151			1594		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	0	0	65	408								
Volume Left	0	0	43	0								
Volume Right	0	0	0	391								
cSH	1700	1700	1151	1594								
Volume to Capacity	0.00	0.51	0.04	0.00								
Queue Length 95th (m)	0.0	0.0	0.9	0.0								
Control Delay (s)	0.0	0.0	5.6	0.0								
Lane LOS	A	A	A	0.0								
Approach Delay (s)	0.0	0.0	5.6	0.0								
Approach LOS	A	A	0.0	0.0								
Intersection Summary												
Average Delay			0.8									
Intersection Capacity Utilizat	tion		33.1%	IC	CU Level (of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Υ			र्स	eî.	
Volume (veh/h)	0	0	0	90	135	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	98	147	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	245	147	147			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	245	147	147			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	744	900	1435			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	0	98	147			
Volume Left	0	90 0	0			
Volume Right	0	0	0			
cSH	1700	1435	1700			
Volume to Capacity	0.00	0.00	0.09			
Queue Length 95th (m)	0.00	0.00	0.09			
Control Delay (s)	0.0	0.0	0.0			
Lane LOS	0.0 A	0.0	0.0			
Approach Delay (s)	0.0	0.0	0.0			
Approach LOS	0.0 A	0.0	0.0			
	//					
Intersection Summary			0.0			
Average Delay	ation			10		fCondoc
Intersection Capacity Utiliza	au011		10.4%	IC	CU Level a	Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	Υ		ef 🗧			र्स	
Volume (veh/h)	275	95	25	160	75	35	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	299	103	27	174	82	38	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type			None			None	
Median storage veh)							
Upstream signal (m)							
pX, platoon unblocked							
vC, conflicting volume	315	114			201		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	315	114			201		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)							
tF (s)	3.5	3.3			2.2		
p0 queue free %	53	89			94		
cM capacity (veh/h)	637	938			1371		
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total	402	201	120				
Volume Left	299	0	82				
Volume Right	103	174	02				
cSH	695	1700	1371				
Volume to Capacity	0.58	0.12	0.06				
Queue Length 95th (m)	28.4	0.0	1.4				
Control Delay (s)	17.1	0.0	5.5				
Lane LOS	C	0.0	0.0 A				
Approach Delay (s)	17.1	0.0	5.5				
Approach LOS	C	0.0	5.0				
Intersection Summary							
Average Delay			10.4				
Intersection Capacity Utiliza	ation		48.2%	IC	U Level of	Service	
Analysis Period (min)			40.270	IC.		JUNUC	
			15				

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Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	Υ		eî.			र्स		
Volume (veh/h)	150	95	140	260	110	255		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	163	103	152	283	120	277		
Pedestrians								
Lane Width (m)								
Walking Speed (m/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			None			None		
Median storage veh)								
Upstream signal (m)								
pX, platoon unblocked								
vC, conflicting volume	810	293			435			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	810	293			435			
tC, single (s)	6.4	6.2			4.1			
tC, 2 stage (s)								
tF (s)	3.5	3.3			2.2			
p0 queue free %	48	86			89			
cM capacity (veh/h)	312	746			1125			
Direction, Lane #	WB 1	NB 1	SB 1					
Volume Total	266	435	397					
Volume Left	163	0	120					
Volume Right	103	283	0					
cSH	403	1700	1125					
Volume to Capacity	0.66	0.26	0.11					
Queue Length 95th (m)	35.0	0.0	2.7					
Control Delay (s)	29.7	0.0	3.3					
Lane LOS	D		A					
Approach Delay (s)	29.7	0.0	3.3					
Approach LOS	D							
Intersection Summary								
Average Delay			8.4					
Intersection Capacity Utiliza	ation		67.0%	IC	CU Level of	Service		
Analysis Period (min)			15					
			10					

HCM Unsignalized Intersection Capacity Analysis 3: Line 7 & Ridge Rd

Burls Creek Event Grounds 2025 Automotive Flea Market (Saturday mid-afternoon)

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		÷			4			4			\$	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	45	45	35	20	55	20	15	75	15	40	130	35
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	49	49	38	22	60	22	16	82	16	43	141	38
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	136	103	114	223								
Volume Left (vph)	49	22	16	43								
Volume Right (vph)	38	22	16	38								
Hadj (s)	-0.06	-0.05	-0.02	-0.03								
Departure Headway (s)	4.8	4.8	4.8	4.6								
Degree Utilization, x	0.18	0.14	0.15	0.29								
Capacity (veh/h)	693	682	708	736								
Control Delay (s)	8.8	8.6	8.6	9.5								
Approach Delay (s)	8.8	8.6	8.6	9.5								
Approach LOS	А	А	А	А								
Intersection Summary												
Delay			9.0									
Level of Service			А									
Intersection Capacity Utilization	n		35.2%	IC	U Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		र्स	4		Y		
Volume (veh/h)	15	90	65	135	30	30	
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
eak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
ourly flow rate (vph)	16	98	71	147	33	33	
edestrians							
ne Width (m)							
alking Speed (m/s)							
cent Blockage							
ht turn flare (veh)							
dian type		None	None				
dian storage veh)							
stream signal (m)							
, platoon unblocked							
conflicting volume	217				274	144	
, stage 1 conf vol							
, stage 2 conf vol							
i, unblocked vol	217				274	144	
single (s)	4.1				6.4	6.2	
2 stage (s)							
s)	2.2				3.5	3.3	
, jueue free %	99				95	96	
capacity (veh/h)	1352				707	903	
ction, Lane #	EB 1	WB 1	SB 1				
me Total	114	217	65				
me Left	114	0	33				
ime Right	0	147	33				
	1352	1700	793				
I ume to Capacity	0.01	0.13	0.08				
eue Length 95th (m)	0.3	0.15	2.0				
ntrol Delay (s)	1.2	0.0	9.9				
e LOS	A	0.0	A				
proach Delay (s)	1.2	0.0	9.9				
proach LOS	1.2	0.0	A				
ersection Summary							
rage Delay			2.0				
ersection Capacity Utiliz	ation		27.6%	IC	CU Level c	of Service	А
alysis Period (min)			15				
			10				

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			<u>स</u>	4	
Volume (veh/h)	55	225	155	15	20	20
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	60	245	168	16	22	22
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	386	33	43			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	386	33	43			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	89	77	89			
cM capacity (veh/h)	551	1041	1565			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	304	185	43			
Volume Left	60	168	0			
Volume Right	245	0	22			
cSH	886	1565	1700			
Volume to Capacity	0.34	0.11	0.03			
Queue Length 95th (m)	11.7	2.7	0.0			
Control Delay (s)	11.2	7.0	0.0			
Lane LOS	В	А				
Approach Delay (s)	11.2	7.0	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			8.8			
Intersection Capacity Utiliza	ation		39.6%	IC	CU Level of	Service
Analysis Period (min)			15			
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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Υ			र्स	4	
Volume (veh/h)	25	145	65	25	80	220
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	27	158	71	27	87	239
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	375	207	326			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	375	207	326			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	95	81	94			
cM capacity (veh/h)	590	834	1234			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	185	98	326			
Volume Left	27	71	0			
Volume Right	158	0	239			
cSH	786	1234	1700			
Volume to Capacity	0.24	0.06	0.19			
Queue Length 95th (m)	6.9	1.4	0.0			
Control Delay (s)	11.0	6.0	0.0			
Lane LOS	В	А				
Approach Delay (s)	11.0	6.0	0.0			
Approach LOS	В					
Intersection Summary						
Average Delay			4.3			
Intersection Capacity Utilization	ation		43.0%	IC	CU Level of	f Service
Analysis Period (min)			15			
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HCM Unsignalized Intersection Capacity Analysis 7: Line 9 & Ridge Rd E

Burls Creek Event Grounds 2025 Automotive Flea Market (Saturday mid-afternoon)

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (veh/h)	15	75	25	20	70	15	15	70	15	15	105	105
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	82	27	22	76	16	16	76	16	16	114	114
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	92			109			427	264	95	310	269	84
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	92			109			427	264	95	310	269	84
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			96	88	98	97	82	88
cM capacity (veh/h)	1502			1482			400	625	962	562	621	975
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	125	114	109	245								
Volume Left	125	22	109	16								
	27	16	16	114								
Volume Right cSH	1502	1482	606	741								
	0.01		0.18	0.33								
Volume to Capacity	0.01	0.01 0.3	0.18 4.9	0.33								
Queue Length 95th (m)	0.3	0.3	4.9	12.2								
Control Delay (s) Lane LOS			12.2 B	12.2 B								
	A 1.0	A	в 12.2									
Approach Delay (s) Approach LOS	1.0	1.5	12.2 B	12.2 B								
			U	ט								
Intersection Summary			7.0									
Average Delay	1		7.8			(Carla			٨			
Intersection Capacity Utiliza	llion		29.8%	IC	CU Level o	I Service			А			
Analysis Period (min)			15									

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	l	
Lane Configurations	Υ		4			ન		
Volume (veh/h)	30	270	110	30	225	175		
Sign Control	Stop		Free			Free		
Grade	0%		0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	33	293	120	33	245	190		
Pedestrians								
Lane Width (m)								
Walking Speed (m/s)								
Percent Blockage								
Right turn flare (veh)								
Median type			None			None		
Median storage veh)								
Upstream signal (m)								
pX, platoon unblocked								
vC, conflicting volume	815	136			152			
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
vCu, unblocked vol	815	136			152			
tC, single (s)	6.4	6.2			4.1			
tC, 2 stage (s)								
tF (s)	3.5	3.3			2.2			
p0 queue free %	89	68			83			
cM capacity (veh/h)	288	913			1429			
Direction, Lane #	WB 1	NB 1	SB 1					
Volume Total	326	152	435					
Volume Left	33	0	245					
Volume Right	293	33	0					
cSH	750	1700	1429					
Volume to Capacity	0.43	0.09	0.17					
Queue Length 95th (m)	16.8	0.0	4.7					
Control Delay (s)	13.4	0.0	5.2					
Lane LOS	В	0.0	A					
Approach Delay (s)	13.4	0.0	5.2					
Approach LOS	В							
Intersection Summary								
Average Delay			7.3					
Intersection Capacity Utiliza	ation		57.6%	IC	U Level of	Service		
Analysis Period (min)			15	.0		2 0. 1100		
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HCM Unsignalized Intersection Capacity Analysis 9: Line 8 S & Line 8 Access

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			÷			÷	
Volume (veh/h)	0	0	0	0	0	0	0	20	120	225	15	0
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	0	0	0	0	22	130	245	16	0
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	592	658	16	592	592	87	16			152		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	592	658	16	592	592	87	16			152		
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	100	100	100	100	100	100	100			83		
cM capacity (veh/h)	363	319	1063	363	347	972	1601			1429		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	0	0	152	261								
Volume Left	0	0	0	245								
Volume Right	0	0	130	0								
cSH	1700	1700	1601	1429								
Volume to Capacity	0.00	0.51	0.00	0.17								
Queue Length 95th (m)	0.0	0.0	0.0	4.7								
Control Delay (s)	0.0	0.0	0.0	7.6								
Lane LOS	А	А		А								
Approach Delay (s)	0.0	0.0	0.0	7.6								
Approach LOS	А	А										
Intersection Summary												
Average Delay			4.8									
Intersection Capacity Utiliza	tion		28.4%	IC	CU Level o	of Service			А			
Analysis Period (min)			15									

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्भ	¢Î,	
Volume (veh/h)	0	0	0	90	225	0
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	0	0	98	245	0
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	342	245	245			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	342	245	245			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	100	100	100			
cM capacity (veh/h)	654	794	1322			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	0	98	245			
Volume Left	0	0	0			
Volume Right	0	0	0			
cSH	1700	1322	1700			
Volume to Capacity	0.00	0.00	0.14			
Queue Length 95th (m)	0.0	0.0	0.0			
Control Delay (s)	0.0	0.0	0.0			
Lane LOS	А					
Approach Delay (s)	0.0	0.0	0.0			
Approach LOS	А					
Intersection Summary						
Average Delay			0.0			
Intersection Capacity Utiliz	ation		15.2%	IC	CU Level o	f Service
Analysis Period (min)			15			
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