## C.C.Tatham \& Associates Ltd.

## Consulting Engineers

# BURL'S CREEK EVENT GROUNDS <br> Township of Oro-Medonte 

Traffic Impact Study

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## Introduction

C.C. Tatham \& Associates was retained by Bur's Creek Event Grounds to address the traffic impacts associated with the proposed Event Grounds development, located in the Township of Oro-Medonte. The location of the development site and the immediate area road network is illustrated in Figure 1.

The purpose of this study is to address the requirements of the Township of Oro-Medonte, the County of Simcoe and the Ministry of Transportation of Ontario (MTO) with respect to the potential transportation impacts of the development on the respective road systems (Township, County and Provincial). In particular, the following will be discussed:

- the operations of the road system through the study area prior to the proposed development;
- an estimation of the growth in the traffic volumes not otherwise attributed to the development (ie. from overall growth in the area and/or other developments);
- an estimation of the number of new trips the proposed Event Grounds development is likely to generate (in consideration of various levels of events);
- the operations of the study area road system upon completion of the development; and
- the resulting impacts and need for mitigating measures (if required) to ensure acceptable overall road operations.

Chapter 2 of this report addresses the existing conditions, detailing the road system and corresponding traffic operations. Chapter 3 addresses future conditions, prior to the completion of the proposed development, and will address the expected growth in the traffic levels and the resulting operating conditions. Chapters 4 and 5 address the proposed development, the ensuing vehicle trips that it will generate, and the associated impacts on the road system. Lastly, Chapter 6 summarizes the report and the key findings.

Burl's Creek has proposed a number of events to occur over the 2015 season, ranging from locally based events (eg. minor soccer and farmers market), to regionally based events (festivals and automotive flea market) to provincially (and beyond) based events (Wayhome Arts \& Music Festival and Boots \& Hearts Music Festival). In consideration of the event durations and significant traffic volumes associated with the noted major music festivals, a separate traffic study has been prepared by a special events transportation consultant to address corresponding traffic operations, routings and protocols. The intent of this study is therefore to address the remaining local and regional based events and the associated traffic impacts of such (if any).

## 2 Existing Conditions

This chapter will describe the road network, traffic volumes and operations for the existing conditions.

### 2.1 Study Area Road Network

The road network to be addressed by this study consists of: Highway 11; Simcoe Road 20 (Ridge Road); Line 7; Line 8 and Line 9. Photographs of the road system are provided in Figure 2.

### 2.1.1 Road Sections

Highway 11 is a provincial highway under the jurisdiction of the MTO. While through the immediate study area, the highway is oriented south-west to north-east and vice versa; in consideration of the larger area and general orientation, Highway 11 is referenced as north-south. The highway has a posted speed of $90 \mathrm{~km} / \mathrm{h}$ and provides 2 travel lanes per direction with a median (box beam guiderail) separation. Although a provincial highway, there are numerous commercial access points and municipal road intersections along the highway. Given the divided nature of the highway, the access points and intersections are limited to right-in/right-out movements with the exception of those at Line 3, Line 5, Line 7, Line 9 and Simcoe Road 20. At these locations, full access to Highway 11 North and Highway 11 South is provided through means of an overpass and connections to the highway on either side. The capacity of Highway 11 has been determined following MTO procedures for maximum service flow for a multi-lane highway, considering factors such as base capacity ( 2000 vehicles per hour per lane) and adjustment factors for lane width and/or lateral clearance restrictions, presence of heavy vehicles, development environment and driver population. Based on the MTO methodology, a maximum service flow of 3270 vehicles has been determined per direction, which equates to 1635 vehicles per hour per lane for the 2-lane highway.

Simcoe Road 20 (known locally as Ridge Road) is classified as a secondary arterial in the County of Simcoe Transportation Master Plan, and thus under the jurisdiction of the County. As per the Master Plan, County roads are facilities where traffic movement is the primary consideration while land access is a secondary function. In this respect, County roads are intended to serve higher traffic volumes and all vehicle types. Simcoe Road 20 provides 1 lane per direction, with a posted speed limit of $60 \mathrm{~km} / \mathrm{h}$, reduced to $50 \mathrm{~km} / \mathrm{h}$ through the hamlets of Oro-Station (at Line 7) and Hawkestone (at Line 11). Typically, County roads are assumed to have capacities of 900 to 1100 vehicles per hour per lane. In consideration of the "secondary arterial" designation and the reduced speed limit (County roads are typically posted $80 \mathrm{~km} / \mathrm{h}$ ), a reduced capacity of 800 vehicles per hour per lane has been assumed (which is consistent with assumptions as per the County of Simcoe Transportation Master Plan).

Line 7, Line 8 and Line 9 are all Township of Oro-Medonte roads, providing 1 lane per direction with an assumed capacity of 600 vehicles per hour per lane (in consideration of their role as collector roads). As with Simcoe Road 20, these roads have reduced speed limits of $60 \mathrm{~km} / \mathrm{h}$ through the study area.

### 2.1. 2 Key Intersections

The key intersections are those of the noted road sections, which are detailed below and illustrated in Figure 3.

As previously noted, the intersections of Line 7 and Line 9 with Highway 11 provide full moves access to/from the highway via a right-in/right-out intersection (as the highway is median separated) and an overpass with 2 stop controlled intersections with the Township road. In all cases, single lane approaches are provided (ie. no dedicated turn lanes exist). The intersection of Line 8 with Highway 11 is restricted to right-in/right-out movements only, both of which are yield controlled and single lanes. At each intersection with the highway, there are acceleration and deceleration lanes to accommodate the right-in and right-out movements.

The intersection of Line 7 with Simcoe Road 20 is a 4-way stop controlled intersection, with each leg providing a single approach lane. This intersection is located in Oro Station.

The intersection of Line 8 with Simcoe Road 20 is a 3-legged intersection (Line 8 terminates at the County road) with stop control on Line 8. All approaches are limited to a single lane.

Lastly, the intersection of Line 9 with Simcoe Road 20 is a 4 -legged intersection stop controlled on Line 9 , with single lanes on each approach.

### 2.2 Existing Traffic Volumes

### 2.2.1 Link Volumes

Traffic volumes on the study area road sections were requested of the MTO, Simcoe County and Township; a summary of which is provided in Table 1. The corresponding daily traffic volumes are presented in Table 2 for spring (May), summer (July-August) and fall (October) conditions, whereas additional details are provided in Appendix A. No recent count data was available for Line 8 or Line 9.

## Table 1: Available Traffic Data

| Road <br> Section | Location | Source | Date | Type |
| :--- | :---: | :--- | :---: | :---: |
| Highway 11 | Line 5 to Line 6 <br> $(8.23 k m$ north of <br> Simcoe Road 93) | MTO | July 2014 | 24 hour counts by <br> hour |
| Simcoe | Line 3 to Line 7 |  | May 2014 | 24 hour counts by |
| Road 20 | Line 7 to Line 11 | County | August 2014 <br> October 2014 | Line 11 to Hwy 11 |
| Line 7 | south of overpass | Township | May 2014 | 24 hour |

Table 2: 2014 Daily Traffic Volumes

| Road <br> Section | Location | Spring | Summer | Fall |
| :--- | :---: | :---: | :---: | :---: |
| Highway 11 | Line 5 to Line 6 | - | 40,000 to 58,000 | - |
|  | Line 3 to Line 7 | 1500 to 1700 | 1350 to 1500 | 1350 to 1500 |
| Simcoe | Line 7 to Line 11 | 1000 to 1150 | 1000 to 1150 | 900 to 1000 |
| Road 20 | Line 11 to Hwy 11 | 1950 to 2050 | 1700 to 1850 | 1800 to 1950 |
| Line 7 | south of overpass | 4100 | - | - |

### 2.2.2 Intersection Volumes

Further to the link volumes provided by the road authorities, additional traffic counts were completed at a number of the key intersections to provide an indication of peak hour turning volumes. Traffic counts were completed on Thursday April 2, 2015 from 7AM to 10AM and 4PM to 7PM at the following intersections (details of which are provided in Appendix A):

- Line 7 \& Highway 11 overpass (both intersections);
- Line 8 \& Highway 11 (right-in/right/out);
- Line 9 \& Highway 11 overpass (both intersections); and
- Line 7 \& Simcoe Road 20.

A Thursday count is considered representative of typical weekday operations, whereas April volumes are typically considered representative of average conditions throughout the year (as is October). While counts were not completed at the Line 7 and Line 9 right-in/right-outs with Highway 11, the traffic volumes can be established from the counts at the adjacent overpass intersections in that there are generally no significant traffic generators or attractors between the intersections. The exception is the access to the Tim Hortons/gas station on Line 7 between Highway 11 and the west overpass intersection (this access was included in the traffic count and thus associated volumes established).

### 2.2.3 Seasonal Variations

The traffic data for Simcoe Road 20 was reviewed for seasonal variations, given that the counts were completed in the spring, summer and fall. The corresponding daily volumes were relatively consistent suggesting no significant increases in summer volumes on Simcoe Road 20. For example, for the road section from Line 3 to Line 7, the average daily volumes observed in the spring, summer and fall were $1634,1429,1424$. For the section from Line 7 to Line 11 , the spring, summer and fall volumes were 1088, 1046 and 977 respectively.

On Highway 11, a review of published MTO average and summer daily traffic volumes for the year 2010 (the most current published data available) suggests summer volumes are approximately $12 \%$ greater than average volumes ( 43,200 vs 39,000 ). Given the significance of Highway 11 in serving areas to the north (eg. "cottage country"), such an increase as compared to Simcoe Road 20 is expected (ie. similar increases on the local road system, which primarily serve the local residences, are not expected).

### 2.2.4 Daily Variations

The available data was also reviewed with respect to daily variations (namely weekday vs weekend). Counts for Simcoe Road 20 were typically conducted Tuesday to Thursday with little variation, and thus are considered typical weekday. Counts were not completed over the weekend and thus there is no basis for comparison along Simcoe Road 20.

The Highway 11 counts were conducted for a continuous week, with the daily variations outlined in Table 3. Based on the summer 2014 traffic data observed between Line 5 and Line 6, the following increases were observed:

- Friday peak hour volumes are approximately $5 \%$ greater than the average, with a more pronounced increase during the northbound direction (which corresponds to travel to cottage country);
- Saturday peak hour volumes are approximately $30 \%$ greater than the average, with a greater increase in the AM northbound direction (which corresponds to travel to cottage country); and
- Sunday peak hour volumes are approximately $15 \%$ greater, with greater increases in the AM and PM southbound direction (which corresponds to travel from cottage country).

Table 3: 2014 Daily Traffic Variations on Highway 11

| Date | AM Peak Hour |  | PM Peak Hour |  |
| :--- | :---: | :---: | :---: | :---: |
|  | NB | SB | NB | SB |
| Average <br> Weekday | 1500 vph | 1210 vph | 1985 vph | 1400 vph |
| Friday |  | 1245 vph | 2790 vph | 1505 vph |
|  | $67 \%$ higher | $3 \%$ higher | $41 \%$ higher | $7 \%$ higher |
| Saturday | 2490 vph | 1565 vph | 2450 vph | 1620 vph |
|  | $66 \%$ higher | $29 \%$ higher | $23 \%$ higher | $16 \%$ higher |
| Sunday | 1735 vph | 2190 vph | 1695 vph | 2580 vph |
|  | $16 \%$ higher | $81 \%$ higher | $15 \%$ lower | $84 \%$ higher |

vph - vehicles per hour

### 2.2.5 Peak Hour Volumes

The peak hour volumes reflective of weekday AM and PM peak hour conditions (as per the April 2, 2015 traffic count), are illustrated in Figure 4. Traffic volumes on Highway 11 correspond to the noted average weekday conditions from the MTO 2014 count, increased by 1\% per annum to reflect 2015 conditions (further discussion of the noted growth rate is provided in Section 3.2). As traffic counts were not completed at the intersections of Line 8 and Line 9 with Simcoe Road 20, they have been estimated based on the available data. The volumes on Line 7 and Line 9 just east of Highway 11 are comparable (as per the intersection counts at the east Highway 11 overpass intersections) and thus comparable volumes at Simcoe Road 20 have also been assumed (ie. the volumes at Line 9/Simcoe Road 20 have been estimated based on the Line 7/Simcoe Road 20 intersection volumes). Volumes at the Line $8 /$ Simcoe Road 20 intersection were estimated based on the volumes on Line 8 as determined from the traffic count at the Highway 11 right-in/right-out. In most cases, the traffic volumes are not considered significant and thus if the assumptions are understated, the corresponding margin of error will not be critical.

In consideration of the event schedules planned for Burl's Creek, consideration has also been given to Friday, Saturday and Sunday traffic levels. Given the broad range of days and intervals considered, traffic counts corresponding to each were not completed. Rather, the volumes for the Friday, Saturday and Sunday periods have been estimated in consideration of the daily variations observed on Highway 11 (not considering the increases corresponding to the peak cottage country demands). Namely:

- Friday volumes are estimated as $5 \%$ greater than the weekday volumes (as per the traffic counts);
- Saturday volumes are estimated as 30\% greater; and
- Sunday volumes are estimated as $15 \%$ greater.

Given the local nature of the area road system in serving the local development, such increases are not likely. Furthermore, the commuter peaks, as evident in the weekday data, would not be expected to be as significant over the weekends (travel over the weekends tends to be more distributed throughout the day, and not as pronounced during the early morning and late afternoon). Notwithstanding, the noted factors have been assumed to employ a conservative approach.

Traffic volumes on Highway 11 have been established for each of the noted days and peak periods based on the actual 2014 Highway 11 traffic counts, adjusted to reflect 2015 conditions.

The corresponding 2015 volumes are noted in Figure 5, Figure 6 and Figure 7 for the Friday, Saturday and Sunday peak hour periods.

### 2.3 Existing Traffic Operations

### 2.3.1 Intersection Operations

The assessment of existing conditions provides the baseline from which the future traffic volumes and operations (both with and without the subject development) can be assessed. The capacity, and hence operations, of a road system is effectively dictated by its intersections and thus the operational analysis considers the operations of the key intersections previously noted. The analysis is based on the 2015 Saturday traffic volumes, the existing intersection configuration and controls, and procedures outlined in the 2010 Highway Capacity Manual ${ }^{1}$ (using Synchro v. 8 software). As the Saturday traffic volumes at the key intersections are greatest, their use in the assessment will represent the most conservative operational assessment. While it is acknowledged that travel patterns on Highway 11 vary between the Saturday and Sunday peak hours (on Saturday, the northbound volumes are greatest, whereas on Sunday the southbound volumes are greatest), these volumes have no bearing on the operations of the key intersections.

A summary of the 2015 Saturday peak hour traffic volumes analyses is provided in Table 4. Results of the operational assessment are provided in the form of average delay (measured in seconds), level of service (LOS) and volume to capacity (v/c) for the critical stop controlled movements. Level of service A corresponds to the best operating condition with minimal delays whereas level of service $F$ corresponds to poor operations resulting from high intersection delays. A v/c ratio of less than 1.0 indicates the intersection movement/approach is operating at less than capacity while v/c of 1.0 indicates capacity has been reached. Detailed operations worksheets for the existing traffic conditions are included in Appendix $B$.

As indicated, all of the key study area intersections are currently providing excellent levels of service (level of service $A$ or $B$ ) with minimal delays. In this respect, no improvements to the existing road system are considered necessary to support the existing peak hour traffic volumes. As the traffic operations under the peak Saturday conditions are considered acceptable, it can be inferred that operations under the weekday, Friday and Sunday conditions will also be acceptable recognizing that the intersection volumes will be less for these time periods.

[^0]Table 4: Intersection Operations - 2015 Saturday

| Intersection and Movement |  | Control | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | delay | LOS | v/c | delay | LOS | v/c |
|  <br> Hwy 11 Overpass W | SB |  | stop | 1 | A | 0.01 | 4 | A | 0.02 |
|  <br> Hwy 11 Overpass E | SB | stop | 6 | A | 0.06 | 4 | A | 0.07 |
|  <br> Simcoe Road 20 | EB | 4-way stop | 8 | A | ? | 8 | A | 0.09 |
|  | WB |  | 8 | A | ? | 8 | A | 0.08 |
|  | NB |  | 9 | A | ? | 8 | A | 0.11 |
|  | SB |  | 8 | A | ? | 8 | A | 0.20 |
|  <br> Simcoe Road 20 | SB | stop | 8.9 | A | 0.02 | 9 | A | 0.02 |
|  <br> Hwy 11 Overpass W | SB | stop | 0 | A | 0.03 | 0 | A | 0.02 |
|  <br> Hwy 11 Overpass E | SB | stop | 0 | A | 0.03 | 0 | A | 0.06 |
| Line 9 \& | NB | stop | 11 | B | 0.25 | 11 | B | 0.12 |
| Simcoe Road 20 | SB | stop | 9 | A | 0.06 | 11 | B | 0.17 |

### 2.3.2 Road Section Operations

Further to the operations of the key intersections, the operations of the key road sections has also been investigated, based on the projected volumes and the following lane capacities (as previously presented):

- 1635 vehicles per hour per lane (vphpl) for Highway 11 ( 3270 vehicles per hour given 2 lanes per direction);
- 800 vphpl for Simcoe Road 20; and
- 600 vphpl for Line 7, Line 8 and Line 9.

A summary of the corresponding traffic volumes, road capacities and resulting volume to capacity (v/c) ratios is provided in Table 5. A v/c ratio of 1.0 or greater suggests that the road section is operating at or above capacity; v/c of less than 1.0 indicates that there is excess capacity available.

Table 5: Road Section Operations - 2015

| Road Section \& Period |  | AM Peak Hour Volumes |  | PM Peak Hour Volumes |  | AM Peak Volume/Capacity |  | PM Peak Volume/Capacity |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SBMB |
| Highway 11 | Friday | 2,540 | 1,260 | 2,820 | 1,525 | 0.78 | 0.39 | 0.86 | 0.47 |
|  | Saturday | 2,520 | 1,585 | 2,475 | 1,640 | 0.77 | 0.48 | 0.76 | 0.50 |
|  | Sunday | 1,755 | 2,215 | 1,715 | 2,605 | 0.54 | 0.68 | 0.52 | 0.80 |
|  | Weekday | 1,520 | 1,225 | 2,005 | 1,420 | 0.46 | 0.37 | 0.61 | 0.43 |
| Simcoe Road 20 (Line 3 to Line 7) | Friday | 35 | 55 | 55 | 45 | 0.04 | 0.07 | 0.07 | 0.06 |
|  | Saturday | 40 | 65 | 70 | 55 | 0.05 | 0.08 | 0.09 | 0.07 |
|  | Sunday | 35 | 55 | 60 | 50 | 0.04 | 0.07 | 0.08 | 0.06 |
|  | Weekday | 35 | 50 | 55 | 40 | 0.04 | 0.06 | 0.07 | 0.05 |
| Simcoe Road 20 (Line 7 to Line 11) | Friday | 30 | 50 | 60 | 45 | 0.04 | 0.06 | 0.08 | 0.06 |
|  | Saturday | 30 | 60 | 65 | 60 | 0.04 | 0.08 | 0.08 | 0.08 |
|  | Sunday | 30 | 50 | 60 | 55 | 0.04 | 0.06 | 0.08 | 0.07 |
|  | Weekday | 25 | 45 | 50 | 45 | 0.03 | 0.06 | 0.06 | 0.06 |
| Line 7 | Friday | 60 | 175 | 120 | 85 | 0.10 | 0.29 | 0.20 | 0.14 |
|  | Saturday | 70 | 215 | 145 | 105 | 0.12 | 0.36 | 0.24 | 0.18 |
|  | Sunday | 65 | 190 | 130 | 90 | 0.11 | 0.32 | 0.22 | 0.15 |
|  | Weekday | 55 | 165 | 110 | 80 | 0.09 | 0.28 | 0.18 | 0.13 |
| Line 8 | Friday | 10 | 5 | 10 | 15 | 0.02 | 0.01 | 0.02 | 0.03 |
|  | Saturday | 10 | 10 | 10 | 15 | 0.02 | 0.02 | 0.02 | 0.03 |
|  | Sunday | 10 | 5 | 10 | 15 | 0.02 | 0.01 | 0.02 | 0.03 |
|  | Weekday | 10 | 5 | 5 | 15 | 0.02 | 0.01 | 0.01 | 0.03 |
| Line 9 | Friday | 30 | 140 | 90 | 60 | 0.05 | 0.23 | 0.15 | 0.10 |
|  | Saturday | 40 | 170 | 110 | 70 | 0.07 | 0.28 | 0.18 | 0.12 |
|  | Sunday | 30 | 155 | 100 | 65 | 0.05 | 0.26 | 0.17 | 0.11 |
|  | Weekday | 30 | 135 | 85 | 55 | 0.05 | 0.23 | 0.14 | 0.09 |

As noted in the table, the maximum v/c ratios across the time periods considered are as follows (excess capacity is denoted in brackets):

- Highway 11-0.86 Friday PM northbound (excess capacity for 450 vehicles remaining);
- Simcoe Road 20-0.09 Saturday PM northbound (730 vehicles);
- Line 7-0.36 Saturday AM westbound (385 vehicles);
- Line 8-0.03 all days (585 vehicles); and
- Line 9-0.28 Saturday AM westbound (430 vehicles).

In consideration of the above, the existing road system has sufficient capacity to accommodate existing travel demands.

## 3 Future Background Conditions

This chapter will describe the road network and background traffic volumes (ie. without Burl's Creek Event Grounds) expected for the years 2020 and 2025, in order to consider the longer-term impacts (5 and 10 years beyond opening).

### 3.1 Road Network

No planned improvements to the study area road network (apart from routine maintenance) have been identified and thus the road network as discussed in Section 2.1 will be maintained through all future horizons.

### 3.2 Background Traffic Volumes

Future background traffic volumes expected for the 2020 and 2025 horizon years have been determined based on the existing traffic volumes, historical and projected growth, and additional increases in volumes due to other development within the immediate area (apart from the subject development).

### 3.2.1 Background Growth

Historic traffic volumes on Highway 11 and Simcoe Road 20 through the study area were reviewed for the 10-year period 2000 through to 2010 for Highway 11 (the most current published MTO data) and for the period 2002 to 2014 for Simcoe Road 20 (corresponding data is provided in Appendix A); 10 year planning horizons have been employed to reflect more recent growth trends and changes in travel patterns and behaviours. The following annual growth rates were realized:

- $1.5 \%$ for average annual daily traffic and $0.1 \%$ for summer average daily traffic on Highway 11 (slightly higher growth rates of $2 \%$ and $1.1 \%$ were realized for the period 2000 to 2009); and
- $\quad 0.5$ to $1.9 \%$ for daily traffic on Simcoe Road 20 (depending on road section).

To consider growth on the Township roads, population statistics have been reviewed. In 2011, the Township had a census population of 20,078 , which represent an increase of $0.2 \%$ from the 2006 level of 20,031 . On an annual basis, this translates to negligible growth over the same period. As per the Simcoe County Transportation Master Plan, the 2031 population is projected to be 28,100 which suggests an annual growth of $1.7 \%$ from 2011 to 2031 (which will largely be accommodated in the more developed, populated areas as opposed to the rural areas adjacent to the site).

In consideration of the historic growth in the area relating to both traffic (specifically the summer average daily traffic volumes in that the summer months are the subject of this study) and population,
and future growth projections for the Township, the annual growth rates as noted in Table 6 have been assumed. The resulting overall growth through to the 2020 and 2025 horizon years are also noted.

## Table 6: Background Growth

| Road | Annual Growth <br> Rate | Growth in Horizon Period |  |  |
| :--- | :---: | :---: | :---: | :---: |
| 2014 to 2015 | 2015 to 2020 | 2015 to 2025 |  |  |
| Highway 11 | $1 \%$ | $1 \%$ | $5 \%$ | $10 \%$ |
| Simcoe Road 20 | $2 \%$ | - | $10 \%$ | $22 \%$ |
| Line 7 | $1 \%$ | - | $5 \%$ | $10 \%$ |
| Line 8 | $1 \%$ | - | $5 \%$ | $10 \%$ |
| Line 9 | $1 \%$ | - | $5 \%$ | $10 \%$ |

note: compound growth has been assumed

### 3.2.2 Background Development

No other significant developments in the immediate area were identified by the Township for consideration in the background traffic volumes.

### 3.2.3 Traffic Volumes

The corresponding 2020 and 2025 background traffic volumes, as derived from the 2015 traffic counts and applying the noted adjustment and growth factors, are provided in Figure 8 through Figure 15.

### 3.3 Background Traffic Operations

### 3.3.1 Intersection Operations

The key intersections were again analysed given the projected background volumes, the results of which are provided in Table 7 for the critical 2025 Saturday peak hours. As the 2025 Saturday traffic volumes through the key intersections are greatest, this represents the worst case scenario. Provided the intersections operate with acceptable levels of service under 2025 Saturday operations, so too will they under 2020 conditions. Corresponding detailed worksheets are provided in Appendix C.

As indicated, despite the increase in background traffic volumes assumed, all of the study area key intersections will continue to provide excellent levels of service (level of service B or better) with minimal delays. In this regard, the existing road and intersection configurations are considered appropriate to serve the future background traffic volumes.

Table 7: Intersection Operations - 2025 Saturday

| Intersection and Movement |  | Control | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | delay | LOS | v/c | delay | LOS | v/c |
|  <br> Hwy 11 Overpass W | SB |  | stop | 12 | B | 0.38 | 11 | B | 0.24 |
|  <br> Hwy 11 Overpass E | SB | stop | 11 | B | 0.17 | 13 | B | 0.18 |
|  <br> Simcoe Road 20 | EB | 4-way stop | 8 | A | 0.08 | 8 | A | 0.07 |
|  | WB |  | 8 | A | 0.11 | 8 | A | 0.11 |
|  | NB |  | 9 | A | 0.29 | 8 | A | 0.13 |
|  | SB |  | 8 | A | 0.08 | 9 | A | 0.23 |
|  <br> Simcoe Road 20 | SB | stop | 9 | A | 0.04 | 9 | A | 0.04 |
|  <br> Hwy 11 Overpass W | SB | stop | 10 | A | 0.19 | 9 | A | 0.11 |
|  <br> Hwy 11 Overpass E | SB | stop | 10 | B | 0.06 | 10 | A | 0.10 |
|  <br> Simcoe Road 20 | NB | stop | 12 | B | 0.31 | 11 | B | 0.16 |
|  | SB | stop | 10 | B | 0.08 | 12 | B | 0.21 |

### 3.3.2 Road Section Operations

The resulting 2020 and 2025 background traffic volumes are presented in Table 8 and Table 9 respectively. As with the 2015 conditions, the volume to capacity ratios (ie. the extent to which the available road capacity will be consumed) has been determined. In all cases, the projected traffic volumes remain less than the corresponding traffic volumes (ie. v/c<1.0), thus suggesting despite the assumed annual growth, there will remain excess capacity to accommodate additional traffic. Of particular interest are the County and Township roads, which will operate at less than $12 \%$ and $45 \%$ of their respective capacities.

Table 8: Road Section Operations - 2020 Volumes

| Road Section \& Period |  | AM Peak Hour Volumes |  | PM Peak Hour Volumes |  | AM Peak Volume/Capacity |  | PM PeakVolume/Capacity |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | NB/EB | SB/WB | NBIEB | SB/WB | NB/EB | SBMB | NB/EB | SBMB |
| Highway 11 | Friday | 2,685 | 1,335 | 2,980 | 1,610 | 0.82 | 0.41 | 0.91 | 0.49 |
|  | Saturday | 2,660 | 1,675 | 2,615 | 1,730 | 0.81 | 0.51 | 0.80 | 0.53 |
|  | Sunday | 1,855 | 2,340 | 1,815 | 2,745 | 0.57 | 0.72 | 0.56 | 0.84 |
|  | Weekday | 1,695 | 1,295 | 2,240 | 1,580 | 0.52 | 0.40 | 0.69 | 0.48 |
| Simcoe Road 20 (Line 3 to Line 7) | Friday | 50 | 70 | 70 | 60 | 0.06 | 0.09 | 0.09 | 0.08 |
|  | Saturday | 55 | 80 | 85 | 70 | 0.07 | 0.10 | 0.11 | 0.09 |
|  | Sunday | 50 | 70 | 75 | 65 | 0.06 | 0.09 | 0.09 | 0.08 |
|  | Weekday | 50 | 65 | 70 | 55 | 0.06 | 0.08 | 0.09 | 0.07 |
| Simcoe Road 20 (Line 7 to Line 11) | Friday | 45 | 65 | 75 | 60 | 0.06 | 0.08 | 0.09 | 0.08 |
|  | Saturday | 45 | 75 | 80 | 75 | 0.06 | 0.09 | 0.10 | 0.09 |
|  | Sunday | 45 | 65 | 75 | 70 | 0.06 | 0.08 | 0.09 | 0.09 |
|  | Weekday | 40 | 60 | 65 | 60 | 0.05 | 0.08 | 0.08 | 0.08 |
| Line 7 | Friday | 70 | 195 | 140 | 100 | 0.12 | 0.33 | 0.23 | 0.17 |
|  | Saturday | 80 | 245 | 165 | 120 | 0.13 | 0.41 | 0.28 | 0.20 |
|  | Sunday | 75 | 215 | 150 | 105 | 0.13 | 0.36 | 0.25 | 0.18 |
|  | Weekday | 65 | 185 | 125 | 95 | 0.11 | 0.31 | 0.21 | 0.16 |
| Line 8 | Friday | 15 | 10 | 15 | 20 | 0.03 | 0.02 | 0.03 | 0.03 |
|  | Saturday | 15 | 15 | 15 | 20 | 0.03 | 0.03 | 0.03 | 0.03 |
|  | Sunday | 15 | 10 | 15 | 20 | 0.03 | 0.02 | 0.03 | 0.03 |
|  | Weekday | 15 | 10 | 10 | 20 | 0.03 | 0.02 | 0.02 | 0.03 |
| Line 9 | Friday | 40 | 160 | 105 | 70 | 0.07 | 0.27 | 0.18 | 0.12 |
|  | Saturday | 50 | 195 | 125 | 85 | 0.08 | 0.33 | 0.21 | 0.14 |
|  | Sunday | 40 | 175 | 115 | 75 | 0.07 | 0.29 | 0.19 | 0.13 |
|  | Weekday | 40 | 150 | 100 | 65 | 0.07 | 0.25 | 0.17 | 0.11 |

Table 9: Road Section Operations - 2025 Volumes

| Road Section \& Period |  | AM Peak Hour Volumes |  | PM Peak Hour Volumes |  | AM Peak Volume/Capacity |  | PM PeakVolume/Capacity |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | NB/EB | SB/WB | NBIEB | SB/WB | NB/EB | SBMB | NB/EB | SBIWB |
| Highway 11 | Friday | 2,820 | 1,410 | 3,145 | 1,700 | 0.86 | 0.43 | 0.96 | 0.52 |
|  | Saturday | 2,805 | 1,770 | 2,760 | 1,825 | 0.86 | 0.54 | 0.84 | 0.56 |
|  | Sunday | 1,955 | 2,465 | 1,920 | 2,890 | 0.60 | 0.75 | 0.59 | 0.88 |
|  | Weekday | 1,695 | 1,370 | 2,240 | 1,580 | 0.52 | 0.42 | 0.69 | 0.48 |
| Simcoe Road 20 (Line 3 to Line 7) | Friday | 50 | 75 | 75 | 65 | 0.06 | 0.09 | 0.09 | 0.08 |
|  | Saturday | 55 | 90 | 95 | 75 | 0.07 | 0.11 | 0.12 | 0.09 |
|  | Sunday | 50 | 75 | 85 | 70 | 0.06 | 0.09 | 0.11 | 0.09 |
|  | Weekday | 50 | 65 | 75 | 60 | 0.06 | 0.08 | 0.09 | 0.08 |
| Simcoe Road 20 (Line 7 to Line 11) | Friday | 45 | 70 | 80 | 65 | 0.06 | 0.09 | 0.10 | 0.08 |
|  | Saturday | 45 | 80 | 85 | 80 | 0.06 | 0.10 | 0.11 | 0.10 |
|  | Sunday | 45 | 70 | 80 | 75 | 0.06 | 0.09 | 0.10 | 0.09 |
|  | Weekday | 40 | 60 | 70 | 65 | 0.05 | 0.08 | 0.09 | 0.08 |
| Line 7 | Friday | 75 | 215 | 155 | 110 | 0.13 | 0.36 | 0.26 | 0.18 |
|  | Saturday | 90 | 270 | 180 | 130 | 0.15 | 0.45 | 0.30 | 0.22 |
|  | Sunday | 85 | 235 | 165 | 115 | 0.14 | 0.39 | 0.28 | 0.19 |
|  | Weekday | 70 | 205 | 135 | 105 | 0.12 | 0.34 | 0.23 | 0.18 |
| Line 8 | Friday | 15 | 10 | 15 | 20 | 0.03 | 0.02 | 0.03 | 0.03 |
|  | Saturday | 15 | 15 | 15 | 20 | 0.03 | 0.03 | 0.03 | 0.03 |
|  | Sunday | 15 | 10 | 15 | 20 | 0.03 | 0.02 | 0.03 | 0.03 |
|  | Weekday | 15 | 10 | 10 | 20 | 0.03 | 0.02 | 0.02 | 0.03 |
| Line 9 | Friday | 40 | 175 | 115 | 75 | 0.07 | 0.29 | 0.19 | 0.13 |
|  | Saturday | 50 | 215 | 135 | 90 | 0.08 | 0.36 | 0.23 | 0.15 |
|  | Sunday | 40 | 190 | 125 | 80 | 0.07 | 0.32 | 0.21 | 0.13 |
|  | Weekday | 40 | 170 | 110 | 70 | 0.07 | 0.28 | 0.18 | 0.12 |

## 4 Burl's Creek Event Grounds

This chapter will provide additional details with respect to the proposed development, including its location, the projected site generated traffic volumes and the assignment of such to the adjacent road network.

### 4.1 Proposed Use

The Burl's Creek Event Grounds is planned to be a premier event venue within Simcoe County. A range of events are to be hosted through the year including, but not limited to:

- music events;
- flea markets;
- camping and arts festivals;
- Oro-Medonte minor soccer; and
- a farmers market.

A site plan is provided in Figure 16.

### 4.2 Location \& Access

As illustrated in Figure 1, Burl's Creek Events Ground is situated between Line 7 and Line 9, on the east side (water side) of Highway 11.

As per the site plan of Figure 16, access to the site is provided via each of Line 7, Line 8 (several access points) and Line 9 . In all cases, the access points will provide for a single entry lane and single exit lane. Access points on Line 7 and Line 8 currently exist and served previous site uses; the access on Line 9 is proposed. The actual use of the access points will be dictated by the on-site event and location of activities within the site. For the larger musical events, control gates will be implemented immediately within the site boundaries at each of the Line 7, Line 8 and Line 9 access points - the single lane entry will flare to 18 control gates at Line 7, 9 at Line 8 (for VIP and RV access only) and 16 at Line 9.

The distances between the centre of Highway 11 and the centre of the access points are as follows:

- 830 metres for the Line 7 access point;
- 115, 465 and 1040 metres for the Line 8 access points;
- 1400 metres for the Line 9 access point.


### 4.3 Scheduled Events

A calendar of events planned for the year 2015 is provided in Appendix D, spanning from May to midOctober. For purposes of this assessment, 3 levels of events have been established as described below.

### 4.3.1 Level 1 Events

Level 1 events are more local in nature drawing from the Township of Oro-Medonte and immediately surrounding areas. They are expected to have shorter durations and/or fewer participants, and thus will generate fewer vehicle trips. These events include the following, with the noted number of occurrences scheduled in 2015:

- Huronia Fur \& Feathers (1 Sunday morning event);
- Farmers Market (15 Friday events); and
- Oro-Medonte Minor Soccer (13 Wednesday evening events).


### 4.3.2 Level 2 Events

Level 2 events are intended to draw from a more significant regional market, resulting in greater attendance and associated traffic levels. Events considered as Level 2 include:

- Tough Mudder parking (1 Saturday event);
- Contemporary Music \& Camping Festival (1 Saturday to Sunday event); and
- Automotive Flea Market (2 Thursday to Sunday events).


### 4.3.3 Level 3 Events

Level 3 events are the highest level of events to be staged at Burl's Creek. These events have broad market draw (provincial and beyond) and include:

- Wayhome Arts \& Music Festival (1 Thursday to Sunday event); and
- Boots and Hearts Music Festival (1 Wednesday to Sunday event).


### 4.4 Event Traffic - Trip Generation

Typically, estimates of site generated traffic volumes are based on the proposed land use, size of development and trip generation rates published in the ITE Trip Generation Manual. However, given the uniqueness of the proposed undertaking, applicable trip rates are not available. Rather trip generation estimates have been established in consideration of the specific events and operating
details associated with each, as provided by Burl's Creek Events Ground and/or as determined from the event organizers. Specific details pertaining to the derivation of the site traffic volumes for each event are detailed in the following sections.

### 4.4.1 Level 1 Events

## Huronia Fur \& Feathers

The Huronia Fur and Feathers event is oriented towards the buying, selling and/or trading of poultry, game birds, pigeons, waterfowl, rabbits, cavies, etc., as hosted by the Huronia Fur and Feather Breeders Association. Assumptions regarding the event operations are:

- auto occupancy of 1.5 people per vehicle to reflect the specialized nature of the event and propensity for single attendees; and
- on average, $20 \%$ of the daily trips will occur during each event hour (5 hour duration); assume $40 \%$ ( $2 x$ average) of daily trips in the peak hour with equal inbound and outbound trips.

| Schedule | Event <br> Attendance | Vehicle <br> Occupancy | Event <br> Vehicles | Peak Hour Vehicle Trips <br> Time |  | In |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | Out | Sunday (x1) <br> 7AM to 12PM | 500 | 1.5 per <br> vehicle | 333 | mid-morning | 133 |
| :--- | :---: | :---: | :---: | :---: | :---: |

## Farmers Market

The Farmers Market is a multi-stall market at which local farmers/producers sell products and merchandise directly to the general public, such as fresh fruit and vegetables, meat products, and other associated merchandise. Assumptions regarding the event operations are:

- vendors will arrive at the start of the market and depart at the end of the market, but market patrons will arrive/depart throughout the event;
- auto occupancy of 1.5 people per vehicle to reflect the specialized nature of the event and propensity for single attendees (which is considered conservative in that many families attend farmers markets); and
- on average, $11 \%$ of the daily trips will occur during each event hour (9 hour duration); assume $25 \%$ ( $2 x$ average) of daily trips in the peak hour with equal inbound and outbound trips

| Schedule | Event <br> Attendance | Vehicle <br> Occupancy | Event <br> Vehicles | Peak Hour Vehicle Trips |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | In | Out |  |  |  |  |
| Friday (x15) <br> 12PM to 9PM | 500 | 1.5 per <br> vehicle | 333 | mid-event | 83 | 83 |

## Oro-Medonte Minor Soccer

Assumptions regarding the soccer matches are:

- as per the Oro-Medonte Minor Soccer website, athletes will train from 6:15PM to 6:40PM with a game to follow;
- younger athletes will end at 7:30PM or 7:40PM, whereas the older athletes will end at 8:00PM;
- auto occupancy of 1.2 athletes per vehicle to reflect multiple athlete families recognizing that all athletes will play at the same time; and
- assume all athletes arrive during the initial event hour and depart during the final event hour with $10 \%$ of athletes being dropped-off and picked-up

| Schedule | Event Attendance | Vehicle Occupancy | Event Vehicles | Peak Hour Vehicle Trips |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Time | In | Out |
| Wednesday | 600 athletes | 1.2 athletes | 400 | start: | 5:15 to 6:15PM | 400 | 40 |
| 6PM to 8PM |  | per vehicle |  | end: | 7:30 to 8:30PM | 40 | 400 |

### 4.4.2 Level 2 Events

## Tough Mudder

Tough Mudder is an endurance event series in which participants attempt a military-style obstacle course. The event will be hosted at Mount St. Louis Moonstone, but Burl's Creek Events Ground will serve as the event parking facility with shuttle services provided to Mount St. Louis Moonstone. In considering the associated traffic volumes, the following have been assumed:

- 2000 vehicles to be parked;
- arrivals to the parking area are expected to occur between 5AM and 1PM (8 hours); departures are to occur between 11AM and 7PM (8 hours); and
- on average, $12.5 \%$ of the daily trips will occur during each arrival or departure hour (8 hour durations); assume 25\% ( $2 x$ average) of daily trips in the peak hour for peak direction

| Schedule | Event Attendance | Vehicle Occupancy | Event Vehicles | Peak Hour Vehicle Trips |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Time | In | Out |
| Saturday (x1) 5AM to 7PM | 2000 cars per day | n/a | $\begin{aligned} & 2000 \text { per } \\ & \text { day } \end{aligned}$ | arrive: | mid-morning | 500 | 250 |
|  |  |  |  | depart: | mid-afternoon | 250 | 500 |

## Contemporary Music \& Camping Festival

Assumptions regarding the event operations are:

- weekend event with $50 \%$ of participants to arrive on the Saturday and $50 \%$ to arrive on the Sunday;
. auto occupancy of 2.5 people per vehicle;
- $25 \%$ of daily participants are assumed to arrive during the 1 hour period prior to the start of the event with the remaining arriving during the course of the event; and
- $50 \%$ of participants are assumed to depart during the 1 hour period following the end of the event with the remaining $50 \%$ to camp overnight and depart the next morning (over the course of the morning).

| Schedule | Event <br> Attendance | Vehicle <br> Occupancy | Event <br> Vehicles |  | Peak Hour Vehicle Trips |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Time | In | Out |  |  |
| Sat to Sun (x1) <br> 12PM to 11PM | 8000 | 2.5 per <br> vehicle | 3200 per <br> day | Sat: | 11AM to 12PM | 400 | - |

With respect to the auto occupancy, data has been compiled by Burl's Creek from 12 similar events as noted below, including the 2014 Boots \& Hearts festival.

- Bonnaroo (2009 to 2013) 2.56 persons per vehicle
- Life is Good (2010) 2.98
- Electric Forest (2013) 2.48
- Rothbury (2008-2009) 2.52
- Jimmy Buffet (2010) 2.73
- All Good (2011) 2.43
- Boots \& Hearts (2014) 2.66

The above provide a range of auto occupancies from 2.43 to 2.98 people per vehicle with an average of 2.62 . For this assessment, the occupancy of 2.5 has been assumed to provide a conservative assessment.

## Barrie Automotive Flea Market

The Barrie Automotive Flea Market is an event oriented to collectibles, classic cars and automotive parts. There are also automobile auctions, shows and other associated events. Assumptions regarding the event operations are:

- auto occupancy of 2.5 persons per car to reflect the greater number of attendees, and varied nature and target markets for the event;
- vendors will arrive at the start of the flea market and depart at the end of the flea market, but patrons will arrive/depart throughout the event;
- some vendors and/or patrons will camp overnight on site; and
- on average, $8 \%$ of the daily trips will occur during each event hour (12 hour duration per day); assume $15 \%$ ( $2 x$ average) of daily trips in the peak hour with equal inbound and outbound trips.

| Schedule | Event <br> Attendance | Vehicle <br> Occupancy | Event <br> Vehicles | Peak Hour Vehicle Trips |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | In | Out |  |  |  |
| Thurs to Sun (x2) 10,000 per <br> 7AM to 7PM  | 2.5 per <br> day | 4000 per <br> vehicle | mid-event | 600 | 600 |

### 4.4.3 Level 3 Events

The Level 3 events are major music festivals, namely Wayhome Arts \& Music Festival and Boots \& Hearts Music Festival.

## Wayhome Arts \& Music Festival

Assumptions regarding the event operations are:

- auto occupancy of 2.5 people per vehicle as per the previous music festival discussion (refer to the Level 2 event Contemporary Music \& Camping Festival);
- vehicle arrivals and departures as per past events, corresponding data for which is provided in Figure 17;
- arrivals are to begin on Thursday with peak arrivals late morning/early afternoon Friday (10:00AM to 1:00PM as per past events); and
- $50 \%$ of participants are assumed to depart at the conclusion of the event (likely over several hours), with the remaining $50 \%$ to camp overnight and depart the next morning (over the course of the morning).

| Schedule | Event Attendance | Vehicle Occupancy | Event Vehicles | Peak Hour Vehicle Trips |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Time | In | Out |
| Thurs to Sun (x1) | 40,000 | 2.5 per vehicle | 16,000 | arrive: | late morn/early aft Friday | 1200 | - |
|  |  |  |  | depart: | Sun night | - | 2400 |

## Boots \& Hearts Music Festival

Assumptions regarding the event operations, as considered in the above, are:

- auto occupancy of 2.5 people per vehicle as per the previous music festival discussion (refer to the Level 2 event Contemporary Music \& Camping Festival);
- vehicle arrivals and departures as per past events, corresponding data for which is provided in Figure 18;
- arrivals are to begin on Wednesday with peak arrivals mid-day Friday (as per past events); and
- $50 \%$ of participants are assumed to depart at the conclusion of the event (likely over several hours), with the remaining $50 \%$ to camp overnight and depart the next morning (over the course of the morning).

| Schedule | Event Attendance | Vehicle Occupancy | Event Vehicles | Peak Hour Vehicle Trips |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Time | In | Out |
| Wed to Sun (x1) | 40,000 | 2.5 per vehicle | 16,000 | arrive: | late morn/early aft Friday | 900 | - |
|  |  |  |  | depart: | Sun night | - | 2400 |

### 4.5 Event Traffic - Trip Distribution \& Assignment

This section will detail the distribution of trips to surrounding areas and assignment of such to the road system.

### 4.5.1 Level 1 Events

For the Farmers Market and Oro-Medonte Minor Soccer, the visitors to Burl's Creek Event Grounds are expected to be from the immediately local area (ie. within the Township primarily). A slightly larger market area (encompassing the Cities and Orillia and Barrie, and other neighbouring Towns) is expected for the Huronia Fur and Feathers.

The following trip distributions have been assumed (based on the location of the Burl's Creek Event Grounds in relation to the anticipated markets and means of access to such):

|  | Farmers Market <br> Minor Soccer | Huronia Fur and Feathers |
| :--- | :--- | :---: |
| . tolfrom Highway 11 south | $30 \%$ | $50 \%$ |
| . tolfrom Highway 11 north | $30 \%$ | $30 \%$ |
| . to/from Lines 7, 8 \& 9 west | $30 \%$ | $10 \%$ |
| . to/from Simcoe Road 20 south | $5 \%$ | $5 \%$ |
| " to/from Simcoe Road 20 north | $5 \%$ | $5 \%$ |

### 4.5.2 Level 2 Events

The assumed distribution for the Level 2 events is as follows:
Contemporary Music \& Camping Festival
Automotive Flea Market Tough Mudder

- to/from Highway 11 south

50\%
70\%

- tolfrom Highway 11 north

30\%
20\%

- to/from Lines 7, 8 \& 9 west

10\%
10\%

- to/from Simcoe Road 20 south
- to/from Simcoe Road 20 north

5\%
0\%
5\%
0\%
As evident, it is assumed that the Tough Mudder will have a greater draw to areas to the south (eg. York Region and the GTA) and less of a draw from the local areas.

### 4.5.3 Level 3 Events

As previously noted, Level 3 events are considered major events and will generate significant traffic volumes. Given the limited occurrence of such, and the traffic volumes to be accommodated, separate traffic control plans have been established for the Wayhome Arts \& Music Festival and the Boots \& Hearts Music Festival, the intent of which is to establish transportation strategies to best accommodate the demands and minimize impacts through temporary traffic control measures. In this regard, these events have not been considered further in this assessment.

### 4.5.4 Traffic Volumes

The resulting assignments of the event related traffic volumes to the area road system, following the previously noted distributions, are presented in Figure 19 through Figure 24. The assignment of the site trips to the site access points recognizes the location of each event within the event grounds as per information provided by Burl's Creek Event Grounds (the information is noted on the event calendar and corresponding zone map in Appendix D). For those events with defined inbound and outbound peaks (ie. patrons will arrive at the start of the event and leave at the end), volumes have been illustrated separately.

## 5 Transportation Impacts

This chapter will address the resulting impacts of the proposed Burl's Creek Event Grounds development on the adjacent road system.

### 5.1 Traffic Volumes with Burl's Creek Traffic

The site generated traffic volumes for each individual event have been combined with the future 2015, 2020 and 2025 background traffic volumes for the corresponding day (eg. Friday, Saturday, Sunday or weekday) and time period (AM or PM) to realize the future total traffic volumes. For events over multiple days, volumes for the most critical day have been established. The AM or PM peak period was selected to best correspond to the peak hour of the event - the AM peak was used for events that peak mid-morning, whereas the PM peak was used for events that peak mid-afternoon or evening. In most cases, this represents a conservative approach in that the daily peaks (particularly on weekdays) correspond more closely to the start and end of the work day, volumes outside of these periods (during which the events are expected to peak) are generally less.

The corresponding traffic volumes are provided in Figure 25 through Figure 42; derivations of the future total traffic volumes (ie. background traffic volumes + site traffic volumes $=$ total volumes) are provided for each event in Appendix E. Only data corresponding to the peak hour deemed most applicable to the event operations has been illustrated (eg. the Huronia Fur \& Feathers event will occur during the morning, and thus only the AM peak hour has been considered as the event will not generate any significant traffic during the PM peak hour, if any at all).

### 5.2 Traffic Operations with Burl's Creek Traffic

### 5.2.1 Intersection Operations

The key intersections were again analyzed to establish potential impacts associated with the Burl's Creek Event Grounds. In consideration of the number of events and horizon years, the analysis has focussed on the 2025 horizon, recognizing that the associated volumes are greatest. Provided the 2025 traffic operations are acceptable, so too will the 2015 and 2020 operations be acceptable. The intermediate horizon years of 2015 and 2020 were only considered if significant operational impacts have been identified and further details as to related timings was considered necessary.

The results of the assessments are summarized in Table 10 through Table 15 for each specific event and considering the associated day and time of the event peak hours (eg. weekday evening vs Saturday morning). The corresponding detailed worksheets are provided in Appendix F.

Table 10: Intersection Operations - 2025 Huronia Fur \& Feathers (Sunday mid-morning)

| Intersection and Movement |  | Control | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | delay | LOS | v/c | delay | LOS | v/c |
|  <br> Hwy 11 Overpass W | WB |  | stop | 14 | B | 0.48 | - | - | - |
|  <br> Hwy 11 Overpass E | WB | stop | 16 | C | 0.34 | - | - | - |
|  <br> Simcoe Road 20 | EB |  | 8 | A | 0.08 | - | - | - |
|  | WB | 4-way | 8 | A | 0.11 | - | - | - |
|  | NB | stop | 9 | A | 0.24 | - | - | - |
|  | SB |  | 8 | A | 0.10 | - | - | - |
|  <br> Simcoe Road 20 | SB | stop | 9 | A | 0.04 | - | - | - |
|  <br> Hwy 11 Overpass W | EB | stop | 9 | A | 0.17 | - | - | - |
|  <br> Hwy 11 Overpass E | EB | stop | 10 | B | 0.05 | - | - | - |
|  <br> Simcoe Road 20 | NB | stop | 12 | B | 0.28 | - | - | - |
|  | SB | stop | 11 | B | 0.08 | - | - | - |
|  <br> Site Access | WB | stop | 5 | B | 0.19 | - | - | - |
| Line 8 \& Site Access | EB | stop | 0 | A | 0 | - | - | - |
|  | WB | stop | 0 | A | 0 | - | - | - |
| Line 9 \& Site Access | EB | stop | 0 | A | 0 | - | - | - |

Under the Huronia Fur and Feathers event, all intersection operations are considered excellent (levels of service A or B) with minimal delays. As such, no road system improvements are considered necessary to support this event, as based on the noted traffic operations.

Table 11: Intersection Operations - 2025 Farmers Market (Friday Evening)

| Intersection and Movement |  | Control | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | delay | LOS | v/c | delay | LOS | v/c |
|  <br> Hwy 11 Overpass W | WB |  | stop | - | - | - | 10 | B | 0.20 |
|  <br> Hwy 11 Overpass E | WB | stop | - | - | - | 11 | B | 0.27 |
|  <br> Simcoe Road 20 | EB |  | - | - | - | 8 | A | 0.11 |
|  | WB | 4-way | - | - | - | 8 | A | 0.10 |
|  | NB | stop | - | - | - | 8 | A | 0.12 |
|  | SB |  | - | - | - | 9 | A | 0.19 |
|  <br> Simcoe Road 20 | SB | stop | - | - | - | 9 | A | 0.05 |
|  <br> Hwy 11 Overpass W | EB | stop | - | - | - | 9 | A | 0.15 |
|  <br> Hwy 11 Overpass E | EB | stop | - | - | - | 10 | A | 0.08 |
| Line 9 \& Simcoe Road 20 | NB | stop | - | - | - | 11 | B | 0.14 |
|  | SB | stop | - | - | - | 11 | B | 0.18 |
|  <br> Site Access | WB | stop | - | - | - | 0 | A | 0.19 |
| Line 8 \& Site Access | EB | stop | - | - | - | 10 | A | 0.10 |
|  | WB | stop | - | - | - | 0 | A | 0 |
|  <br> Site Access | EB | stop | - | - | - | 0 | A | A |

Under the Farmers Market event, all intersection operations are considered excellent (levels of service A or B) with minimal delays. As such, no road system improvements are considered necessary to support this event, as based on the noted traffic operations.

Table 12: Intersection Operations - 2025 Minor Soccer (Wednesday evening)

| Intersection and Movement |  | Control | Inbound Peak Hour |  |  | Outbound Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | delay | LOS | v/c | delay | LOS | v/c |
|  <br> Hwy 11 Overpass W | WB |  | stop | 16 | C | 0.37 | 14 | B | 0.56 |
|  <br> Hwy 11 Overpass E | WB | stop | 60 | F | 0.93 | 20 | C | 0.32 |
|  <br> Simcoe Road 20 | EB | 4-way stop | 8 | A | 0.14 | 8 | A | 0.11 |
|  | WB |  | 8 | A | 0.12 | 8 | A | 0.10 |
|  | NB |  | 8 | A | 0.11 | 8 | A | 0.11 |
|  | SB |  | 9 | A | 0.20 | 9 | A | 0.24 |
|  <br> Simcoe Road 20 | SB | stop | 1 | A | 0.01 | 1 | A | 0.01 |
|  <br> Hwy 11 Overpass W | EB | stop | 9 | A | 0.09 | 9 | A | 0.09 |
|  <br> Hwy 11 Overpass E | EB | stop | 9 | A | 0.08 | 9.4 | A | 0.08 |
|  <br> Simcoe Road 20 | NB | stop | 11 | B | 0.10 | 11 | B | 0.10 |
|  | SB | stop | 11 | B | 0.15 | 11 | B | 0.15 |
| Line 7 \& Site Access | WB | stop | 12 | B | 0.10 | 15 | B | 0.59 |
| Line 8 \& Site Access | EB | stop | 0 | A | 0.10 | 0 | A | 0.10 |
|  | WB | stop | 0 | A | 0 | 0 | A | 0 |
|  <br> Site Access | EB | stop | 0 | A | 0 | 0 | A | 0 |

For the Minor Soccer events, some higher delays are incurred, resulting in slightly poorer levels of service. Notwithstanding, all intersection operations are considered acceptable (in that they are less than the level of service $F$, which denotes the intersection or movement capacity has been fully consumed), with the exception of the intersection of Line 7 with the east Highway 11 overpass. At this intersection, the average delay is 60 seconds for the westbound movement, thus resulting in a level of service F. However, such a delay is not considered unbearable. Furthermore, as these conditions will be experienced only for the peak inbound direction of soccer related traffic, which is limited to a 1 hour period, no road improvements are considered necessary at this intersection.

Table 13: Intersection Operations - 2025 Tough Mudder (Saturday)

| Intersection and Movement |  | Control | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | delay | LOS | v/c | delay | LOS | v/c |
|  <br> Hwy 11 Overpass W | WB |  | stop | 14 | B | 0.42 | 11 | B | 0.24 |
|  <br> Hwy 11 Overpass E | WB | stop | 12 | B | 0.34 | 12 | B | 0.24 |
|  <br> Simcoe Road 20 | EB | 4-way stop | 8 | A | 0.07 | 8 | A | 0.12 |
|  | WB |  | 8 | A | 0.10 | 8 | A | 0.10 |
|  | NB |  | 9 | A | 0.27 | 8 | A | 0.14 |
|  | SB |  | 8 | A | 0.08 | 9 | A | 0.23 |
|  <br> Simcoe Road 20 | SB | stop | 9 | A | 0.04 | 1 | A | 0.01 |
|  <br> Hwy 11 Overpass W | EB | stop | 11 | B | 0.39 | 13 | B | 0.54 |
|  <br> Hwy 11 Overpass E | EB | stop | 12 | B | 0.07 | 17 | C | 0.21 |
|  <br> Simcoe Road 20 | NB | stop | 12 | B | 0.29 | 11 | B | 0.15 |
|  | SB | stop | 10 | B | 0.08 | 11 | B | 0.20 |
|  <br> Site Access | WB | stop | 0 | A | 0 | 0 | A | 0 |
| Line 8 \& Site Access | EB | stop | 0 | A | 0 | 0 | A | 0 |
|  | WB | stop | 10 | A | 0.26 | 12 | B | 0.51 |
|  <br> Site Access | EB | stop | 0 | A | 0 | 0 | A | 0 |

During the Tough Mudder peak hour operations, all intersections will provide acceptable operations (level of service C or better) and thus no improvements are considered necessary.

Table 14: Intersection Operations - 2025 Contemporary Music \& Camping Festival (Sat mid-day)

| Intersection and Movement |  | Control | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | delay | LOS | v/c | delay | LOS | v/c |
|  <br> Hwy 11 Overpass W | WB |  | stop | - | - | - | 12 | B | 0.30 |
|  <br> Hwy 11 Overpass E | WB | stop | - | - | - | 12 | B | 0.36 |
|  <br> Simcoe Road 20 | EB |  | - | - | - | 9 | A | 0.16 |
|  | WB | 4-way | - | - | - | 8 | A | 0.11 |
|  | NB | stop | - | - | - | 8 | A | 0.15 |
|  | SB |  | - | - | - | 9 | A | 0.24 |
|  <br> Simcoe Road 20 | SB | stop | - | - | - | 9.5 | A | 0.04 |
|  <br> Hwy 11 Overpass W | EB | stop | - | - | - | 9 | A | 0.11 |
|  <br> Hwy 11 Overpass E | EB | stop | - | - | - | 10 | A | 0.10 |
| Line 9 \& Simcoe Road 20 | NB | stop | - | - | - | 11 | B | 0.16 |
|  | SB | stop | - | - | - | 12 | B | 0.21 |
|  <br> Site Access | WB | stop | - | - | - | 0 | A | 0 |
| Line 8 \& Site Access | EB | stop | - | - | - | 0 | A | 0 |
|  | WB | stop | - | - | - | 0 | A | 0.51 |
|  <br> Site Access | EB | stop | - | - | - | 0 | A | 0 |

Under the Contemporary Music and Camping Festival, all intersection operations are considered excellent (levels of service A or B) with minimal delays. As such, no road system improvements are considered necessary to support this event, as based on the noted traffic operations.

Table 15: Intersection Operations - 2025 Barrie Automotive Flea Market (Sat mid-day)

| Intersection and Movement |  | Control | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | delay | LOS | v/c | delay | LOS | v/c |
|  <br> Hwy 11 Overpass W | WB |  | stop | 17 | C | 0.58 | - | - | - |
|  <br> Hwy 11 Overpass E | WB | stop | 30 | D | 0.66 | - | - | - |
|  <br> Simcoe Road 20 | EB |  | 9 | A | 0.18 | - | - | - |
|  | WB | 4-way | 9 | A | 0.14 | - | - | - |
|  | NB | stop | 9 | A | 0.15 | - | - | - |
|  | SB |  | 10 | A | 0.29 | - | - | - |
|  <br> Simcoe Road 20 | SB | stop | 1 | A | 0.01 | - | - | - |
|  <br> Hwy 11 Overpass W | EB | stop | 11 | B | 0.34 | - | - | - |
|  <br> Hwy 11 Overpass E | EB | stop | 11 | B | 0.24 | - | - | - |
|  <br> Simcoe Road 20 | NB | stop | 12 | B | 0.18 | - | - | - |
|  | SB | stop | 12 | B | 0.33 | - | - | - |
|  <br> Site Access | WB | stop | 13 | B | 0.43 | - | - | - |
| Line 8 \& Site Access | EB | stop | 0 | A | 0 | - | - | - |
|  | WB | stop | 0 | A | 0.51 | - | - | - |
| Line 9 \& Site Access | EB | stop | 0 | A | 0 | - | - | - |

All intersection operating conditions are considered acceptable under this event (level of service D or better) and thus no improvements are warranted to address poor traffic operations.

### 5.2.2 Queve Operations

Further to the intersection operations, the presence of traffic queues has been reviewed for each stop controlled movement and also the left turns on the major roads (which operate under free flow conditions and are thus only obstructed by opposing traffic). The corresponding 95th percentile queues are noted in Table 16 for the 2025 Saturday operations (the $95^{\text {th }}$ percentile queue represents the queue length that would only be exceeded $5 \%$ of the time).

Table 16: Queue Operations - 2025 with Burl's Creek

| Intersection and Movement |  | Contro | 95th Percentile Queues (metres) |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | \& | $\frac{\stackrel{\pi}{6}}{\stackrel{\pi}{y}}$ |  |  |  |  |
| Line 7 \& | WB |  | stop | 20 | 6 | 13 | 7 | 9 | 28 |
| Hwy 11 Overpass W | SB | left | 1 | 1 | 4 | 1 | 2 | 1 |
| Line 7 \& | WB | stop | 11 | 8 | 79 | 7 | 13 | 35 |
| Hwy 11 Overpass E | SB | left | 2 | 2 | 2 | 2 | 2 | 3 |
|  | EB |  | 1 | 1 | 1 | 1 | 1 | 1 |
| Line 7 \& | WB | 4-way | 1 | 1 | 1 | 1 | 1 | 1 |
| Simcoe Road 20 | NB | stop | 1 | 1 | 1 | 1 | 1 | 1 |
|  | SB |  | 1 | 1 | 1 | 1 | 1 | 1 |
| Line 8 \& | SB | stop | 1 | 1 | 1 | 1 | 1 | 1 |
| Simcoe Road 20 | EB | left | 1 | 1 | 1 | 1 | 1 | 1 |
| Line 9 \& | EB | stop | 5 | 4 | 2 | 26 | 3 | 12 |
| Hwy 11 Overpass W | NB | left | 1 | 1 | 1 | 1 | 1 | 3 |
| Line 9 \& | EB | stop | 1 | 2 | 2 | 6 | 3 | 7 |
| Hwy 11 Overpass E | NB | left | 2 | 1 | 1 | 2 | 1 | 0 |
|  | NB | stop | 1 | 1 | 1 | 1 | 1 | 1 |
| Line 9 \& | SB | stop | 1 | 1 | 1 | 1 | 2 | 1 |
| Simcoe Road 20 | EB | left | 9 | 4 | 3 | 4 | 11 | 5 |
|  | WB | left | 2 | 5 | 4 | 6 | 12 | 11 |


| Intersection and Movement |  | Contro | 95th Percentile Queues (metres) |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | ※品 | $\frac{\stackrel{\pi}{0}}{\frac{\ddot{\omega}}{6}}$ |  |  |  |  |
| Line 7 \& | WB |  | stop | 5 | 0 | 2 | 0 | 0 | 17 |
| Site Access | SB | left | 0 | 0 | 12 | 0 | 0 | 5 |
|  | EB | stop | 0 | 3 | 0 | 0 | 0 | 0 |
| Line 8 \& | WB | stop | 0 | 0 | 0 | 23 | 0 | 0 |
| Site Access | NB | left | 0 | 1 | 0 | 0 | 1 | 0 |
|  | SB | left | 0 | 0 | 0 | 5 | 0 | 5 |
| Line 9 \& | EB | stop | 0 | 0 | 0 | 0 | 0 | 0 |
| Site Access | NB | left | 0 | 0 | 0 | 0 | 0 | 0 |

As noted, the $95^{\text {th }}$ percentile queues are typically in the order of 1 vehicle or less (1 vehicle $=7.6$ metre queue length). Queues at the Line 7 and Line 9 overpass intersections are somewhat greater (up to 10 cars or 79 metres under the Minor Soccer event), but remain within reason. At the site access points, any queueing can be accommodated readily within the site. At no point are traffic queues on Line 7 or Line 9 projected to extend to Highway 11 thus interfering with highway operations.

### 5.2.3 Right-In/Right-Out Operations

There are 2 right-in/right-out intersections at each of the intersections of Line 7, Line 8 and Line 9 with Highway 11. In all cases, the right-in to the highway is provided with a free flow movement, with an acceleration lane provided on the highway. This allows motorists to enter the highway and attain an appropriate travel speed prior to merging with the outside lane of traffic. Similarly, each right-out movement has a separate deceleration and a free-flow exit manoeuvre from the highway, thereby minimizing (if not negating) potential impacts to Highway 11. In consideration of these operations, the right-in/right-out intersections were not specifically considered (as each movement operates under free flow there are no delays and hence a level of service A results).

Traffic simulations were also reviewed for the Tough Mudder event, which results in the greatest volume of traffic entering and exiting Highway 11 at a single right-in/right-out location (the corresponding projected volumes are estimated at 515 to 520 vehicles per hour, which equates to less than 10 vehicles per minute). Based on the simulation results, no impacts to the highway operations are expected.

### 5.2.4 Link Operations

The resulting 2015, 2020 and 2025 total volume to capacity ratios are presented in the tables of Appendix E for each event being considered, for those days and time periods to which the event contributes additional traffic. Summaries of the peak hour v/c ratios for each event are provided in Table 17, Table 18 and Table 19 for the 2015, 2020 and 2025 horizon years. Where an event has multiple peaks, only the most critical figures are noted (ie. the greatest v/c ratios). When a v/c ratio exceeds 0.90 , the road section is nearing capacity whereas when the v/c exceeds 1.0 , the road section volumes surpass the capacity.

Table 17: Volume to Capacity Ratios - 2015 Volumes with Burl's Creek

| Road Section \& Direction |  | Level 1 Events |  |  | Level 2 Events |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Fur \& | Farmers | Minor | Tough | Music \& | Auto Flea |
| Highway 11 | NB | 0.56 | 0.89 | 0.66 | 0.92 | 0.82 | 0.86 |
|  | SB | 0.70 | 0.48 | 0.48 | 0.62 | 0.83 | 0.89 |
| Simcoe Road 20 | NB | 0.05 | 0.08 | 0.10 | - | 0.11 | 0.13 |
|  | SB | 0.08 | 0.06 | 0.09 | - | 0.10 | 0.12 |
| Line 7 | NB | 0.31 | - | 0.93 | - | - | 0.62 |
|  | SB | 0.52 | - | 0.88 | - | - | 0.81 |
| Line 8 | EB | - | 0.14 | - | 0.85 | 0.62 | 0.39 |
|  | WB | - | 0.15 | - | 0.86 | 0.09 | 0.48 |
| Line 9 | EB | - | 0.23 | - | 0.52 | - | 0.48 |
|  | WB | - | - | - | - | - | - |

## Highway 11 Operations

In considering operations on Highway 11, the various events will contribute an additional 30 to 500 vehicles per hour per direction on select sections of Highway 11. Combined with the existing and future traffic volumes on the highway, the future total traffic volumes (ie. with consideration for Burl's Creek) on Highway 11 will approach capacity (ie. v/c $>0.90$ ) or slightly exceed its capacity (v/c of 1.01) during the associated peak hours of the noted events.

Table 18: Volume to Capacity Ratios - 2020 Volumes with Burl's Creek

|  <br> Direction |  <br> Feathers | Level 1 Events <br> Farmers <br> Market | Minor <br> Soccer | Tough <br> Mudder |  <br> Camping | Auto Flea <br> Market |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | NB | 0.59 | 0.93 | 0.73 | 0.97 | 0.86 | 0.91 |
|  | SB | 0.74 | 0.51 | 0.53 | 0.65 | 0.88 | 0.93 |
| Simcoe Road 20 | NB | 0.07 | 0.10 | 0.12 | - | 0.13 | 0.14 |
|  | Line 7 | 0.10 | 0.08 | 0.11 | - | 0.12 | 0.14 |
| Line 8 | NB | 0.33 | - | 0.96 | - | - | 0.65 |
|  | SB | 0.56 | - | 0.91 | - | - | 0.86 |
|  | EB | - | 0.15 | - | 0.86 | 0.63 | 0.40 |
|  | WB | - | 0.16 | - | 0.87 | 0.10 | 0.48 |

Table 19: Volume to Capacity Ratios - 2025 Volumes with Burl's Creek

| Road Section \& Direction |  | Level 1 Events |  |  | Level 2 Events |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Fur \& | Farmers | Minor | Tough | Music \& | Auto Flea |
| Highway 11 | NB | 0.62 | 0.98 | 0.73 | 1.01 | 0.91 | 0.95 |
|  | SB | 0.77 | 0.54 | 0.53 | 0.68 | 0.92 | 0.98 |
| Simcoe Road 20 | NB | 0.07 | 0.11 | 0.13 | - | 0.14 | 0.16 |
|  | SB | 0.10 | 0.09 | 0.11 | - | 0.13 | 0.15 |
| Line 7 | NB | 0.34 | - | 0.98 | - | - | 0.68 |
|  | SB | 0.59 | - | 0.93 | - | - | 0.90 |
| Line 8 | EB | - | 0.15 | - | 0.86 | 0.63 | 0.40 |
|  | WB | - | 0.16 | - | 0.87 | 0.10 | 0.48 |
| Line 9 | EB | - | 0.28 | - | 0.56 | - | 0.53 |
|  | WB | - | - | - | - | - | - |

The greatest increase will result from the Tough Mudder event (only 1 occurrence) with arrivals primarily occurring the morning hours and departures during the afternoon hours on the corresponding Saturday. The associated v/c ratios range from 0.92 to 1.01 in the Highway 11 northbound direction of the AM peak hour, which is conservative in that it assumes the peak hours for the Tough Mudder will correspond to the peak hours of the highway. It is further noted that only the section of Highway 11 between Line 7 and Line 8 will realize an increase of 500 vehicles during the arrival peak hour, whereas only the section from Line 8 to Line 9 will realize the same increase during the departure peak hour. Outside of these road sections, the additional volumes are projected in the order of 100 to 350 vehicles depending on direction of travel. This increase results from the need for travelers to utilize the downstream intersections at Line 9 and Line 7 to change travel direction for access to/from Line 8. This also assumes that all vehicles travelling to/from the site will utilize Line 8 to access the highway. Any redistribution of traffic to Line 7 or Line 9 will lessen the incremental increases in traffic volumes through the noted sections of Highway 11. Given the short duration of the increased traffic volumes, the operating conditions are considered acceptable, and thus no improvements are warranted. It is noted that consideration could be given to providing access to the parking area via Line 7 or Line 9 as appropriate, to provide a more distributed arrival pattern during the Tough Mudder event.

The Automotive Flea Market is expected to contribute 300 vehicles per hour per direction on Highway 11 to/from areas to the south, resulting in a v/c ratio of 0.89 to 0.98 in the vicinity of Line 7. Burl's Creek will operate parking/access off Line 7 and Line 8 during this event, thus distributing the traffic between these 2 roads accordingly, and alleviating demands on the highway system beyond Line 7 . Again, given the short nature of this event (2 occurences per year), no improvements to the highway system are considered necessary.

## Simcoe Road 20 Operations

The existing traffic volumes on Simcoe Road 20 equate to approximately 9\% or less of its available road capacity (ie. volumes of 70 vehicles per hour or less vs a capacity of 800 vehicles per hour per lane). While Simcoe Road 20 provides an alternative and parallel travel route to Highway 11, it does not extend beyond Line 11 to the north. To the south, Simcoe Road 20 turns into Shanty Bay Road in the north end of the City of Barrie. While Simcoe Road 20 provides a connection to the City, the majority of site visitors are expected to access the site via Highway 11. Increases of 20 to 30 vehicles per hour per direction are projected (as per Figure 19 through Figure 24), which can be readily accommodated on the road ( $25 x$ the amount could in fact be accommodated).

## Township Road Operations

The only noted issue on the Township roads is that of Line 7 associated with the Minor Soccer events, during which time traffic volumes from Highway 11 to the site access are projected to be 93 to $98 \%$ of capacity. During this event ( 13 Wednesday evening occurrences), all 600 athletes are assumed to arrive during the same hour in that all of the soccer sessions will commence at 6:15 (which corresponds to the commuter PM peak hour). Based on the traffic projections and assignment of
traffic volumes to the area roads, peak hour peak direction volumes during this arrival hour will approach 600 vehicles per hour (which is the assumed planning capacity of Line 7). It is assumed that the primary access route will consist of Line 7 from Highway 11 to the site, although there are other possible routes via Line 9 and Simcoe Road 20 that would reduce demands on Line 7. The concluding times for the soccer events will be staggered somewhat due to different match durations for different age groups. A similar approach could also be adopted to stagger the start times by age group, thus alleviating some of the peak hour demands. Given that the road will continue to operate below capacity, that other alternative routes are possible and that changes to the actual event operations can also be considered (eg. staggering start and end times, hosting the soccer matches over several nights or providing access via Line 8), no improvements to the road system are considered necessary to support the soccer events.

The more significant increases in traffic volumes on the local roads associated with the remaining events are as follows:

- 250 to 500 vehicles per direction on Line 8 and 200 per direction on Line 9 during Tough Mudder;
- up to 360 vehicles per direction on Line 8 during the Contemporary Music and Camping Festival; and
- 180 to 270 vehicles per direction on Lines 7, 8 and 9 during the Barrie Automotive Flea Market.

Notwithstanding the noted increases, the resulting total volumes will remain below the assumed road capacities (particularly on Line 8 on which existing volumes are minimal) and thus are acceptable.

### 5.3 Turn Lane Requirements

The need for turn lanes at the site access points has been considered based on the nature of the event (occurrences per year and duration) and corresponding traffic volumes (both existing and future). As several of the events will occur over a single day, and may only happen once or twice per year, it is not considered necessary to implement permanent infrastructure improvements.

### 5.3.1 Right Turn Lanes

As per the MTO Geometric Design Guidelines for Ontario Highways, right turn lanes are recommended when the turning volume exceeds 60 vehicles per hour and has the potential to impact through movements due to turning vehicles. In considering the projected volumes for each event (as per Figure 19 through Figure 24), the noted threshold for a right turn lane is exceeded during the following events:

- Farmers Market - 75 EB right turns at the Line 8 access (Friday evening);
- Contemporary Music \& Camping Festival - 360 EB right turns at the Line 8 access (Saturday/ Sunday mid-day); and
- Barrie Automotive Flea Market - 120 WB right turns at the Line 8 access (Saturday/Sunday midday).

However, as Line 8 has a limited service area (it extends only from Highway 11 to Simcoe Road 20), existing volumes on this road section are minimal (less than 20 vehicles per during the peak hours). In this regard, any impacts to through traffic, resulting from the right turning traffic, will be negligible. In this regard, right turn lanes are not considered necessary.

### 5.3.2 Left Turn Lanes

As per the MTO Geometric Design Guidelines for Ontario Highways, the need for left turn lanes is based on left turn traffic volumes, through volumes and the design speed. As with right turn lanes, left turn lanes are recommended when the volume of turning traffic unduly impacts the through traffic. Increased left turn movements are anticipated for the following events:

- Huronia Fur \& Feathers - 120 EB left turns at the Line 7 access (Sunday mid-morning);
- Minor Soccer - 450 EB left turns at the Line 7 access (Wednesday evening);
- Tough Mudder - 250 to 500 EB left turns at the Line 8 access (Saturday mid-morning and midafternoon); and
- Barrie Automotive Flea Market - 225 EB left turns at the Line 7 and Line 8 access (Saturday/Sunday mid-day).

Apart from Minor Soccer, the occurrences of the remaining events is limited (2 Sunday mornings for Fur \& Feathers, 1 Saturday for Tough Mudder and 4 days for the Flea Market x 2 events) and thus left turn lanes to support these events are not considered necessary (particularly on Line 8 given the limited volume of other traffic on the road during the noted peak hours).

The Minor Soccer events are expected on 13 Wednesday evenings over the months of May, June, July and August. Based on the MTO left turn lane warrants, an EB left turn lane is warranted at the Line 7 access (the provision of which would also serve the remaining events using the same access). In considering a $70 \mathrm{~km} / \mathrm{h}$ design speed ( $10 \mathrm{~km} / \mathrm{h}$ over the speed limit), the left turn lane should consist of a 25 metre storage length, a 40 metre parallel lane and a 115 metre taper. Based on a review of Simcoe County mapping, Line 7 has a 26 metre right-of-way in the area of the site access (original 20 metres +6 metre widening along the north side). As the road is centred in the original 20 metre right-of-way, it is offset to the south within the 26 metre right-of-way. In consideration of this, and given the line of large, mature trees along the south side of Line 7, the left turn lane should be implemented on the north side of the road where it can be more readily implemented with lesser impacts.

### 5.4 Monitoring of Traffic Operations

It is acknowledged that the traffic volumes for the basis of the operational and turn lane assessments have been established from limited available existing data and projections for the corresponding events. In this regard, it is recommended that site operations be monitored over the course of the initial year of operations to confirm traffic volumes along the area roads and to/from the site, from which the need for and exact timing of, the road improvements can be established.

As many of the events will have limited duration and limited occurrences, supporting permanent infrastructure may not be required. While some impacts may occur (eg. congestion and increased delays to the motoring public) such are tolerable given the limited nature. Furthermore, for particular events, temporary traffic control measures could be considered as warranted by operations (eg. traffic control persons at the site access points).

## Summary

This study has reviewed the transportation impacts associated with the proposed Burl's Creek Event Grounds, located east of Highway 11 between Line 7 and Line 9 in Oro-Medonte.

## Existing Traffic Operations

Based on 2014/15 traffic volumes on the study area roads, all relevant road sections (including those on Highway 11) are operating below their respective planning capacities. There are no known operational issues at any of the corresponding study area intersections given the limited volumes single lane approaches and stop control are considered appropriate.

## Future Traffic Operations

A 2015 horizon (opening year), 2020 horizon (5-year beyond opening) and 2025 horizon (10-year beyond opening) were considered for this study.

An annual growth rate of $1 \%$ was applied to the existing traffic volumes on Highway 11 and the local Township roads, and $2 \%$ on Simcoe Road 20 to estimate future background traffic volumes for the horizon years. These were based on historic growth in summer traffic conditions and in considering future growth potential in the area.

Under future background conditions (ie. without Burl's Creek Event Grounds traffic) the study area roads will continue to operate below their capacity levels. Highway 11 will operate at $86 \%$ or less of its capacity during the Saturday, Sunday and weekday peak hours, and $96 \%$ or less during the more critical Friday peak hour (which reflects the increase in NB traffic heading to "cottage country"). Simcoe Road 20 and the Township roads will operate at less than $12 \%$ and $36 \%$ of their respective capacities.

## Burl's Creek Event Grounds

The Burl's Creek Event Grounds has 8 distinct events currently planned for the 2015 season, including soccer, music festivals, satellite parking, famer and flea markets, and camping. Attendance at the site events is expected to range from 500 people per day to 40,000 people per day. In considering auto occupancy levels of 1.5 to 2.5 persons per vehicles (depending on event) 333 to 16,000 vehicles are expected per day. Based on event durations and arrival/departure times, and not considering the major music festivals (which have been addressed in a separate traffic control report), the site is expected to generate 167 to 1200 vehicle trips per hour (total of inbound and outbound trips).

Existing site access will be largely maintained, providing access via Line 7 and Line 8 (with multiple access points along the latter road). Access is also proposed vial Line 9 for the major music festivals.

## Future Traffic Operations with Burl's Creek

In considering the future total traffic volumes (eg. background traffic + event traffic) associated with each event (with the exception of the major music festivals), the area intersections will continue to operate below their capacities with a few exceptions.

- The key study area intersections will typically operate at level of service A or B, with decreased levels of service $C$ or $D$ occurring at select intersections under select events. All of these operations are considered acceptable.
- Under the Minor Soccer events, the intersection of Line 7 and the Highway 11 overpass on the east side of the highway will experience a level of service $F$ under 2025 conditions resulting from increased delays for those at the stop sign. However, the corresponding delay of 60 seconds is not considered unbearable, and will only occur over a short arrival period (all athletes will arrive over the course of 1 hour). The $95^{\text {th }}$ percentile queue for the southbound movements on the overpass is estimated at 79 metres under the 2025 conditions, which correlates to approximately 10 vehicles, and can be accommodated on the overpass.

Similarly, the key road sections will typically operate below their respective capacities, following consideration for the Burl's Creek Event Grounds site traffic.

- Highway 11 is projected to operate at $48 \%$ to $101 \%$ of capacity over the 2015 to 2025 horizon years (the maximum under the future background traffic levels was $96 \%$ with the incremental increase associated with site operations). As such, Highway 11, the major road providing service to the area can accommodate the projected event traffic (operations at capacity will be limited in duration and occurrence).
- Simcoe Road 20 is expected to operate at $5 \%$ to $16 \%$ during the same periods.
- On Line 7, the projected volumes will approach the planning capacity of 600 vehicles per hour per lane on the section from Highway 11 to the site access during the Minor Soccer events. Peak hour peak directional volumes are projected in the order of 555 to 585 vehicles per hour. However, this is not considered critical given the relatively short section of road in question and the rural and remote nature of the area, and limited duration (soccer occurs over 2 hours). Notwithstanding, consideration could be given to staggering the start time of the soccer matches to better accommodate arrival traffic (as it is, all 600 participants start at 6:15PM). There are also alternative routes to/from the soccer pitches that users will divert to should operations become problematic (the majority of traffic was assumed to access the site via Line 7 and Highway 11). Additional means of access to/from the site could also be provided for these events (eg. utilizing site access of Line 7 and Line 8).
- On Line 8, traffic volumes associated with the Tough Mudder will amount to in excess of 500 vehicles per hour through the section from Highway 11 to the site access (approximately $86 \%$ of capacity). However, this road has minimal background traffic volumes (ie. most of the noted volumes will be site related) and thus minimal impacts to other road users will result. This event is limited to a single day also.

In considering MTO warrants for left and right turn lanes, the associated event traffic volumes will exceed the thresholds for EB left turns at the Line 7 access; and EB left and WB right turns at the Line 8 access points.

- Notwithstanding, given the limited nature of the events and traffic otherwise on Line 8, no additional turn lanes are considered necessary. While the associated turn volumes may exceed the threshold limits, impacts to other road users will be limited (ie. it will primarily be site traffic on the road).
- At the Line 7 access, an EB left turn lane is recommended in consideration of the projected traffic volumes and more regular use of the Line 7 access for site events. Given the location of the road within the existing right-of-way, and presence of large mature trees on the south side of the road, the left turn lane is recommended to be constructed on the north side of the road. Prior to implementation, the need for the turn lane should be confirmed based on actual event and Line 7 traffic volumes during the course of the event (as suggested in the monitoring program).


## Monitoring

The future traffic volumes projected under each event scenario have been established based on limited traffic volumes through the area and in consideration of anticipated event attendance and travel patterns. As the development proceeds, and events occur, it is recommended that traffic volumes and conditions on the area roads and at the site access points be counted and monitored during the initial year of operations to establish actual traffic levels and corresponding peak durations.

Based on the first year of operations, the need for and timing of, external road improvements can be confirmed (for implementation in the following year).

Temporary traffic control measures can also be considered as warranted (eg. traffic control persons at the site access points) during peak events to provide additional guidance and control to site visitors and those on the road system.

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Boots \& Hearts Arrivals (On-Site Camping and Day Parking)













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Burl's Creek Event Grounds Traffic Impact Study
Figure


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APPENDIX A: TRAFFIC COUNTS




| Highway | Location Description | Dist | Year | Patt <br> Type | AADT | SADT | SAWDT | WADT | AR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | 1999 | IR | 31,100 | 38,300 | 33,900 | 27,200 | 0.6 |
|  |  |  | 2000 | IR | 32,000 | 38,700 | 34,400 | 28,000 | 0.6 |
|  |  |  | 2001 | IR | 32,900 | 40,100 | 35,200 | 28,600 | 0.5 |
|  |  |  | 2002 | IR | 33,800 | 41,300 | 36,300 | 29,600 | 0.9 |
|  |  |  | 2003 | IR | 34,700 | 42,300 | 37,100 | 30,500 | 0.6 |
|  |  |  | 2004 | IR | 35,600 | 42,600 | 37,900 | 31,300 | 0.6 |
|  |  |  | 2005 | IR | 36,500 | 43,500 | 38,700 | 31,900 | 0.5 |
|  |  |  | 2006 | IT | 37,900 | 48,200 | 47,900 | 31,100 | 0.5 |
|  |  |  | 2007 | IT | 38,400 | 49,200 | 51,400 | 31,400 | 0.6 |
|  |  |  | 2008 | IT | 39,300 | 50,500 | 47,800 | 33,000 | 0.4 |
|  |  |  | 2009 | IR | 40,200 | 45,700 | 44,200 | 36,500 | 0.3 |
|  |  |  | 2010 | IR | 41,100 | 46,600 | 45,100 | 37,300 | 0.5 |
| 11 | SIMCOE RD 20-ORO MEDONTE LINE 11 IC 121 | 7.8 | 1988 | IR | 22,400 | 29,100 | 22,600 | 17,900 | 0.7 |
|  |  |  | 1989 | IR | 24,300 | 31,300 | 26,900 | 19,900 | 0.7 |
|  |  |  | 1990 | IR | 26,250 | 33,300 | 29,100 | 21,500 | 0.8 |
|  |  |  | 1991 | IR | 26,500 | 33,300 | 29,400 | 21,900 | 0.6 |
|  |  |  | 1992 | IR | 26,800 | 32,900 | 29,400 | 22,700 | 0.8 |
|  |  |  | 1993 | IR | 28,000 | 34,400 | 29,100 | 23,500 | 0.6 |
|  |  |  | 1994 | IR | 28,900 | 36,100 | 30,900 | 24,600 | 0.4 |
|  |  |  | 1995 | IR | 29,400 | 36,500 | 31,500 | 24,700 | 0.5 |
|  |  |  | 1996 | IR | 30,000 | 39,300 | 34,800 | 24,200 | 0.7 |
|  |  |  | 1997 | IR | 30,500 | 40,000 | 35,400 | 25,000 | 0.5 |
|  |  |  | 1998 | IR | 31,000 | 40,000 | 35,600 | 24,800 | 0.5 |
|  |  |  | 1999 | IR | 31,500 | 40,600 | 36,100 | 25,200 | 0.5 |
|  |  |  | 2000 | IR | 33,500 | 42,900 | 38,400 | 27,000 | 0.6 |
|  |  |  | 2001 | IR | 34,300 | 44,200 | 39,400 | 27,800 | 0.4 |
|  |  |  | 2002 | IR | 35,200 | 45,400 | 40,300 | 28,700 | 0.5 |
|  |  |  | 2003 | IR | 36,100 | 46,900 | 41,500 | 29,200 | 0.6 |
|  |  |  | 2004 | IR | 37,300 | 47,200 | 42,300 | 30,200 | 0.2 |
|  |  |  | 2005 | IR | 37,600 | 47,300 | 42,400 | 30,800 | 0.5 |
|  |  |  | 2006 | IR | 37,800 | 45,600 | 41,300 | 32,100 | 0.4 |
|  |  |  | 2007 | IR | 38,600 | 46,800 | 46,600 | 32,700 | 0.4 |
|  |  |  | 2008 | IR | 40,200 | 48,700 | 47,800 | 34,400 | 0.4 |
|  |  |  | 2009 | IR | 40,100 | 47,300 | 42,200 | 35,200 | 0.3 |
|  |  |  | 2010 | IC | 39,000 | 43,200 | 43,500 | 34,600 | 0.5 |
| 11 | MEMORIAL AVE IC | 1.7 | 1988 | IR | 14,300 | 18,500 | 14,400 | 11,400 | 0.9 |
|  |  |  | 1989 | IR | 16,300 | 21,000 | 18,000 | 13,300 | 1.0 |
|  |  |  | 1990 | LR | 18,350 | 32,100 | 25,500 | 11,500 | 1.1 |
|  |  |  | 1991 | LR | 19,600 | 34,400 | 27,200 | 12,300 | 0.7 |
|  |  |  | 1992 | LR | 20,050 | 32,200 | 25,800 | 13,200 | 1.0 |
|  |  |  | 1993 | LR | 22,250 | 32,200 | 26,200 | 15,500 | 0.6 |
|  |  |  | 1994 | LR | 23,100 | 33,500 | 27,500 | 16,700 | 0.5 |
|  |  |  | 1995 | LR | 24,400 | 36,600 | 29,500 | 17,600 | 0.5 |
|  |  |  | 1996\| | LR | 25,700 | 38,600 | 31,100 | 18,600 | 0.6 |

County Road 20 - Spring 2014 Line 2 Oro-Medonte to ine 7 Oro-Medonte

Transportation and Engineering Department
Midhurst, Ontario
(705)-726-9300

Site Code: 02002

Line 2 Oro-Medonte Line 7 Oro-Medonte

| Start | 19-May-14 |  | Tue |  | Wed |  | Thu |  | Fri |  | Sat |  | Sun |  | Week Average |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB |
| 12:00 AM | * | * | 1 | 6 | 1 | 1 | 2 | 1 | * | * | * | * | * | * | 1 | 3 |
| 01:00 | * | * | 3 | 0 | 4 | 1 | 2 | 1 | * | * | * | * | * | * | 3 | 1 |
| 02:00 | * | * | 2 | 0 | 4 | 1 | 1 | 1 | * | * | * | * | * | * | 2 | 1 |
| 03:00 | * | * | 1 | 0 | 0 | 0 | 0 | 0 | * | * | * | * | * | * | 0 | 0 |
| 04:00 | * | * | 1 | 1 | 0 | 3 | 0 | 2 | * | * | * | * | * | * | 0 | 2 |
| 05:00 | * | * | 0 | 5 | 2 | 4 | 2 | 6 | * | * | * | * | * | * | 1 | 5 |
| 06:00 | * | * | 6 | 17 | 6 | 26 | 3 | 24 | * | * | * | * | * | * | 5 | 22 |
| 07:00 | * | * | 17 | 73 | 15 | 73 | 16 | 73 | * | * | * | * | * | * | 16 | 73 |
| 08:00 | * | * | 24 | 71 | 35 | 71 | 34 | 66 | * | * | * | * | * | * | 31 | 69 |
| 09:00 | * | * | 28 | 54 | 32 | 74 | 36 | 69 | * | * | * | * | * | * | 32 | 66 |
| 10:00 | * | * | 33 | 49 | 28 | 32 | 35 | 53 | * | * | * | * | * | * | 32 | 45 |
| 11:00 | * | * | 43 | 45 | 44 | 40 | 44 | 48 | * | * | * | * | * | * | 44 | 44 |
| 12:00 PM | * | * | 56 | 54 | 57 | 54 | 54 | 54 | * | * | * | * | * | * | 56 | 54 |
| 01:00 | * | * | 50 | 53 | 50 | 49 | 51 | 59 | * | * | * | * | * | * | 50 | 54 |
| 02:00 | * | * | 66 | 46 | 50 | 53 | 50 | 57 | * | * | * | * | * | * | 55 | 52 |
| 03:00 | * | * | 55 | 46 | 52 | 50 | 62 | 33 | * | * | * | * | * | * | 56 | 43 |
| 04:00 | * | * | 79 | 61 | 72 | 63 | 69 | 61 | * | * | * | * | * | * | 73 | 62 |
| 05:00 | * | * | 80 | 46 | 88 | 50 | 100 | 55 | * | * | * | * | * | * | 89 | 50 |
| 06:00 | * | * | 71 | 46 | 106 | 47 | 84 | 68 | * | * | * | * | * | * | 87 | 54 |
| 07:00 | * | * | 58 | 33 | 57 | 34 | 81 | 45 | * | * | * | * | * | * | 65 | 37 |
| 08:00 | * | * | 44 | 34 | 54 | 113 | 67 | 36 | * | * | * | * | * | * | 55 | 61 |
| 09:00 | * | * | 16 | 11 | 32 | 26 | 22 | 20 | * | * | * | * | * | * | 23 | 19 |
| 10:00 | * | * | 17 | 7 | 19 | 10 | 19 | 9 | * | * | * | * | * | * | 18 | 9 |
| 11:00 | * | * | 12 | 2 | 8 | 4 | 9 | 7 | * | * | * | * | * | * | 10 | 4 |
| Lane | 0 | 0 | 763 | 760 | 816 | 879 | 843 | 848 | 0 | 0 | 0 | 0 | 0 | 0 | 804 | 830 |
| Day |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| AM Peak | - | - | 11:00 | 07:00 | 11:00 | 09:00 | 11:00 | 07:00 | - | - | - | - | - | - | 11:00 | 07:00 |
| Vol. | - | - | 43 | 73 | 44 | 74 | 44 | 73 | - | - | - | - | - | - | 44 | 73 |
| PM Peak | - | - | 17:00 | 16:00 | 18:00 | 20:00 | 17:00 | 18:00 | - | - | - | - | - | - | 17:00 | 16:00 |
| Vol. | - | - | 80 | 61 | 106 | 113 | 100 | 68 | - | - | - | - | - | - | 89 | 62 |

County Road 20 - Summer 2014 Line 3 Oro-Medonte to ine 7 Oro-Medonte

Transportation and Engineering Department
Midhurst, Ontario
(705)-726-9300

| Start | 11-Aug-14 |  | Tue |  | Wed |  | Thu |  | Fri |  | Sat |  | Sun |  | Week Average |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB |
| 12:00 AM | 4 | 5 | 3 | 5 | 6 | 4 | 3 | 1 | * | * | * | * | * | * | 4 | 4 |
| 01:00 | 2 | 0 | 3 | 1 | 4 | 3 | 5 | 1 | * | * | * | * | * | * | 4 | 1 |
| 02:00 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | * | * | * | * | * | * | 2 | 2 |
| 03:00 | 4 | 1 | 3 | 1 | 2 | 0 | 4 | 1 | * | * | * | * | * | * | 3 | 1 |
| 04:00 | 1 | 3 | 0 | 1 | 0 | 2 | 0 | 0 | * | * | * | * | * | * | 0 | 2 |
| 05:00 | 1 | 3 | 1 | 3 | 2 | 7 | 1 | 7 | * | * | * | * | * | * | 1 | 5 |
| 06:00 | 5 | 12 | 3 | 13 | 5 | 20 | 4 | 15 | * | * | * | * | * | * | 4 | 15 |
| 07:00 | 14 | 31 | 7 | 47 | 8 | 39 | 14 | 43 | * | * | * | * | * | * | 11 | 40 |
| 08:00 | 18 | 58 | 27 | 69 | 24 | 70 | 20 | 71 | * | * | * | * | * | * | 22 | 67 |
| 09:00 | 25 | 50 | 24 | 38 | 33 | 55 | 34 | 38 | * | * | * | * | * | * | 29 | 45 |
| 10:00 | 33 | 51 | 29 | 42 | 36 | 42 | 35 | 41 | * | * | * | * | * | * | 33 | 44 |
| 11:00 | 46 | 55 | 44 | 46 | 39 | 47 | 30 | 46 | * | * | * | * | * | * | 40 | 48 |
| 12:00 PM | 48 | 65 | 43 | 59 | 41 | 52 | 48 | 49 | * | * | * | * | * | * | 45 | 56 |
| 01:00 | 54 | 57 | 58 | 62 | 45 | 48 | 45 | 52 | * | * | * | * | * | * | 50 | 55 |
| 02:00 | 85 | 53 | 36 | 48 | 50 | 49 | 52 | 34 | * | * | * | * | * | * | 56 | 46 |
| 03:00 | 43 | 55 | 54 | 57 | 51 | 36 | 52 | 43 | * | * | * | * | * | * | 50 | 48 |
| 04:00 | 68 | 55 | 75 | 46 | 66 | 38 | 68 | 50 | * | * | * | * | * | * | 69 | 47 |
| 05:00 | 93 | 59 | 124 | 53 | 78 | 55 | 96 | 45 | * | * | * | * | * | * | 98 | 53 |
| 06:00 | 54 | 43 | 73 | 43 | 60 | 40 | 80 | 50 | * | * | * | * | * | * | 67 | 44 |
| 07:00 | 50 | 37 | 46 | 30 | 35 | 27 | 47 | 23 | * | * | * | * | * | * | 44 | 29 |
| 08:00 | 37 | 31 | 28 | 26 | 40 | 10 | 27 | 16 | * | * | * | * | * | * | 33 | 21 |
| 09:00 | 20 | 19 | 32 | 21 | 17 | 15 | 32 | 13 | * | * | * | * | * | * | 25 | 17 |
| 10:00 | 16 | 11 | 21 | 24 | 18 | 11 | 15 | 10 | * | * | * | * | * | * | 18 | 14 |
| 11:00 | 12 | 6 | 5 | 6 | 12 | 6 | 10 | 9 | * | * | * | * | * | * | 10 | 7 |
| Lane | 735 | 762 | 741 | 743 | 674 | 678 | 724 | 660 | 0 | 0 | 0 | 0 | 0 | 0 | 718 | 711 |
| Day |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| AM Peak | 11:00 | 08:00 | 11:00 | 08:00 | 11:00 | 08:00 | 10:00 | 08:00 | - | - | - | - | - | - | 11:00 | 08:00 |
| Vol. | 46 | 58 | 44 | 69 | 39 | 70 | 35 | 71 | - | - | - | - | - | - | 40 | 67 |
| PM Peak | 17:00 | 12:00 | 17:00 | 13:00 | 17:00 | 17:00 | 17:00 | 13:00 | - | - | - | - | - | - | 17:00 | 12:00 |
| Vol. | 93 | 65 | 124 | 62 | 78 | 55 | 96 | 52 | - | - | - | - | - | - | 98 | 56 |

Transporation and Engineering Department
Midhurst, Ontario
(705) 726-9300

Site Code: 02002
Date Start: 14-Oct-14 Date End: 16-Oct-14

| Start | 13-Oct-14 |  |  | Tue |  | Wed |  | Thu |  | Fri |  | Sat |  | Sun |  | Week Average |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | EB |  | WB | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB |
| 12:00 AM |  | * | * | 3 | 1 | 3 | 1 | 6 | 1 | * | * | * | * | * | * | 4 | 1 |
| 01:00 |  | * | * | 3 | 1 | 1 | 0 | 1 | 1 | * | * | * | * | * | * | 2 | 1 |
| 02:00 |  | * | * | 6 | 2 | 3 | 1 | 2 | 1 | * | * | * | * | * | * | 4 | 1 |
| 03:00 |  | * | * | 0 | 1 | 0 | 1 | 0 | 0 | * | * | * | * | * | * | 0 | 1 |
| 04:00 |  | * | * | 1 | 4 | 2 | 2 | 1 | 2 | * | * | * | * | * | * | 1 | 3 |
| 05:00 |  | * | * | 2 | 7 | 2 | 4 | 0 | 5 | * | * | * | * | * | * | 1 | 5 |
| 06:00 |  | * | * | 4 | 30 | 3 | 32 | 3 | 29 | * | * | * | * | * | * | 3 | 30 |
| 07:00 |  | * | * | 15 | 84 | 9 | 85 | 13 | 85 | * | * | * | * | * | * | 12 | 85 |
| 08:00 |  | * | * | 27 | 61 | 34 | 72 | 27 | 61 | * | * | * | * | * | * | 29 | 65 |
| 09:00 |  | * | * | 38 | 45 | 21 | 51 | 30 | 53 | * | * | * | * | * | * | 30 | 50 |
| 10:00 |  | * | * | 28 | 42 | 27 | 42 | 31 | 41 | * | * | * | * | * | * | 29 | 42 |
| 11:00 |  | * | * | 61 | 44 | 40 | 38 | 36 | 44 | * | * | * | * | * | * | 46 | 42 |
| 12:00 PM |  | * | * | 72 | 56 | 53 | 48 | 46 | 34 | * | * | * | * | * | * | 57 | 46 |
| 01:00 |  | * | * | 45 | 62 | 52 | 40 | 36 | 38 | * | * | * | * | * | * | 44 | 47 |
| 02:00 |  | * | * | 71 | 66 | 48 | 40 | 42 | 35 | * | * | * | * | * | * | 54 | 47 |
| 03:00 |  | * | * | 67 | 58 | 66 | 34 | 73 | 51 | * | * | * | * | * | * | 69 | 48 |
| 04:00 |  | * | * | 95 | 55 | 80 | 51 | 98 | 48 | * | * | * | * | * | * | 91 | 51 |
| 05:00 |  | * | * | 92 | 51 | 86 | 35 | 77 | 49 | * | * | * | * | * | * | 85 | 45 |
| 06:00 |  | * | * | 45 | 27 | 54 | 40 | 62 | 52 | * | * | * | * | * | * | 54 | 40 |
| 07:00 |  | * | * | 26 | 18 | 40 | 16 | 46 | 29 | * | * | * | * | * | * | 37 | 21 |
| 08:00 |  | * | * | 25 | 12 | 30 | 28 | 19 | 13 | * | * | * | * | * | * | 25 | 18 |
| 09:00 |  | * | * | 20 | 10 | 11 | 18 | 23 | 17 | * | * | * | * | * | * | 18 | 15 |
| 10:00 |  | * | * | 7 | 8 | 10 | 8 | 12 | 5 | * | * | * | * | * | * | 10 | 7 |
| 11:00 |  | * | * | 4 | 3 | 5 | 5 | 5 | 2 | * | * | * | * | * | * | 5 | 3 |
| Lane | 0 | 0 | 0 | 757 | 748 | 680 | 692 | 689 | 696 | 0 | 0 | 0 | 0 | 0 | 0 | 710 | 714 |
| Day |  | 0 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| AM Peak |  | - | - | 11:00 | 07:00 | 11:00 | 07:00 | 11:00 | 07:00 | - | - | - | - | - | - | 11:00 | 07:00 |
| Vol. |  | - | - | 61 | 84 | 40 | 85 | 36 | 85 | - | - | - | - | - | - | 46 | 85 |
| PM Peak |  | - | - | 16:00 | 14:00 | 17:00 | 16:00 | 16:00 | 18:00 | - | - | - | - | - | - | 16:00 | 16:00 |
| Vol. |  | - | - | 95 | 66 | 86 | 51 | 98 | 52 | - | - | - | - | - | - | 91 | 51 |

County Road 20 - Spring 2014 Line 7 Oro-Medonte to Line 11 Oro-Medonte

Transportation and Engineering Department
Midhurst, Ontario
(705)-726-9300

Site Code: 02003

Line 7 Oro-Medonte Line 11 Oro-Medonte


Comb
Tota
ADT 1,090

1135
1124
0
0
1088

County Road 20 - Summer 2014 Line 7 Oro-Medonte to Line 11 Oro-Medonte

Transportation and Engineering Department
Midhurst, Ontario
(705)-726-9300

| Start | 11-Aug-14 |  | Tue |  | Wed |  | Thu |  | Fri |  | Sat |  | Sun |  | Week Average |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB |
| 12:00 AM | 4 | 2 | 0 | 3 | 2 | 1 | 8 | 3 | * | * | * | * | * | * | 4 | 2 |
| 01:00 | 0 | 0 | 1 | 1 | 1 | 0 | 1 | 1 | * | * | * | * | * | * | 1 | 0 |
| 02:00 | 0 | 0 | 1 | 0 | 3 | 2 | 1 | 1 | * | * | * | * | * | * | 1 | 1 |
| 03:00 | 3 | 2 | 1 | 1 | 0 | 1 | 2 | 1 | * | * | * | * | * | * | 2 | 1 |
| 04:00 | 2 | 0 | 2 | 0 | 3 | 1 | 2 | 2 | * | * | * | * | * | * | 2 | 1 |
| 05:00 | 12 | 0 | 10 | 0 | 12 | 1 | 11 | 0 | * | * | * | * | * | * | 11 | 0 |
| 06:00 | 21 | 3 | 21 | 1 | 26 | 1 | 20 | 2 | * | * | * | * | * | * | 22 | 2 |
| 07:00 | 31 | 11 | 45 | 12 | 33 | 9 | 39 | 9 | * | * | * | * | * | * | 37 | 10 |
| 08:00 | 50 | 16 | 51 | 24 | 53 | 30 | 52 | 22 | * | * | * | * | * | * | 52 | 23 |
| 09:00 | 47 | 23 | 38 | 26 | 47 | 25 | 50 | 39 | * | * | * | * | * | * | 46 | 28 |
| 10:00 | 25 | 27 | 32 | 15 | 39 | 28 | 50 | 26 | * | * | * | * | * | * | 36 | 24 |
| 11:00 | 31 | 34 | 29 | 26 | 21 | 21 | 31 | 26 | * | * | * | * | * | * | 28 | 27 |
| 12:00 PM | 57 | 33 | 31 | 34 | 32 | 30 | 40 | 32 | * | * | * | * | * | * | 40 | 32 |
| 01:00 | 38 | 27 | 55 | 31 | 48 | 27 | 44 | 34 | * | * | * | * | * | * | 46 | 30 |
| 02:00 | 42 | 39 | 41 | 30 | 36 | 28 | 26 | 33 | * | * | * | * | * | * | 36 | 32 |
| 03:00 | 43 | 38 | 34 | 20 | 29 | 33 | 38 | 38 | * | * | * | * | * | * | 36 | 32 |
| 04:00 | 52 | 31 | 46 | 49 | 41 | 30 | 32 | 36 | * | * | * | * | * | * | 43 | 36 |
| 05:00 | 53 | 51 | 44 | 43 | 64 | 45 | 41 | 38 | * | * | * | * | * | * | 50 | 44 |
| 06:00 | 44 | 45 | 35 | 41 | 37 | 24 | 39 | 54 | * | * | * | * | * | * | 39 | 41 |
| 07:00 | 43 | 26 | 23 | 23 | 30 | 21 | 31 | 29 | * | * | * | * | * | * | 32 | 25 |
| 08:00 | 39 | 26 | 18 | 13 | 18 | 14 | 16 | 18 | * | * | * | * | * | * | 23 | 18 |
| 09:00 | 22 | 12 | 14 | 5 | 12 | 10 | 13 | 9 | * | * | * | * | * | * | 15 | 9 |
| 10:00 | 9 | 7 | 12 | 11 | 11 | 7 | 10 | 5 | * | * | * | * | * | * | 10 | 8 |
| 11:00 | 3 | 1 | 1 | 6 | 2 | 7 | 4 | 11 | * | * | * | * | * | * | 2 | 6 |
| Lane | 671 | 454 | 585 | 415 | 600 | 396 | 601 | 469 | 0 | 0 | 0 | 0 | 0 | 0 | 614 | 432 |
| Day |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| AM Peak | 08:00 | 11:00 | 08:00 | 09:00 | 08:00 | 08:00 | 08:00 | 09:00 | - | - | - | - | - | - | 08:00 | 09:00 |
| Vol. | 50 | 34 | 51 | 26 | 53 | 30 | 52 | 39 | - | - | - | - | - | - | 52 | 28 |
| PM Peak | 12:00 | 17:00 | 13:00 | 16:00 | 17:00 | 17:00 | 13:00 | 18:00 | - | - | - | - | - | - | 17:00 | 17:00 |
| Vol. | 57 | 51 | 55 | 49 | 64 | 45 | 44 | 54 | - | - | - | - | - | - | 50 | 44 |

Comb
Tota
1125

ADT 1,048

Transporation and Engineering Department
Midhurst, Ontario
(705) 726-9300

Site Code: 02003
Date Start: 14-Oct-14 Date End: 16-Oct-14

| Start | 13-Oct-14 |  | Tue |  | Wed |  | Thu |  | Fri |  | Sat |  | Sun |  | Week Average |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB | EB | WB |
| 12:00 AM | * | * | 1 | 3 | 1 | 0 | 0 | 0 | * | * | * | * | * | * | 1 | 1 |
| 01:00 | * | * | 1 | 5 | 1 | 4 | 0 | 2 | * | * | * | * | * | * | 1 | 4 |
| 02:00 | * | * | 1 | 0 | 0 | 1 | 0 | 0 | * | * | * | * | * | * | 0 | 0 |
| 03:00 | * | * | 1 | 2 | 1 | 2 | 3 | 3 | * | * | * | * | * | * | 2 | 2 |
| 04:00 | * | * | 0 | 6 | 0 | 14 | 0 | 10 | * | * | * | * | * | * | 0 | 10 |
| 05:00 | * | * | 0 | 23 | 1 | 17 | 2 | 19 | * | * | * | * | * | * | 1 | 20 |
| 06:00 | * | * | 2 | 34 | 2 | 40 | 2 | 40 | * | * | * | * | * | * | 2 | 38 |
| 07:00 | * | * | 19 | 44 | 13 | 46 | 19 | 57 | * | * | * | * | * | * | 17 | 49 |
| 08:00 | * | * | 15 | 40 | 19 | 46 | 22 | 57 | * | * | * | * | * | * | 19 | 48 |
| 09:00 | * | * | 25 | 31 | 19 | 28 | 16 | 34 | * | * | * | * | * | * | 20 | 31 |
| 10:00 | * | * | 31 | 29 | 24 | 20 | 29 | 34 | * | * | * | * | * | * | 28 | 28 |
| 11:00 | * | * | 40 | 37 | 24 | 42 | 27 | 48 | * | * | * | * | * | * | 30 | 42 |
| 12:00 PM | * | * | 31 | 33 | 26 | 35 | 27 | 21 | * | * | * | * | * | * | 28 | 30 |
| 01:00 | * | * | 28 | 43 | 27 | 32 | 29 | 31 | * | * | * | * | * | * | 28 | 35 |
| 02:00 | * | * | 28 | 54 | 28 | 26 | 21 | 41 | * | * | * | * | * | * | 26 | 40 |
| 03:00 | * | * | 30 | 34 | 28 | 38 | 47 | 36 | * | * | * | * | * | * | 35 | 36 |
| 04:00 | * | * | 35 | 37 | 42 | 31 | 40 | 49 | * | * | * | * | * | * | 39 | 39 |
| 05:00 | * | * | 45 | 43 | 33 | 35 | 41 | 46 | * | * | * | * | * | * | 40 | 41 |
| 06:00 | * | * | 29 | 34 | 33 | 42 | 30 | 35 | * | * | * | * | * | * | 31 | 37 |
| 07:00 | * | * | 14 | 20 | 16 | 21 | 15 | 28 | * | * | * | * | * | * | 15 | 23 |
| 08:00 | * | * | 9 | 12 | 6 | 20 | 17 | 10 | * | * | * | * | * | * | 11 | 14 |
| 09:00 | * | * | 9 | 13 | 11 | 5 | 4 | 10 | * | * | * | * | * | * | 8 | 9 |
| 10:00 | * | * | 2 | 6 | 2 | 6 | 6 | 12 | * | * | * | * | * | * | 3 | 8 |
| 11:00 | * | * | 4 | 1 | 5 | 4 | 2 | 3 | * | * | * | * | * | * | 4 | 3 |
| Lane | 0 | 0 | 400 | 584 | 362 | 555 | 399 | 626 | 0 | 0 | 0 | 0 | 0 | 0 | 389 | 588 |
| Day |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| AM Peak | - | - | 11:00 | 07:00 | 10:00 | 07:00 | 10:00 | 07:00 | - | - | - | - | - | - | 11:00 | 07:00 |
| Vol. | - | - | 40 | 44 | 24 | 46 | 29 | 57 | - | - | - | - | - | - | 30 | 49 |
| PM Peak | - | - | 17:00 | 14:00 | 16:00 | 18:00 | 15:00 | 16:00 | - | - | - | - | - | - | 17:00 | 17:00 |
| Vol. | - | - | 45 | 54 | 42 | 42 | 47 | 49 | - | - | - | - | - | - | 40 | 41 |

County Road 20 - Spring 2014 Line 11 Oro-Medonte to Line 11 / Highway 11

Transportation and Engineering Department
Midhurst, Ontario
(705)-726-9300

Site Code: 02004

Line 11 Oro-Medonte Line 11 / Highway 11

| Start | 19-May-14 |  |  | Tue |  |  | Wed |  | Thu |  | Fri |  | Sat |  | Sun |  | Week Average |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | SB |  | NB | SB |  | NB | SB | NB | SB | NB | SB | NB | SB | NB | SB | NB | SB | NB |
| 12:00 AM |  | * | * |  | * | * | 1 | 10 | 3 | 9 | * | * | * | * | * | * | 2 | 10 |
| 01:00 |  | * | * |  | * | * | 1 | 3 | 0 | 1 | * | * | * | * | * | * | 0 | 2 |
| 02:00 |  | * | * |  | * | * | 1 | 5 | 0 | 3 | * | * | * | * | * | * | 0 | 4 |
| 03:00 |  | * | * |  | * | * | 2 | 3 | 2 | 2 | * | * | * | * | * | * | 2 | 2 |
| 04:00 |  | * | * |  | * | * | 7 | 1 | 6 | 2 | * | * | * | * | * | * | 6 | 2 |
| 05:00 |  | * | * |  | * | * | 19 | 1 | 19 | 0 | * | * | * | * | * | * | 19 | 0 |
| 06:00 |  | * | * |  | * | * | 41 | 3 | 46 | 5 | * | * | * | * | * | * | 44 | 4 |
| 07:00 |  | * | * |  | * | * | 63 | 26 | 64 | 24 | * | * | * | * | * | * | 64 | 25 |
| 08:00 |  | * | * |  | * | * | 92 | 49 | 89 | 43 | * | * | * | * | * | * | 90 | 46 |
| 09:00 |  | * | * |  | * | * | 94 | 37 | 78 | 40 | * | * | * | * | * | * | 86 | 38 |
| 10:00 |  | * | * |  | * | * | 43 | 51 | 52 | 47 | * | * | * | * | * | * | 48 | 49 |
| 11:00 |  | * | * |  | * | * | 46 | 54 | 37 | 47 | * | * | * | * | * | * | 42 | 50 |
| 12:00 PM |  | * | * |  | * | * | 52 | 54 | 50 | 62 | * | * | * | * | * | * | 51 | 58 |
| 01:00 |  | * | * |  | * | * | 56 | 88 | 51 | 55 | * | * | * | * | * | * | 54 | 72 |
| 02:00 |  | * | * |  | * | * | 46 | 72 | 48 | 55 | * | * | * | * | * | * | 47 | 64 |
| 03:00 |  | * | * |  | * | * | 40 | 79 | 46 | 77 | * | * | * | * | * | * | 43 | 78 |
| 04:00 |  | * | * |  | * | * | 54 | 119 | 58 | 115 | * | * | * | * | * | * | 56 | 117 |
| 05:00 |  | * | * |  | * | * | 64 | 164 | 56 | 156 | * | * | * | * | * | * | 60 | 160 |
| 06:00 |  | * | * |  | * | * | 54 | 108 | 49 | 117 | * | * | * | * | * | * | 52 | 112 |
| 07:00 |  | * | * |  | * | * | 50 | 65 | 39 | 88 | * | * | * | * | * | * | 44 | 76 |
| 08:00 |  | * | * |  | * | * | 28 | 66 | 27 | 57 | * | * | * | * | * | * | 28 | 62 |
| 09:00 |  | * | * |  | * | * | 18 | 39 | 13 | 50 | * | * | * | * | * | * | 16 | 44 |
| 10:00 |  | * | * |  | * | * | 11 | 41 | 22 | 42 | * | * | * | * | * | * | 16 | 42 |
| 11:00 |  | * | * |  | * | * | 1 | 17 | 3 | 12 | * | * | * | * | * | * | 2 | 14 |
| Lane |  | 0 | 0 |  | 0 | 0 | 884 | 1155 | 858 | 1109 | 0 | 0 | 0 | 0 | 0 | 0 | 872 | 1131 |
| Day |  | 0 |  |  | 0 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| AM Peak |  | - | - |  | - | - | 09:00 | 11:00 | 08:00 | 10:00 | - | - | - | - | - | - | 08:00 | 11:00 |
| Vol. |  | - | - |  | - | - | 94 | 54 | 89 | 47 | - | - | - | - | - | - | 90 | 50 |
| PM Peak |  | - | - |  | - | - | 17:00 | 17:00 | 16:00 | 17:00 | - | - | - | - | - | - | 17:00 | 17:00 |
| Vol. |  | - | - |  | - | - | 64 | 164 | 58 | 156 | - | - | - | - | - | - | 60 | 160 |

County Road 20 - Summer 2014 ine 11 Oro-Medonte to Line 11/ Highway 11

Transportation and Engineering Department
Midhurst, Ontario
(705)-726-9300

| Start | 11-Aug-14 |  | Tue |  | Wed |  | Thu |  | Fri |  | Sat |  | Sun |  | Week Average |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | SB | NB | SB | NB | SB | NB | SB | NB | SB | NB | SB | NB | SB | NB | SB | NB |
| 12:00 AM | 14 | 3 | 4 | 1 | 4 | 1 | 10 | 9 | * | * | * | * | * | * | 8 | 4 |
| 01:00 | 4 | 4 | 6 | 0 | 2 | 0 | 4 | 1 | * | * | * | * | * | * | 4 | 1 |
| 02:00 | 2 | 0 | 2 | 0 | 4 | 1 | 1 | 1 | * | * | * | * | * | * | 2 | 0 |
| 03:00 | 2 | 2 | 2 | 1 | 1 | 1 | 4 | 2 | * | * | * | * | * | * | 2 | 2 |
| 04:00 | 1 | 11 | 3 | 6 | 2 | 6 | 1 | 11 | * | * | * | * | * | * | 2 | 8 |
| 05:00 | 2 | 16 | 1 | 22 | 0 | 24 | 2 | 20 | * | * | * | * | * | * | 1 | 20 |
| 06:00 | 4 | 18 | 2 | 22 | 5 | 17 | 2 | 25 | * | * | * | * | * | * | 3 | 20 |
| 07:00 | 11 | 48 | 18 | 36 | 13 | 37 | 16 | 41 | * | * | * | * | * | * | 14 | 40 |
| 08:00 | 42 | 94 | 24 | 87 | 41 | 95 | 26 | 90 | * | * | * | * | * | * | 33 | 92 |
| 09:00 | 39 | 55 | 38 | 60 | 34 | 69 | 35 | 70 | * | * | * | * | * | * | 36 | 64 |
| 10:00 | 45 | 42 | 35 | 59 | 33 | 48 | 48 | 54 | * | * | * | * | * | * | 40 | 51 |
| 11:00 | 43 | 61 | 61 | 50 | 34 | 54 | 42 | 45 | * | * | * | * | * | * | 45 | 52 |
| 12:00 PM | 60 | 56 | 43 | 50 | 51 | 40 | 61 | 46 | * | * | * | * | * | * | 54 | 48 |
| 01:00 | 71 | 59 | 64 | 52 | 64 | 34 | 58 | 48 | * | * | * | * | * | * | 64 | 48 |
| 02:00 | 71 | 40 | 56 | 37 | 62 | 42 | 46 | 40 | * | * | * | * | * | * | 59 | 40 |
| 03:00 | 74 | 50 | 67 | 39 | 81 | 50 | 59 | 41 | * | * | * | * | * | * | 70 | 45 |
| 04:00 | 103 | 51 | 95 | 50 | 95 | 49 | 102 | 58 | * | * | * | * | * | * | 99 | 52 |
| 05:00 | 130 | 68 | 129 | 73 | 144 | 57 | 123 | 60 | * | * | * | * | * | * | 132 | 64 |
| 06:00 | 93 | 56 | 98 | 44 | 101 | 37 | 122 | 50 | * | * | * | * | * | * | 104 | 47 |
| 07:00 | 69 | 40 | 62 | 24 | 50 | 28 | 62 | 40 | * | * | * | * | * | * | 61 | 33 |
| 08:00 | 58 | 28 | 47 | 28 | 47 | 28 | 53 | 23 | * | * | * | * | * | * | 51 | 27 |
| 09:00 | 44 | 14 | 35 | 14 | 44 | 11 | 46 | 20 | * | * | * | * | * | * | 42 | 15 |
| 10:00 | 34 | 10 | 30 | 3 | 28 | 9 | 32 | 6 | * | * | * | * | * | * | 31 | 7 |
| 11:00 | 20 | 9 | 12 | 1 | 18 | 10 | 20 | 5 | * | * | * | * | * | * | 18 | 6 |
| Lane | 1036 | 835 | 934 | 759 | 958 | 748 | 975 | 806 | 0 | 0 | 0 | 0 | 0 | 0 | 975 | 786 |
| Day |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| AM Peak | 10:00 | 08:00 | 11:00 | 08:00 | 08:00 | 08:00 | 10:00 | 08:00 | - | - | - | - | - | - | 11:00 | 08:00 |
| Vol. | 45 | 94 | 61 | 87 | 41 | 95 | 48 | 90 | - | - | - | - | - | - | 45 | 92 |
| PM Peak | 17:00 | 17:00 | 17:00 | 17:00 | 17:00 | 17:00 | 17:00 | 17:00 | - | - | - | - | - | - | 17:00 | 17:00 |
| Vol. | 130 | 68 | 129 | 73 | 144 | 57 | 123 | 60 | - | - | - | - | - | - | 132 | 64 |

County Road 20 - Fall 2014 Line 11 - Oro Medonte to Highway 11

Transporation and Engineering Department
Midhurst, Ontario
(705) 726-9300

Site Code: 02004
Date Start: 14-Oct-14
Date End: 16-Oct-14

| Start | 13-Oct-14 |  | Tue |  | Wed |  | Thu |  | Fri |  | Sat |  | Sun |  | Week Average |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | SB | NB | SB | NB | SB | NB | SB | NB | SB | NB | SB | NB | SB | NB | SB | NB |
| 12:00 AM | * | * | 9 | 4 | 7 | 1 | 8 | 0 | * | * | * | * | * | * | 8 | 2 |
| 01:00 | * | * | 7 | 0 | 5 | 1 | 6 | 0 | * | * | * | * | * | * | 6 | 0 |
| 02:00 | * | * | 3 | 1 | 4 | 1 | 3 | 0 | * | * | * | * | * | * | 3 | 1 |
| 03:00 | * | * | 1 | 3 | 1 | 4 | 2 | 4 | * | * | * | * | * | * | 1 | 4 |
| 04:00 | * | * | 1 | 12 | 1 | 15 | 1 | 14 | * | * | * | * | * | * | 1 | 14 |
| 05:00 | * | * | 4 | 19 | 2 | 21 | 2 | 16 | * | * | * | * | * | * | 3 | 19 |
| 06:00 | * | * | 9 | 48 | 10 | 48 | 7 | 44 | * | * | * | * | * | * | 9 | 47 |
| 07:00 | * | * | 26 | 98 | 19 | 90 | 38 | 85 | * | * | * | * | * | * | 28 | 91 |
| 08:00 | * | * | 40 | 75 | 35 | 83 | 32 | 95 | * | * | * | * | * | * | 36 | 84 |
| 09:00 | * | * | 47 | 56 | 31 | 51 | 33 | 45 | * | * | * | * | * | * | 37 | 51 |
| 10:00 | * | * | 45 | 54 | 38 | 41 | 32 | 59 | * | * | * | * | * | * | 38 | 51 |
| 11:00 | * | * | 47 | 39 | 45 | 57 | 44 | 51 | * | * | * | * | * | * | 45 | 49 |
| 12:00 PM | * | * | 63 | 42 | 67 | 49 | 49 | 37 | * | * | * | * | * | * | 60 | 43 |
| 01:00 | * | * | 60 | 47 | 58 | 40 | 57 | 41 | * | * | * | * | * | * | 58 | 43 |
| 02:00 | * | * | 74 | 53 | 71 | 44 | 74 | 40 | * | * | * | * | * | * | 73 | 46 |
| 03:00 | * | * | 95 | 54 | 97 | 51 | 66 | 46 | * | * | * | * | * | * | 86 | 50 |
| 04:00 | * | * | 120 | 52 | 100 | 54 | 124 | 54 | * | * | * | * | * | * | 115 | 53 |
| 05:00 | * | * | 128 | 44 | 136 | 39 | 128 | 59 | * | * | * | * | * | * | 131 | 47 |
| 06:00 | * | * | 89 | 50 | 164 | 49 | 83 | 38 | * | * | * | * | * | * | 112 | 46 |
| 07:00 | * | * | 63 | 30 | 53 | 74 | 84 | 22 | * | * | * | * | * | * | 67 | 42 |
| 08:00 | * | * | 41 | 23 | 39 | 44 | 47 | 21 | * | * | * | * | * | * | 42 | 29 |
| 09:00 | * | * | 45 | 13 | 40 | 10 | 37 | 27 | * | * | * | * | * | * | 41 | 17 |
| 10:00 | * | * | 20 | 11 | 18 | 6 | 22 | 9 | * | * | * | * | * | * | 20 | 9 |
| 11:00 | * | * | 12 | 2 | 13 | 5 | 13 | 1 | * | * | * | * | * | * | 13 | 3 |
| Lane | 0 | 0 | 1049 | 830 | 1054 | 878 | 992 | 808 | 0 | 0 | 0 | 0 | 0 | 0 | 1033 | 841 |
| Day |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| AM Peak | - | - | 09:00 | 07:00 | 11:00 | 07:00 | 11:00 | 08:00 | - | - | - | - | - | - | 11:00 | 07:00 |
| Vol. | - | - | 47 | 98 | 45 | 90 | 44 | 95 | - | - | - | - | - | - | 45 | 91 |
| PM Peak | - | - | 17:00 | 15:00 | 18:00 | 19:00 | 17:00 | 17:00 | - | - | - | - | - | - | 17:00 | 16:00 |
| Vol. | - | - | 128 | 54 | 164 | 74 | 128 | 59 | - | - | - | - | - | - | 131 | 53 |



| Road\#-Section \# | Distance | Link Description | 2004 | 2005 | 2006 | 2007 | 2008 | 2009 | 2010 | 2011 | 2012 | 2013 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | CR 13 |  |  |  |  |  |  |  |  |  |  |
| 012-01 | 3.9 |  | 1,400 |  |  | 1,400 |  |  | 1,300 |  |  | 1,400 |
|  |  | County Boundary |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | Hwy 89 |  |  |  |  |  |  |  |  |  |  |
| 013-01 | 5.7 |  | 2,800 |  |  | 2,800 |  |  | 2,600 |  |  | 2,500 |
|  |  | CR 5 |  |  |  |  |  |  |  |  |  |  |
| 013-02 | 9.5 |  | 2,000 |  |  | 1,900 |  |  | 1,900 |  |  | 2,900 |
|  |  | CR 12 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | CR 50 |  |  |  |  |  |  |  |  |  |  |
| 014-01 | 3.8 |  | 1,000 |  |  | 1,000 |  |  | 950 |  |  | 1,200 |
|  |  | Adjala-New Tech Townline |  |  |  |  |  |  |  |  |  |  |
| 014-02 | 2.9 |  | 1,700 |  |  | 2,000 |  |  | 1,700 |  |  | 1,700 |
|  |  | CR 10 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | Victoria St. / Alliston |  |  |  |  |  |  |  |  |  |  |
| 015-01A | 1.1 |  | 8,950 |  |  | 8,000 |  |  | 8,800 |  |  | 9,100 |
|  |  | Essa Rd |  |  |  |  |  |  |  |  |  |  |
| 015-01 | 4.7 |  | 5,100 |  |  | 5,800 |  |  | 5,600 |  |  | 5,200 |
|  |  | CR 5 |  |  |  |  |  |  |  |  |  |  |
| 015-02 | 3.5 |  | 4,800 |  |  | 4,900 |  |  | 5,000 |  |  | 4,800 |
|  |  | CR 21 |  |  |  |  |  |  |  |  |  |  |
| 015-03 | 1.2 |  | 3,100 |  |  | 3,000 |  |  | 2,400 |  |  | 2,100 |
|  |  | Base Borden South Limit |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | CR 23 |  |  |  |  |  |  |  |  |  |  |
| 016-01 | 6.2 |  |  | 5,100 |  |  | 4,500 |  |  | 5,100 |  |  |
|  |  | Hwy 400 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | Coldwater / North Limits |  |  |  |  |  |  |  |  |  |  |
| 017-01 | 6.8 |  |  | 1,600 |  |  | 2,100 |  |  | 1,500 |  |  |
|  |  | Quarry Road |  |  |  |  |  |  |  |  |  |  |
| 017-02 | 4.3 |  |  | 1,200 |  |  | 1,200 |  |  | 1,200 |  |  |
|  |  | 4th Conc. Silkline |  |  |  |  |  |  |  |  |  |  |
| 017-03 | 11.9 |  |  | 450 |  |  | 400 |  |  | 400 |  |  |
|  |  | Big Chute |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | Hwy 12 |  |  |  |  |  |  |  |  |  |  |
| 019-01 | 2.5 |  |  | 1,000 |  |  | 900 |  |  | 1,200 |  |  |
|  |  | Hwy 400 |  |  |  |  |  |  |  |  |  |  |
| 019-02 | 2.0 |  |  | 1,700 |  |  | 1,500 |  |  | 1,600 |  |  |
|  |  | 8th Conc / Moonstone |  |  |  |  |  |  |  |  |  |  |
| 019-03 | 10.8 |  |  | 1,000 |  |  | 950 |  |  | 1,000 |  |  |
|  |  | HWy 93 |  |  |  |  |  |  |  |  |  |  |
| 019-04 | 8.1 |  |  | 1,700 |  |  | 1,500 |  |  | 1,900 |  |  |
|  |  | CR 27 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | Barrie Limits |  |  |  |  |  |  |  |  |  |  |
| 020-01 | 7.4 |  |  | 3,500 |  |  | 2,800 |  |  | 3,100 |  |  |
|  |  | Line 3 Oro-Medonte |  |  |  |  |  |  |  |  |  |  |
| 020-02 | 6.0 |  |  | 1,800 |  |  | 1,800 |  |  | 1,500 |  |  |
|  |  | Line 7 Oro-Medonte |  |  |  |  |  |  |  |  |  |  |
| 020-03 | 6.2 |  |  | 1,000 |  |  | 1,000 |  |  | 1,000 |  |  |
|  |  | Line 11 Oro-Medonte |  |  |  |  |  |  |  |  |  |  |
| 020-04 | 1.1 |  |  | 1,900 |  |  | 1,800 |  |  | 1,800 |  |  |
|  |  | Line 11/ Hwy 11 |  |  |  |  |  |  |  |  |  |  |

County of Simcoe

## Transportation \& Engineering

Annual Average Daily Traffic Summary (A.A.D.T.)
Updated Nov 2011

| Road\# - Section \# | Distance | Link Description | $\underline{2002}$ | 2003 | 2004 | 2005 | 2006 | $\underline{2007}$ | 2008 | 2009 | 2010 | 2011 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | CR 27 |  |  |  |  |  |  |  |  |  |  |
| 001-01 | 6.1 |  |  |  | 5,000 |  |  | 4,300 |  |  | 4,100 |  |
|  |  | 15th SR New Tecumseth |  |  |  |  |  |  |  |  |  |  |
| 001-02 | 2.0 |  |  |  | 5,300 |  |  | 4,900 |  |  | 4,700 |  |
|  |  | East Limits / Beeton |  |  |  |  |  |  |  |  |  |  |
| 001-03 | 1.9 |  |  |  | 5,000 |  |  | 4,900 |  |  | 4,500 |  |
|  |  | CR 10 |  |  |  |  |  |  |  |  |  |  |
| 001-04 | 6.6 |  |  |  | 2,300 |  |  | 3,000 |  |  | 3,200 |  |
|  |  | CR 50 |  |  |  |  |  |  |  |  |  |  |
| 001-05 | 7.2 |  |  |  | 2,000 |  |  | 1,800 |  |  | 1,900 |  |
|  |  | Simcoe Boundary |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | CR 39 |  |  |  |  |  |  |  |  |  |  |
| 003-01 | 3.1 |  |  |  | 4,600 |  |  | 4,400 |  |  | 4,700 |  |
|  |  | CR 4 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | Bradford Limits |  |  |  |  |  |  |  |  |  |  |
| 004-01 | 9.8 |  |  |  | 13,400 |  |  | 12,700 |  |  | 12,500 |  |
|  |  | CR 89/CR 3 |  |  |  |  |  |  |  |  |  |  |
| 004-02 | 4.4 |  |  |  | 8,600 |  |  | 9,600 |  |  | 9,200 |  |
|  |  | Line 4 / Churchill |  |  |  |  |  |  |  |  |  |  |
| 004-03 | 5.5 |  |  |  | 8,000 |  |  | 10,100 |  |  | 9,000 |  |
|  |  | CR 21 |  |  |  |  |  |  |  |  |  |  |
| 004-04 | 2.8 |  |  |  | 10,500 |  |  | 11,200 |  |  | 11,200 |  |
|  |  | Victoria St. / Stroud |  |  |  |  |  |  |  |  |  |  |
| 004-05 | 2.9 |  |  |  | 10,800 |  |  | 11,700 |  |  | 13,000 |  |
|  |  | Lockhart Dr. / Barrie Limit |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | CR 15 |  |  |  |  |  |  |  |  |  |  |
| 005-01 | 4.1 |  |  |  | 3,600 |  |  | 3,800 |  |  | 4,100 |  |
|  |  | CR 13 |  |  |  |  |  |  |  |  |  |  |
| 005-02 | 5.6 |  |  |  | 1,800 |  |  | 1,700 |  |  | 2,400 |  |
|  |  | County Boundary |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | CR 27 N |  |  |  |  |  |  |  |  |  |  |
| 006-01 | 5.5 |  | 4,700 |  |  | 4,700 |  |  | 4,400 |  |  | 4,600 |
|  |  | Conc 4/ Tiny |  |  |  |  |  |  |  |  |  |  |
| 006-02 | 8.2 |  | 4,200 |  |  | 3,900 |  |  | 3,700 |  |  | 4,200 |
|  |  | CR 25 / Perkinsfield |  |  |  |  |  |  |  |  |  |  |
| 006-03 | 1.4 |  | 4,700 |  |  | 4,700 |  |  | 4,400 |  |  | 4,700 |
|  |  | Conc 11/ Tiny |  |  |  |  |  |  |  |  |  |  |
| 006-04 | 2.8 |  |  |  |  | 3,400 |  |  | 3,400 |  |  | 3,900 |
|  |  | Conc 13/ Tiny |  |  |  |  |  |  |  |  |  |  |
| 006-05 | 4.1 |  | 3,200 |  |  | 3,000 |  |  | 3,100 |  |  | 3,200 |
|  |  | CR 26 |  |  |  |  |  |  |  |  |  |  |


| Road\#-Section \# | Distance | Link Description | 2002 | 2003 | 2004 | 2005 | 2006 | 2007 | 2008 | 2009 | 2010 | 2011 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Barrie Limits |  |  |  |  |  |  |  |  |  |  |
| 020-01 | 7.4 |  | 3,050 |  |  | 3,500 |  |  | 2,800 |  |  | 3,100 |
|  |  | Line 3 Oro-Medonte |  |  |  |  |  |  |  |  |  |  |
| 020-02 | 6.0 |  | 1,600 |  |  | 1,800 |  |  | 1,800 |  |  | 1,500 |
|  |  | Line 7 Oro-Medonte |  |  |  |  |  |  |  |  |  |  |
| 020-03 | 6.2 |  | 900 |  |  | 1,000 |  |  | 1,000 |  |  | 1,000 |
|  |  | Line 11 Oro-Medonte |  |  |  |  |  |  |  |  |  |  |
| 020-04 | 1.1 |  | 1,600 |  |  | 1,900 |  |  | 1,800 |  |  | 1,800 |
|  |  | Line 11/ Hwy 11 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | CR 39 |  |  |  |  |  |  |  |  |  |  |
| 021-01 | 3.0 |  |  |  | 10,600 |  |  | 9,800 |  |  | 11,100 |  |
|  |  | CR 4 |  |  |  |  |  |  |  |  |  |  |
| 021-02 | 3.1 |  |  |  | 10,600 |  |  | 11,400 |  |  | 11,800 |  |
|  |  | CR 54 (10th SR) |  |  |  |  |  |  |  |  |  |  |
| 021-03 | 2.2 |  |  |  | 12,000 |  |  | 13,800 |  |  | 14,000 |  |
|  |  | Hwy 400 |  |  |  |  |  |  |  |  |  |  |
| 021-04 | 0.6 |  |  |  | 10,900 |  |  | 11,700 |  |  | 13,100 |  |
|  |  | 5th SR Innisfil |  |  |  |  |  |  |  |  |  |  |
| 021-05 | 3.0 |  |  |  | 6,900 |  |  | 9,000 |  |  | 9,000 |  |
|  |  | CR 27 |  |  |  |  |  |  |  |  |  |  |
| 021-06 | 6.4 |  |  |  | 4,800 |  |  | 5,100 |  |  | 5,700 |  |
|  |  | CR 56 |  |  |  |  |  |  |  |  |  |  |
| 021-07 | 4.2 |  |  |  | 4,400 |  |  | 5,700 |  |  | 6,100 |  |
|  |  | CR10 |  |  |  |  |  |  |  |  |  |  |
| 021-08 | 4.2 |  |  |  | 3,300 |  |  | 3,600 |  |  | 4,700 |  |
|  |  | CR 15 |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | Hwy 12 |  |  |  |  |  |  |  |  |  |  |
| 022-01 | 9.6 |  | 4,100 |  |  | 4,500 |  |  | 4,600 |  |  | 4,300 |
|  |  | Coulson/ 7th Line |  |  |  |  |  |  |  |  |  |  |
| 022-02 | 4.9 |  | 4,100 |  |  | 4,100 |  |  | 4,600 |  |  | 4,300 |
|  |  | Horseshoe Valley Resort Ent. |  |  |  |  |  |  |  |  |  |  |
| 022-03 | 5.2 |  | 5,700 |  |  | 6,100 |  |  | 6,200 |  |  | 5,500 |
|  |  | CR 93 |  |  |  |  |  |  |  |  |  |  |
| 022-04 | 1.4 |  | 5,600 |  |  | 6,100 |  |  | 6,200 |  |  | 5,800 |
|  |  | Hwy 400 |  |  |  |  |  |  |  |  |  |  |
| 022-05 | 4.6 |  | 4,900 |  |  | 5,300 |  |  | 5,300 |  |  | 5,600 |
|  |  | CR 27 |  |  |  |  |  |  |  |  |  |  |
| 022-06 | 2.6 |  | 4,000 |  |  | 4,100 |  |  | 4,400 |  |  | 4,400 |
|  |  | Wilson Drive - Anten Mills |  |  |  |  |  |  |  |  |  |  |
| 022-07 | 3.9 |  | 4,200 |  |  | 3,900 |  |  | 3,900 |  |  | 3,900 |
|  |  | CR 29 |  |  |  |  |  |  |  |  |  |  |
| 022-08 | 0.5 |  | 5,500 |  |  | 5,200 |  |  | 5,200 |  |  | 6,200 |
|  |  | Hwy 26 |  |  |  |  |  |  |  |  |  |  |



Accu-Traffic Inc
Traffic Monitoring \& Data Analysis


## Accu-Traffic Inc.



Comments


## Accu-Traffic Inc.



Comments

## Accu-Traffic Inc.



Comments

## Accu-Traffic Inc.



Comments

## Accu-Traffic Inc.



Comments





## APPENDIX B: 2015 TRAFFIC OPERATIONS




|  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |





|  | $\rangle$ |  |  | 7 |  |  | 4 | $\dagger$ | $p$ | $\downarrow$ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | \$ |  |  | \$ |  |  | ¢ |  |
| Volume (veh/h) | 10 | 10 | 15 | 5 | 30 | 20 | 20 | 150 | 10 | 10 | 20 | 10 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 11 | 11 | 16 | 5 | 33 | 22 | 22 | 163 | 11 | 11 | 22 | 11 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  | None |  |  | None |  |  |  |  |  |  |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| VC, conflicting volume | 54 |  |  | 27 |  |  | 117 | 106 | 19 | 188 | 103 | 43 |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC2, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu, unblocked vol | 54 |  |  | 27 |  |  | 117 | 106 | 19 | 188 | 103 | 43 |
| tC , single (s) | 4.1 |  |  | 4.1 |  |  | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 2.2 |  |  | 2.2 |  |  | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free \% | 99 |  |  | 100 |  |  | 97 | 79 | 99 | 98 | 97 | 99 |
| cM capacity (veh/h) | 1551 |  |  | 1587 |  |  | 826 | 776 | 1059 | 636 | 779 | 1027 |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB1 |  |  |  |  |  |  |  |  |
| Volume Total | 38 | 60 | 196 | 43 |  |  |  |  |  |  |  |  |
| Volume Left | 11 | 5 | 22 | 11 |  |  |  |  |  |  |  |  |
| Volume Right | 16 | 22 | 11 | 11 |  |  |  |  |  |  |  |  |
| cSH | 1551 | 1587 | 793 | 782 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.01 | 0.00 | 0.25 | 0.06 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.2 | 0.1 | 7.4 | 1.3 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 2.1 | 0.7 | 11.0 | 9.9 |  |  |  |  |  |  |  |  |
| Lane LOS | A | A | B | A |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 2.1 | 0.7 | 11.0 | 9.9 |  |  |  |  |  |  |  |  |
| Approach LOS |  |  | B | A |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 8.0 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 21.8\% | ICU Level of Service |  |  | A |  |  | A |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |




|  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |





|  | 4 |  |  | 7 |  |  | 4 | $\dagger$ | 7 | $\downarrow$ | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | \$ |  |  | $\uparrow$ |  |  | ${ }_{\text {¢ }}$ |  |
| Volume (veh/h) | 10 | 35 | 20 | 15 | 30 | 10 | 10 | 60 | 10 | 10 | 95 | 10 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| $\begin{array}{lllllllll}\text { Hourly flow rate (vph) } & 11 & 38 & 22 & 16 & 33 & 11 & 11 & 65 \\ \text { Pedestrians }\end{array}$ |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed ( $\mathrm{m} / \mathrm{s}$ ) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  | None |  |  | None |  |  |  |  |  |  |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| VC , conflicting volume | 43 |  |  | 60 |  |  | 204 | 147 | 49 | 185 | 152 | 38 |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu, unblocked vol | 43 |  |  | 60 |  |  | 204 | 147 | 49 | 185 | 152 | 38 |
| tC , single (s) | 4.1 |  |  | 4.1 |  |  | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| $\mathrm{tC}, 2$ stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 2.2 |  |  | 2.2 |  |  | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free \% | 99 |  |  | 99 |  |  | 98 | 91 | 99 | 98 | 86 | 99 |
| cM capacity (veh/h) | 1565 |  |  | 1544 |  |  | 656 | 732 | 1020 | 706 | 727 | 1034 |
| Direction, Lane \# E | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 71 | 60 | 87 | 125 |  |  |  |  |  |  |  |  |
| Volume Left | 11 | 16 | 11 | 11 |  |  |  |  |  |  |  |  |
| Volume Right | 22 | 11 | 11 | 11 |  |  |  |  |  |  |  |  |
| cSH 15 | 1565 | 1544 | 747 | 744 |  |  |  |  |  |  |  |  |
| Volume to Capacity 0 | 0.01 | 0.01 | 0.12 | 0.17 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.2 | 0.2 | 3.0 | 4.6 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 1.2 | 2.1 | 10.5 | 10.8 |  |  |  |  |  |  |  |  |
| Lane LOS | A | A | B | B |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 1.2 | 2.1 | 10.5 | 10.8 |  |  |  |  |  |  |  |  |
| Approach LOS |  |  | B | B |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 7.2 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 19.3\% |  | CU Level | f Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |

## APPENDIX C: 2025 TRAFFIC OPERATIONS




|  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |





|  | $\prime$ |  |  | 7 | $\leftarrow$ |  | 4 | $\uparrow$ | 7 | $\downarrow$ | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\dagger$ |  |  | \$ |  |  | ¢ |  |  | ${ }_{4}$ |  |
| Volume (veh/h) | 15 | 15 | 20 | 10 | 40 | 25 | 25 | 170 | 15 | 15 | 25 | 15 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 16 | 16 | 22 | 11 | 43 | 27 | 27 | 185 | 16 | 16 | 27 | 16 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  | None |  |  | None |  |  |  |  |  |  |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| VC , conflicting volume | 71 |  |  | 38 |  |  | 168 | 152 | 27 | 247 | 149 | 57 |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu, unblocked vol | 71 |  |  | 38 |  |  | 168 | 152 | 27 | 247 | 149 | 57 |
| tC , single (s) | 4.1 |  |  | 4.1 |  |  | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| $\mathrm{tC}, 2$ stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 2.2 |  |  | 2.2 |  |  | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free \% | 99 |  |  | 99 |  |  | 96 | 75 | 98 | 97 | 96 | 98 |
| cM capacity (veh/h) | 1530 |  |  | 1572 |  |  | 750 | 727 | 1048 | 552 | 729 | 1009 |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 54 | 82 | 228 | 60 |  |  |  |  |  |  |  |  |
| Volume Left | 16 | 11 | 27 | 16 |  |  |  |  |  |  |  |  |
| Volume Right | 22 | 27 | 16 | 16 |  |  |  |  |  |  |  |  |
| cSH | 1530 | 1572 | 746 | 720 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.01 | 0.01 | 0.31 | 0.08 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.2 | 0.2 | 9.9 | 2.1 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 2.3 | 1.0 | 11.9 | 10.4 |  |  |  |  |  |  |  |  |
| Lane LOS | A | A | B | B |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 2.3 | 1.0 | 11.9 | 10.4 |  |  |  |  |  |  |  |  |
| Approach LOS |  |  | B | B |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 8.4 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 24.5\% |  | CU Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |




|  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |





|  | 4 |  |  | 7 |  |  | 4 | 4 | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 4 |  |  | \$ |  |  | \$ |  |  | ¢ |  |
| Volume (veh/h) | 15 | 45 | 25 | 20 | 40 | 15 | 15 | 70 | 15 | 15 | 105 | 15 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 16 | 49 | 27 | 22 | 43 | 16 | 16 | 76 | 16 | 16 | 114 | 16 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  | None |  |  | None |  |  |  |  |  |  |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC, conflicting volume | 60 |  |  | 76 |  |  | 264 | 198 | 62 | 245 | 204 | 52 |
| vC1, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| VC2, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 60 |  |  | 76 |  |  | 264 | 198 | 62 | 245 | 204 | 52 |
| tC, single (s) | 4.1 |  |  | 4.1 |  |  | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| $\mathrm{tC}, 2$ stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 2.2 |  |  | 2.2 |  |  | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free \% | 99 |  |  | 99 |  |  | 97 | 89 | 98 | 97 | 83 | 98 |
| cM capacity (veh/h) | 1544 |  |  | 1523 |  |  | 579 | 680 | 1002 | 626 | 675 | 1016 |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 92 | 82 | 109 | 147 |  |  |  |  |  |  |  |  |
| Volume Left | 16 | 22 | 16 | 16 |  |  |  |  |  |  |  |  |
| Volume Right | 27 | 16 | 16 | 16 |  |  |  |  |  |  |  |  |
| cSH | 1544 | 1523 | 696 | 695 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.01 | 0.01 | 0.16 | 0.21 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.2 | 0.3 | 4.2 | 6.0 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 1.4 | 2.1 | 11.1 | 11.6 |  |  |  |  |  |  |  |  |
| Lane LOS | A | A | B | B |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 1.4 | 2.1 | 11.1 | 11.6 |  |  |  |  |  |  |  |  |
| Approach LOS |  |  | B | B |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 7.5 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 22.2\% |  | CU Level | f Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |

## APPENDIX D:

## 2015 BURL'S CREEK EVENT CALENDAR

\(\left.$$
\begin{array}{|l|l|l|l|l|l|l|l|l|}\hline \begin{array}{l}\text { FARMER'S } \\
\text { MARKET }\end{array}
$$ \& \& FESTIVAL \& \begin{array}{l}SINGLE DAY <br>

EVENT\end{array} \& \& FLEA MARKET\end{array}\right]\)| COMMUNITY |
| :--- |
| UNCONFIRMED |

## May 2015

| Mon | Tue | Wed | Thu | Fri | Sat | Sun |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: |
|  |  |  |  |  |  |  |

## June 2015

| Mon | Tue | Wed | Thu | Fri | Sat | Sun |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 2 | 3 | 4 | 5 | 6 | 7 |
|  |  |  | BARRIE AUTOMOTIVE FLEA MARKET | BARRIE AUTOMOTIVE FLEA MARKET | BARRIE AUTOMOTIVE FLEA MARKET | BARRIE AUTOMOTIVE FLEA MARKET |
| 8 | 9 | 10 | 11 | 12 | 13 | 14 |
|  |  | SOCCER |  | FARMER'S |  |  |
| 15 | 16 | 17 | 18 | 19 | 20 | 21 |
|  |  | SOCCER |  | FARMER'S MARKET |  |  |
| 22 | 23 | $24$ <br> SOCCER | 25 | 26 | 27 | 28 |
|  |  |  |  | FARMER'S MARKET | CONTEMORARY MUSIC CONCET |  |
| 29 | 30 |  |  |  |  |  |

## July 2015

| Mon | Tue | Wed | Thu | Fri | Sat | Sun |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 1 | 2 | FARMER'S MARKET | 4 | 5 |
| 6 | 7 | $8$ <br> SOCCER | 9 | 10 | 11 | 12 |
|  |  |  |  | FARMER'S MARKET |  |  |
| 13 | 14 | 15 | 16 | 17 | 18 | 19 |
|  |  | SOCCER |  | FARMER'S MARKET |  |  |
| 20 | 21 | 22 | 23 | 24 | 25 | 26 |
|  |  |  |  | WAYHOME | WAYHOME | WAYHOME |
| 27 | 28 | 29 | 30 | 31 |  |  |
|  |  | SOCCER |  | FARMER'S MARKET |  |  |

## August 2015

| Mon | Tue | Wed | Thu | Fri | Sat | Sun |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: |
|  |  |  |  |  |  |  |

## September 2015

| Mon | Tue | Wed | Thu | Fri | Sat | Sun |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1 | 2 | 3 | 4 | 5 | 6 |
|  |  |  |  | FARMER'S MARKET |  |  |
| 7 | 8 | 9 | 10 | 11 | 12 | 13 |
|  |  |  | BARRIE AUTOMOTIVE FLEA MARKET | BARRIE AUTOMOTIVE FLEA MARKET | BARRIE AUTOMOTIVE FLEA MARKET | BARRIE AUTOMOTIVE FLEA MARKET |
| 14 | 15 | 16 | 17 | 18 | 19 | 20 |
|  |  | FARMER'S MARKET |  |  |  |  |
| 21 | 22 | 23 |  | 24 | 25 | 26 | 27 |
|  |  |  | FARMER'S MARKET |  | HURONIA <br> FUR \& FEATHERS |  |
| 28 | 29 | 30 |  |  |  |  |

## October 2015

| Mon | Tue | Wed | Thu | Fri | Sat | Sun |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | 1 |  | 3 | 4 |
| 5 | 6 | 7 | 8 | FARMER'S MARKET | 10 | 11 |
| 12 | 13 | 14 | 15 | 16 | 17 | 18 |



APPENDIX E: burl's CREEK TRAFFIC VOLUMES


| 2015 |  | Background Traffic VolumesAM |  |  |  | Site Traffic Volumes |  |  |  | Total Traffic Volumes |  |  |  | Volume/Capacity |  |  |  | Total Capacity |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | PN |  | A |  | PN |  |  |  |  |  |  |
|  |  | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB |  |
| Hwy 11 | Fri |  |  |  |  | 2,540 | 1,260 | 2,820 | 1,525 |  |  | 75 | 50 | 2,540 | 1,260 | 2,895 | 1,575 |  |  | 0.89 | 0.48 | 3,270 |
|  | Sat | 2,520 | 1,585 | 2,475 | 1,640 |  |  |  |  | 2,520 | 1,585 | 2,475 | 1,640 |  |  |  |  | 3,270 |
|  | Sun | 1,755 | 2,215 | 1,715 | 2,605 |  |  |  |  | 1,755 | 2,215 | 1,715 | 2,605 |  |  |  |  | 3,270 |
|  | Weekday | 1,520 | 1,225 | 2,005 | 1,420 |  |  |  |  | 1,520 | 1,225 | 2,005 | 1,420 |  |  |  |  | 3,270 |
| SR 20 | Fri | 35 | 55 | 55 | 45 |  |  | 4 | 4 | 35 | 55 | 59 | 49 |  |  | 0.07 | 0.06 | 800 |
| Line 3-7 | Sat | 40 | 65 | 70 | 55 |  |  |  |  | 40 | 65 | 70 | 55 |  |  |  |  | 800 |
|  | Sun | 35 | 55 | 60 | 50 |  |  |  |  | 35 | 55 | 60 | 50 |  |  |  |  | 800 |
|  | Weekday | 35 | 50 | 55 | 40 |  |  |  |  | 35 | 50 | 55 | 40 |  |  |  |  | 800 |
| SR 20 | Fri | 30 | 50 | 60 | 45 |  |  | 4 | 4 | 30 | 50 | 64 | 49 |  |  | 0.08 | 0.06 | 800 |
| Line7-11 | Sat | 30 | 60 | 65 | 60 |  |  |  |  | 30 | 60 | 65 | 60 |  |  |  |  | 800 |
|  | Sun | 30 | 50 | 60 | 55 |  |  |  |  | 30 | 50 | 60 | 55 |  |  |  |  | 800 |
|  | Weekday | 25 | 45 | 50 | 45 |  |  |  |  | 25 | 45 | 50 | 45 |  |  |  |  | 800 |
| Line 7 | Fri | 60 | 175 | 120 | 85 |  |  |  |  | 60 | 175 | 120 | 85 |  |  |  |  | 600 |
|  | Sat | 70 | 215 | 145 | 105 |  |  |  |  | 70 | 215 | 145 | 105 |  |  |  |  | 600 |
|  | Sun | 65 | 190 | 130 | 90 |  |  |  |  | 65 | 190 | 130 | 90 |  |  |  |  | 600 |
|  | Weekday | 55 | 165 | 110 | 80 |  |  |  |  | 55 | 165 | 110 | 80 |  |  |  |  | 600 |
| Line 8 | Fri | 10 | 5 | 10 | 15 |  |  | 75 | 75 | 10 | 5 | 85 | 90 |  |  | 0.14 | 0.15 | 600 |
|  | Sat | 10 | 10 | 10 | 15 |  |  |  |  | 10 | 10 | 10 | 15 |  |  |  |  | 600 |
|  | Sun | 10 | 5 | 10 | 15 |  |  |  |  | 10 | 5 | 10 | 15 |  |  |  |  | 600 |
|  | Weekday | 10 | 5 | 5 | 15 |  |  |  |  | 10 | 5 | 5 | 15 |  |  |  |  | 600 |
| Line 9 | Fri | 30 | 140 | 90 | 60 |  |  | 50 |  | 30 | 140 | 140 | 60 |  |  | 0.23 |  | 600 |
|  | Sat | 40 | 170 | 110 | 70 |  |  |  |  | 40 | 170 | 110 | 70 |  |  |  |  | 600 |
|  | Sun | 30 | 155 | 100 | 65 |  |  |  |  | 30 | 155 | 100 | 65 |  |  |  |  | 600 |
|  | Weekday | 30 | 135 | 85 | 55 |  |  |  |  | 30 | 135 | 85 | 55 |  |  |  |  | 600 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2020 |  | Background Traffic VolumesAM |  |  |  | Site Traffic Volumes |  |  |  | Total Traffic Volumes |  |  |  | Volume/Capacity |  |  |  | Total Capacity |
|  |  | A |  | PN |  |  |  |  |  |  |  | P |  |  |
|  |  | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB |  |
| Hwy 11 | Fri |  |  |  |  | 2,685 | 1,335 | 2,980 | 1,610 |  |  | 75 | 50 | 2,685 | 1,335 | 3,055 | 1,660 |  |  | 0.93 | 0.51 | 3,270 |
|  | Sat | 2,660 | 1,675 | 2,615 | 1,730 |  |  |  |  | 2,660 | 1,675 | 2,615 | 1,730 |  |  |  |  | 3,270 |
|  | Sun | 1,855 | 2,340 | 1,815 | 2,745 |  |  |  |  | 1,855 | 2,340 | 1,815 | 2,745 |  |  |  |  | 3,270 |
|  | Weekday | 1,695 | 1,295 | 2,240 | 1,580 |  |  |  |  | 1,695 | 1,295 | 2,240 | 1,580 |  |  |  |  | 3,270 |
| SR 20 | Fri | 50 | 70 | 70 | 60 |  |  | 4 | 4 | 50 | 70 | 74 | 64 |  |  | 0.09 | 0.08 | 800 |
| Line 3-7 | Sat | 55 | 80 | 85 | 70 |  |  |  |  | 55 | 80 | 85 | 70 |  |  |  |  | 800 |
|  | Sun | 50 | 70 | 75 | 65 |  |  |  |  | 50 | 70 | 75 | 65 |  |  |  |  | 800 |
|  | Weekday | 50 | 65 | 70 | 55 |  |  |  |  | 50 | 65 | 70 | 55 |  |  |  |  | 800 |
| $\begin{aligned} & \hline \text { SR } 20 \\ & \text { Line7-11 } \end{aligned}$ | Fri | 45 | 65 | 75 | 60 |  |  | 4 | 4 | 45 | 65 | 79 | 64 |  |  | 0.10 | 0.08 | 800 |
|  | Sat | 45 | 75 | 80 | 75 |  |  |  |  | 45 | 75 | 80 | 75 |  |  |  |  | 800 |
|  | Sun | 45 | 65 | 75 | 70 |  |  |  |  | 45 | 65 | 75 | 70 |  |  |  |  | 800 |
|  | Weekday | 40 | 60 | 65 | 60 |  |  |  |  | 40 | 60 | 65 | 60 |  |  |  |  | 800 |
| Line 7 | Fri | 70 | 195 | 140 | 100 |  |  |  |  | 70 | 195 | 140 | 100 |  |  |  |  | 600 |
|  | Sat | 80 | 245 | 165 | 120 |  |  |  |  | 80 | 245 | 165 | 120 |  |  |  |  | 600 |
|  | Sun | 75 | 215 | 150 | 105 |  |  |  |  | 75 | 215 | 150 | 105 |  |  |  |  | 600 |
|  | Weekday | 65 | 185 | 125 | 95 |  |  |  |  | 65 | 185 | 125 | 95 |  |  |  |  | 600 |
| Line 8 | Fri | 15 | 10 | 15 | 20 |  |  | 75 | 75 | 15 | 10 | 90 | 95 |  |  | 0.15 | 0.16 | 600 |
|  | Sat | 15 | 15 | 15 | 20 |  |  |  |  | 15 | 15 | 15 | 20 |  |  |  |  | 600 |
|  | Sun | 15 | 10 | 15 | 20 |  |  |  |  | 15 | 10 | 15 | 20 |  |  |  |  | 600 |
|  | Weekday | 15 | 10 | 10 | 20 |  |  |  |  | 15 | 10 | 10 | 20 |  |  |  |  | 600 |
| Line 9 | Fri | 40 | 160 | 105 | 70 |  |  | 50 |  | 40 | 160 | 155 | 70 |  |  | 0.26 |  | 600 |
|  | Sat | 50 | 195 | 125 | 85 |  |  |  |  | 50 | 195 | 125 | 85 |  |  |  |  | 600 |
|  | Sun | 40 | 175 | 115 | 75 |  |  |  |  | 40 | 175 | 115 | 75 |  |  |  |  | 600 |
|  | Weekday | 40 | 150 | 100 | 65 |  |  |  |  | 40 | 150 | 100 | 65 |  |  |  |  | 600 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2025 |  | Background Traffic VolumesAM |  |  |  | Site Traffic Volumes |  |  |  | Total Traffic Volumes |  |  |  | Volume/Capacity |  |  |  | Total Capacity |
|  |  |  |  | PM |  | A |  | PN |  |  | Volumer | P |  |  |
|  |  | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB |  | NB/EB | SB/WB |  |
| Hwy 11 | Fri |  |  |  |  | 2,820 | 1,410 | 3,145 | 1,700 |  |  | 75 | 50 | 2,820 | 1,410 | 3,220 | 1,750 |  |  | 0.98 | 0.54 | 3,270 |
|  | Sat | 2,805 | 1,770 | 2,760 | 1,825 |  |  |  |  | 2,805 | 1,770 | 2,760 | 1,825 |  |  |  |  | 3,270 |
|  | Sun | 1,955 | 2,465 | 1,920 | 2,890 |  |  |  |  | 1,955 | 2,465 | 1,920 | 2,890 |  |  |  |  | 3,270 |
|  | Weekday | 1,695 | 1,370 | 2,240 | 1,580 |  |  |  |  | 1,695 | 1,370 | 2,240 | 1,580 |  |  |  |  | 3,270 |
| SR 20 | Fri | 50 | 75 | 75 | 65 |  |  | 4 | 4 | 50 | 75 | 79 | 69 |  |  | 0.10 | 0.09 | 800 |
| Line 3-7 | Sat | 55 | 90 | 95 | 75 |  |  |  |  | 55 | 90 | 95 | 75 |  |  |  |  | 800 |
|  | Sun | 50 | 75 | 85 | 70 |  |  |  |  | 50 | 75 | 85 | 70 |  |  |  |  | 800 |
|  | Weekday | 50 | 65 | 75 | 60 |  |  |  |  | 50 | 65 | 75 | 60 |  |  |  |  | 800 |
| SR 20 | Fri | 45 | 70 | 80 | 65 |  |  | 4 | 4 | 45 | 70 | 84 | 69 |  |  | 0.11 | 0.09 | 800 |
| Line7-11 | Sat | 45 | 80 | 85 | 80 |  |  |  |  | 45 | 80 | 85 | 80 |  |  |  |  | 800 |
|  | Sun | 45 | 70 | 80 | 75 |  |  |  |  | 45 | 70 | 80 | 75 |  |  |  |  | 800 |
|  | Weekday | 40 | 60 | 70 | 65 |  |  |  |  | 40 | 60 | 70 | 65 |  |  |  |  | 800 |
| Line 7 | Fri | 75 | 215 | 155 | 110 |  |  |  |  | 75 | 215 | 155 | 110 |  |  |  |  | 600 |
|  | Sat | 90 | 270 | 180 | 130 |  |  |  |  | 90 | 270 | 180 | 130 |  |  |  |  | 600 |
|  | Sun | 85 | 235 | 165 | 115 |  |  |  |  | 85 | 235 | 165 | 115 |  |  |  |  | 600 |
|  | Weekday | 70 | 205 | 135 | 105 |  |  |  |  | 70 | 205 | 135 | 105 |  |  |  |  | 600 |
| Line 8 | Fri | 15 | 10 | 15 | 20 |  |  | 75 | 75 | 15 | 10 | 90 | 95 |  |  | 0.15 | 0.16 | 600 |
|  | Sat | 15 | 15 | 15 | 20 |  |  |  |  | 15 | 15 | 15 | 20 |  |  |  |  | 600 |
|  | Sun | 15 | 10 | 15 | 20 |  |  |  |  | 15 | 10 | 15 | 20 |  |  |  |  | 600 |
|  | Weekday | 15 | 10 | 10 | 20 |  |  |  |  | 15 | 10 | 10 | 20 |  |  |  |  | 600 |
| Line 9 | Fri | 40 | 175 | 115 | 75 |  |  | 50 |  | 40 | 175 | 165 | 75 |  |  | 0.28 |  | 600 |
|  | Sat | 50 | 215 | 135 | 90 |  |  |  |  | 50 | 215 | 135 | 90 |  |  |  |  | 600 |
|  | Sun | 40 | 190 | 125 | 80 |  |  |  |  | 40 | 190 | 125 | 80 |  |  |  |  | 600 |
|  | Weekday | 40 | 170 | 110 | 70 |  |  |  |  | 40 | 170 | 110 | 70 |  |  |  |  | 600 |


| 2015 |  | Background Traffic VolumesAM |  |  |  | Site Traffic Volumes |  |  |  | Total Traffic Volumes |  |  |  | Volume/Capacity |  |  |  | Total Capacity |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  | A |  | PN |  |  |  | PI |  |  |
|  |  | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB |  |
| Hwy 11 | Fri |  |  |  |  | 2,540 | 1,260 | 2,820 | 1,525 |  |  |  |  | 2,540 | 1,260 | 2,820 | 1,525 |  |  |  |  | 3,270 |
|  | Sat | 2,520 | 1,585 | 2,475 | 1,640 |  |  |  |  | 2,520 | 1,585 | 2,475 | 1,640 |  |  |  |  | 3,270 |
|  | Sun | 1,755 | 2,215 | 1,715 | 2,605 |  |  |  |  | 1,755 | 2,215 | 1,715 | 2,605 |  |  |  |  | 3,270 |
|  | Weekday | 1,520 | 1,225 | 2,005 | 1,420 |  |  | 150 | 150 | 1,520 | 1,225 | 2,155 | 1,570 |  |  | 0.66 | 0.48 | 3,270 |
| SR 20 | Fri | 35 | 55 | 55 | 45 |  |  |  |  | 35 | 55 | 55 | 45 |  |  |  |  | 800 |
| Line 3-7 | Sat | 40 | 65 | 70 | 55 |  |  |  |  | 40 | 65 | 70 | 55 |  |  |  |  | 800 |
|  | Sun | 35 | 55 | 60 | 50 |  |  |  |  | 35 | 55 | 60 | 50 |  |  |  |  | 800 |
|  | Weekday | 35 | 50 | 55 | 40 |  |  | 25 | 25 | 35 | 50 | 80 | 65 |  |  | 0.10 | 0.08 | 800 |
| SR 20 | Fri | 30 | 50 | 60 | 45 |  |  |  |  | 30 | 50 | 60 | 45 |  |  |  |  | 800 |
| Line7-11 | Sat | 30 | 60 | 65 | 60 |  |  |  |  | 30 | 60 | 65 | 60 |  |  |  |  | 800 |
|  | Sun | 30 | 50 | 60 | 55 |  |  |  |  | 30 | 50 | 60 | 55 |  |  |  |  | 800 |
|  | Weekday | 25 | 45 | 50 | 45 |  |  | (25) | 25 | 25 | 45 | 75 | 70 |  |  | 0.09 | 0.09 | 800 |
| Line 7 | Fri | 60 | 175 | 120 | 85 |  |  |  |  | 60 | 175 | 120 | 85 |  |  |  |  | 600 |
|  | Sat | 70 | 215 | 145 | 105 |  |  |  |  | 70 | 215 | 145 | 105 |  |  |  |  | 600 |
|  | Sun | 65 | 190 | 130 | 90 |  |  |  |  | 65 | 190 | 130 | 90 |  |  |  |  | 600 |
|  | Weekday | 55 | 165 | 110 | 80 |  |  | 450 | 450 | 55 | 165 | 560 | 530 |  |  | 0.93 | 0.88 | 600 |
| Line 8 | Fri | 10 | 5 | 10 | 15 |  |  |  |  | 10 | 5 | 10 | 15 |  |  |  |  | 600 |
|  | Sat | 10 | 10 | 10 | 15 |  |  |  |  | 10 | 10 | 10 | 15 |  |  |  |  | 600 |
|  | Sun | 10 | 5 | 10 | 15 |  |  |  |  | 10 | 5 | 10 | 15 |  |  |  |  | 600 |
|  | Weekday | 10 | 5 | 5 | 15 |  |  |  |  | 10 | 5 | 5 | 15 |  |  |  |  | 600 |
| Line 9 | Fri | 30 | 140 | 90 | 60 |  |  |  |  | 30 | 140 | 90 | 60 |  |  |  |  | 600 |
|  | Sat | 40 | 170 | 110 | 70 |  |  |  |  | 40 | 170 | 110 | 70 |  |  |  |  | 600 |
|  | Sun | 30 | 155 | 100 | 65 |  |  |  |  | 30 | 155 | 100 | 65 |  |  |  |  | 600 |
|  | Weekday | 30 | 135 | 85 | 55 |  |  |  |  | 30 | 135 | 85 | 55 |  |  |  |  | 600 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2020 |  | Background Traffic VolumesAM |  |  |  | Site Traffic Volumes |  |  |  | Total Traffic Volumes |  |  |  | Volume/Capacity |  |  |  | Total Capacity |
|  |  | A |  | PN |  |  |  |  |  |  |  | P |  |  |
|  |  | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB |  |
| Hwy 11 | Fri |  |  |  |  | 2,685 | 1,335 | 2,980 | 1,610 |  |  |  |  | 2,685 | 1,335 | 2,980 | 1,610 |  |  |  |  | 3,270 |
|  | Sat | 2,660 | 1,675 | 2,615 | 1,730 |  |  |  |  | 2,660 | 1,675 | 2,615 | 1,730 |  |  |  |  | 3,270 |
|  | Sun | 1,855 | 2,340 | 1,815 | 2,745 |  |  |  |  | 1,855 | 2,340 | 1,815 | 2,745 |  |  |  |  | 3,270 |
|  | Weekday | 1,695 | 1,295 | 2,240 | 1,580 |  |  | 150 | 150 | 1,695 | 1,295 | 2,390 | 1,730 |  |  | 0.73 | 0.53 | 3,270 |
| SR 20 | Fri | 50 | 70 | 70 | 60 |  |  |  |  | 50 | 70 | 70 | 60 |  |  |  |  | 800 |
| Line 3-7 | Sat | 55 | 80 | 85 | 70 |  |  |  |  | 55 | 80 | 85 | 70 |  |  |  |  | 800 |
|  | Sun | 50 | 70 | 75 | 65 |  |  |  |  | 50 | 70 | 75 | 65 |  |  |  |  | 800 |
|  | Weekday | 50 | 65 | 70 | 55 |  |  | 25 | 25 | 50 | 65 | 95 | 80 |  |  | 0.12 | 0.10 | 800 |
| SR 20 | Fri | 45 | 65 | 75 | 60 |  |  |  |  | 45 | 65 | 75 | 60 |  |  |  |  | 800 |
| Line7-11 | Sat | 45 | 75 | 80 | 75 |  |  |  |  | 45 | 75 | 80 | 75 |  |  |  |  | 800 |
|  | Sun | 45 | 65 | 75 | 70 |  |  |  |  | 45 | 65 | 75 | 70 |  |  |  |  | 800 |
|  | Weekday | 40 | 60 | 65 | 60 |  |  | 25 | 25 | 40 | 60 | 90 | 85 |  |  | 0.11 | 0.11 | 800 |
| Line 7 | Fri | 70 | 195 | 140 | 100 |  |  |  |  | 70 | 195 | 140 | 100 |  |  |  |  | 600 |
|  | Sat | 80 | 245 | 165 | 120 |  |  |  |  | 80 | 245 | 165 | 120 |  |  |  |  | 600 |
|  | Sun | 75 | 215 | 150 | 105 |  |  |  |  | 75 | 215 | 150 | 105 |  |  |  |  | 600 |
|  | Weekday | 65 | 185 | 125 | 95 |  |  | 450 | 450 | 65 | 185 | 575 | 545 |  |  | 0.96 | 0.91 | 600 |
| Line 8 | Fri | 15 | 10 | 15 | 20 |  |  |  |  | 15 | 10 | 15 | 20 |  |  |  |  | 600 |
|  | Sat | 15 | 15 | 15 | 20 |  |  |  |  | 15 | 15 | 15 | 20 |  |  |  |  | 600 |
|  | Sun | 15 | 10 | 15 | 20 |  |  |  |  | 15 | 10 | 15 | 20 |  |  |  |  | 600 |
|  | Weekday | 15 | 10 | 10 | 20 |  |  |  |  | 15 | 10 | 10 | 20 |  |  |  |  | 600 |
| Line 9 | Fri | 40 | 160 | 105 | 70 |  |  |  |  | 40 | 160 | 105 | 70 |  |  |  |  | 600 |
|  | Sat | 50 | 195 | 125 | 85 |  |  |  |  | 50 | 195 | 125 | 85 |  |  |  |  | 600 |
|  | Sun | 40 | 175 | 115 | 75 |  |  |  |  | 40 | 175 | 115 | 75 |  |  |  |  | 600 |
|  | Weekday | 40 | 150 | 100 | 65 |  |  |  |  | 40 | 150 | 100 | 65 |  |  |  |  | 600 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2025 |  | Background Traffic VolumesAM |  |  |  | Site Traffic Volumes |  |  |  | Total Traffic Volumes |  |  |  | Volume/Capacity |  |  |  | Total Capacity |
|  |  |  |  |  |  | A |  | PN |  |  |  | PI |  |  |
|  |  | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB |  |
| Hwy 11 | Fri |  |  |  |  | 2,820 | 1,410 | 3,145 | 1,700 |  |  |  |  | 2,820 | 1,410 | 3,145 | 1,700 |  |  |  |  | 3,270 |
|  | Sat | 2,805 | 1,770 | 2,760 | 1,825 |  |  |  |  | 2,805 | 1,770 | 2,760 | 1,825 |  |  |  |  | 3,270 |
|  | Sun | 1,955 | 2,465 | 1,920 | 2,890 |  |  |  |  | 1,955 | 2,465 | 1,920 | 2,890 |  |  |  |  | 3,270 |
|  | Weekday | 1,695 | 1,370 | 2,240 | 1,580 |  |  | 150 | 150 | 1,695 | 1,370 | 2,390 | 1,730 |  |  | 0.73 | 0.53 | 3,270 |
| SR 20 | Fri | 50 | 75 | 75 | 65 |  |  |  |  | 50 | 75 | 75 | 65 |  |  |  |  | 800 |
| Line 3-7 | Sat | 55 | 90 | 95 | 75 |  |  |  |  | 55 | 90 | 95 | 75 |  |  |  |  | 800 |
|  | Sun | 50 | 75 | 85 | 70 |  |  |  |  | 50 | 75 | 85 | 70 |  |  |  |  | 800 |
|  | Weekday | 50 | 65 | 75 | 60 |  |  | 25 | 25 | 50 | 65 | 100 | 85 |  |  | 0.13 | 0.11 | 800 |
| SR 20 | Fri | 45 | 70 | 80 | 65 |  |  |  |  | 45 | 70 | 80 | 65 |  |  |  |  | 800 |
| Line7-11 | Sat | 45 | 80 | 85 | 80 |  |  |  |  | 45 | 80 | 85 | 80 |  |  |  |  | 800 |
|  | Sun | 45 | 70 | 80 | 75 |  |  |  |  | 45 | 70 | 80 | 75 |  |  |  |  | 800 |
|  | Weekday | 40 | 60 | 70 | 65 |  |  | 25 | 25 | 40 | 60 | 95 | 90 |  |  | 0.12 | 0.11 | 800 |
| Line 7 | Fri | 75 | 215 | 155 | 110 |  |  |  |  | 75 | 215 | 155 | 110 |  |  |  |  | 600 |
|  | Sat | 90 | 270 | 180 | 130 |  |  |  |  | 90 | 270 | 180 | 130 |  |  |  |  | 600 |
|  | Sun | 85 | 235 | 165 | 115 |  |  |  |  | 85 | 235 | 165 | 115 |  |  |  |  | 600 |
|  | Weekday | 70 | 205 | 135 | 105 |  |  | 450 | 450 | 70 | 205 | 585 | 555 |  |  | 0.98 | 0.93 | 600 |
| Line 8 | Fri | 15 | 10 | 15 | 20 |  |  |  |  | 15 | 10 | 15 | 20 |  |  |  |  | 600 |
|  | Sat | 15 | 15 | 15 | 20 |  |  |  |  | 15 | 15 | 15 | 20 |  |  |  |  | 600 |
|  | Sun | 15 | 10 | 15 | 20 |  |  |  |  | 15 | 10 | 15 | 20 |  |  |  |  | 600 |
|  | Weekday | 15 | 10 | 10 | 20 |  |  |  |  | 15 | 10 | 10 | 20 |  |  |  |  | 600 |
| Line 9 | Fri | 40 | 175 | 115 | 75 |  |  |  |  | 40 | 175 | 115 | 75 |  |  |  |  | 600 |
|  | Sat | 50 | 215 | 135 | 90 |  |  |  |  | 50 | 215 | 135 | 90 |  |  |  |  | 600 |
|  | Sun | 40 | 190 | 125 | 80 |  |  |  |  | 40 | 190 | 125 | 80 |  |  |  |  | 600 |
|  | Weekday | 40 | 170 | 110 | 70 |  |  |  |  | 40 | 170 | 110 | 70 |  |  |  |  | 600 |

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| 2015 |  | Background Traffic VolumesAM |  |  |  | Site Traffic Volumes |  |  |  | Total Traffic Volumes |  |  |  | Volume/Capacity |  |  |  | Total Capacity |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | PN |  | A |  | PN |  |  |  |  |  |  |
|  |  | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB |  |
| Hwy 11 | Fri |  |  |  |  | 2,540 | 1,260 | 2,820 | 1,525 |  |  |  |  | 2,540 | 1,260 | 2,820 | 1,525 |  |  |  |  | 3,270 |
|  | Sat | 2,520 | 1,585 | 2,475 | 1,640 |  |  | 200 | 120 | 2,520 | 1,585 | 2,675 | 1,760 |  |  | 0.82 | 0.54 | 3,270 |
|  | Sun | 1,755 | 2,215 | 1,715 | 2,605 |  |  | 200 | 120 | 1,755 | 2,215 | 1,915 | 2,725 |  |  | 0.59 | 0.83 | 3,270 |
|  | Weekday | 1,520 | 1,225 | 2,005 | 1,420 |  |  |  |  | 1,520 | 1,225 | 2,005 | 1,420 |  |  |  |  | 3,270 |
| SR 20 Line 3-7 | Fri | 35 | 55 | 55 | 45 |  |  |  |  | 35 | 55 | 55 | 45 |  |  |  |  | 800 |
|  | Sat | 40 | 65 | 70 | 55 |  |  | 20 |  | 40 | 65 | 90 | 55 |  |  | 0.11 |  | 800 |
|  | Sun | 35 | 55 | 60 | 50 |  |  | 20 |  | 35 | 55 | 80 | 50 |  |  | 0.10 |  | 800 |
|  | Weekday | 35 | 50 | 55 | 40 |  |  |  |  | 35 | 50 | 55 | 40 |  |  |  |  | 800 |
| SR 20 | Fri | 30 | 50 | 60 | 45 |  |  |  |  | 30 | 50 | 60 | 45 |  |  |  |  | 800 |
| Line7-11 | Sat | 30 | 60 | 65 | 60 |  |  | 20 | 20 | 30 | 60 | 85 | 80 |  |  | 0.11 | 0.10 | 800 |
|  | Sun | 30 | 50 | 60 | 55 |  |  | 20 | 20 | 30 | 50 | 80 | 75 |  |  | 0.10 | 0.09 | 800 |
|  | Weekday | 25 | 45 | 50 | 45 |  |  |  |  | 25 | 45 | 50 | 45 |  |  |  |  | 800 |
| Line 7 | Fri | 60 | 175 | 120 | 85 |  |  |  |  | 60 | 175 | 120 | 85 |  |  |  |  | 600 |
|  | Sat | 70 | 215 | 145 | 105 |  |  |  |  | 70 | 215 | 145 | 105 |  |  |  |  | 600 |
|  | Sun | 65 | 190 | 130 | 90 |  |  |  |  | 65 | 190 | 130 | 90 |  |  |  |  | 600 |
|  | Weekday | 55 | 165 | 110 | 80 |  |  |  |  | 55 | 165 | 110 | 80 |  |  |  |  | 600 |
| Line 8 | Fri | 10 | 5 | 10 | 15 |  |  |  |  | 10 | 5 | 10 | 15 |  |  |  |  | 600 |
|  | Sat | 10 | 10 | 10 | 15 |  |  | 360 | 40 | 10 | 10 | 370 | 55 |  |  | 0.62 | 0.09 | 600 |
|  | Sun | 10 | 5 | 10 | 15 |  |  | 360 | 40 | 10 | 5 | 370 | 55 |  |  | 0.62 | 0.09 | 600 |
|  | Weekday | 10 | 5 | 5 | 15 |  |  |  |  | 10 | 5 | 5 | 15 |  |  |  |  | 600 |
| Line 9 | Fri | 30 | 140 | 90 | 60 |  |  |  |  | 30 | 140 | 90 | 60 |  |  |  |  | 600 |
|  | Sat | 40 | 170 | 110 | 70 |  |  |  |  | 40 | 170 | 110 | 70 |  |  |  |  | 600 |
|  | Sun | 30 | 155 | 100 | 65 |  |  |  |  | 30 | 155 | 100 | 65 |  |  |  |  | 600 |
|  | Weekday | 30 | 135 | 85 | 55 |  |  |  |  | 30 | 135 | 85 | 55 |  |  |  |  | 600 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2020 |  | Background Traffic VolumesAM |  |  |  | Site Traffic Volumes |  |  |  | Total Traffic Volumes |  |  |  | Volume/Capacity |  |  |  | Total Capacity |
|  |  | A |  | PN |  |  |  |  |  |  |  | PI |  |  |
|  |  | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB |  |
| Hwy 11 | Fri |  |  |  |  | 2,685 | 1,335 | 2,980 | 1,610 |  |  |  |  | 2,685 | 1,335 | 2,980 | 1,610 |  |  |  |  | 3,270 |
|  | Sat | 2,660 | 1,675 | 2,615 | 1,730 |  |  | 200 | 120 | 2,660 | 1,675 | 2,815 | 1,850 |  |  | 0.86 | 0.57 | 3,270 |
|  | Sun | 1,855 | 2,340 | 1,815 | 2,745 |  |  | 200 | 120 | 1,855 | 2,340 | 2,015 | 2,865 |  |  | 0.62 | 0.88 | 3,270 |
|  | Weekday | 1,695 | 1,295 | 2,240 | 1,580 |  |  |  |  | 1,695 | 1,295 | 2,240 | 1,580 |  |  |  |  | 3,270 |
| SR 20 | Fri | 50 | 70 | 70 | 60 |  |  |  |  | 50 | 70 | 70 | 60 |  |  |  |  | 800 |
| Line 3-7 | Sat | 55 | 80 | 85 | 70 |  |  | 20 |  | 55 | 80 | 105 | 70 |  |  | 0.13 |  | 800 |
|  | Sun | 50 | 70 | 75 | 65 |  |  | 20 |  | 50 | 70 | 95 | 65 |  |  | 0.12 |  | 800 |
|  | Weekday | 50 | 65 | 70 | 55 |  |  |  |  | 50 | 65 | 70 | 55 |  |  |  |  | 800 |
| $\begin{aligned} & \hline \text { SR } 20 \\ & \text { Line7-11 } \end{aligned}$ | Fri | 45 | 65 | 75 | 60 |  |  |  |  | 45 | 65 | 75 | 60 |  |  |  |  | 800 |
|  | Sat | 45 | 75 | 80 | 75 |  |  | 20 | 20 | 45 | 75 | 100 | 95 |  |  | 0.13 | 0.12 | 800 |
|  | Sun | 45 | 65 | 75 | 70 |  |  | 20 | 20 | 45 | 65 | 95 | 90 |  |  | 0.12 | 0.11 | 800 |
|  | Weekday | 40 | 60 | 65 | 60 |  |  |  |  | 40 | 60 | 65 | 60 |  |  |  |  | 800 |
| Line 7 | Fri | 70 | 195 | 140 | 100 |  |  |  |  | 70 | 195 | 140 | 100 |  |  |  |  | 600 |
|  | Sat | 80 | 245 | 165 | 120 |  |  |  |  | 80 | 245 | 165 | 120 |  |  |  |  | 600 |
|  | Sun | 75 | 215 | 150 | 105 |  |  |  |  | 75 | 215 | 150 | 105 |  |  |  |  | 600 |
|  | Weekday | 65 | 185 | 125 | 95 |  |  |  |  | 65 | 185 | 125 | 95 |  |  |  |  | 600 |
| Line 8 | Fri | 15 | 10 | 15 | 20 |  |  |  |  | 15 | 10 | 15 | 20 |  |  |  |  | 600 |
|  | Sat | 15 | 15 | 15 | 20 |  |  | 360 | 40 | 15 | 15 | 375 | 60 |  |  | 0.63 | 0.10 | 600 |
|  | Sun | 15 | 10 | 15 | 20 |  |  | 360 | 40 | 15 | 10 | 375 | 60 |  |  | 0.63 | 0.10 | 600 |
|  | Weekday | 15 | 10 | 10 | 20 |  |  |  |  | 15 | 10 | 10 | 20 |  |  |  |  | 600 |
| Line 9 | Fri | 40 | 160 | 105 | 70 |  |  |  |  | 40 | 160 | 105 | 70 |  |  |  |  | 600 |
|  | Sat | 50 | 195 | 125 | 85 |  |  |  |  | 50 | 195 | 125 | 85 |  |  |  |  | 600 |
|  | Sun | 40 | 175 | 115 | 75 |  |  |  |  | 40 | 175 | 115 | 75 |  |  |  |  | 600 |
|  | Weekday | 40 | 150 | 100 | 65 |  |  |  |  | 40 | 150 | 100 | 65 |  |  |  |  | 600 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2025 |  | Background Traffic VolumesAM |  |  |  | Site Traffic Volumes |  |  |  | Total Traffic Volumes |  |  |  | Volume/Capacity |  |  |  | Total Capacity |
|  |  |  |  | PM |  | A |  | PN |  |  | Volumer | PI |  |  |
|  |  | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB |  | NB/EB | SB/WB |  |
| Hwy 11 | Fri |  |  |  |  | 2,820 | 1,410 | 3,145 | 1,700 |  |  |  |  | 2,820 | 1,410 | 3,145 | 1,700 |  |  |  |  | 3,270 |
|  | Sat | 2,805 | 1,770 | 2,760 | 1,825 |  |  | 200 | 120 | 2,805 | 1,770 | 2,960 | 1,945 |  |  | 0.91 | 0.59 | 3,270 |
|  | Sun | 1,955 | 2,465 | 1,920 | 2,890 |  |  | 200 | 120 | 1,955 | 2,465 | 2,120 | 3,010 |  |  | 0.65 | 0.92 | 3,270 |
|  | Weekday | 1,695 | 1,370 | 2,240 | 1,580 |  |  |  |  | 1,695 | 1,370 | 2,240 | 1,580 |  |  |  |  | 3,270 |
| SR 20 | Fri | 50 | 75 | 75 | 65 |  |  |  |  | 50 | 75 | 75 | 65 |  |  |  |  | 800 |
| Line 3-7 | Sat | 55 | 90 | 95 | 75 |  |  | 20 |  | 55 | 90 | 115 | 75 |  |  | 0.14 |  | 800 |
|  | Sun | 50 | 75 | 85 | 70 |  |  | 20 |  | 50 | 75 | 105 | 70 |  |  | 0.13 |  | 800 |
|  | Weekday | 50 | 65 | 75 | 60 |  |  |  |  | 50 | 65 | 75 | 60 |  |  |  |  | 800 |
| SR 20 | Fri | 45 | 70 | 80 | 65 |  |  |  |  | 45 | 70 | 80 | 65 |  |  |  |  | 800 |
| Line7-11 | Sat | 45 | 80 | 85 | 80 |  |  | 20 | 20 | 45 | 80 | 105 | 100 |  |  | 0.13 | 0.13 | 800 |
|  | Sun | 45 | 70 | 80 | 75 |  |  | 20 | 20 | 45 | 70 | 100 | 95 |  |  | 0.13 | 0.12 | 800 |
|  | Weekday | 40 | 60 | 70 | 65 |  |  |  |  | 40 | 60 | 70 | 65 |  |  |  |  | 800 |
| Line 7 | Fri | 75 | 215 | 155 | 110 |  |  |  |  | 75 | 215 | 155 | 110 |  |  |  |  | 600 |
|  | Sat | 90 | 270 | 180 | 130 |  |  |  |  | 90 | 270 | 180 | 130 |  |  |  |  | 600 |
|  | Sun | 85 | 235 | 165 | 115 |  |  |  |  | 85 | 235 | 165 | 115 |  |  |  |  | 600 |
|  | Weekday | 70 | 205 | 135 | 105 |  |  |  |  | 70 | 205 | 135 | 105 |  |  |  |  | 600 |
| Line 8 | Fri | 15 | 10 | 15 | 20 |  |  |  |  | 15 | 10 | 15 | 20 |  |  |  |  | 600 |
|  | Sat | 15 | 15 | 15 | 20 |  |  | 360 | 40 | 15 | 15 | 375 | 60 |  |  | 0.63 | 0.10 | 600 |
|  | Sun | 15 | 10 | 15 | 20 |  |  | 360 | 40 | 15 | 10 | 375 | 60 |  |  | 0.63 | 0.10 | 600 |
|  | Weekday | 15 | 10 | 10 | 20 |  |  |  |  | 15 | 10 | 10 | 20 |  |  |  |  | 600 |
| Line 9 | Fri | 40 | 175 | 115 | 75 |  |  |  |  | 40 | 175 | 115 | 75 |  |  |  |  | 600 |
|  | Sat | 50 | 215 | 135 | 90 |  |  |  |  | 50 | 215 | 135 | 90 |  |  |  |  | 600 |
|  | Sun | 40 | 190 | 125 | 80 |  |  |  |  | 40 | 190 | 125 | 80 |  |  |  |  | 600 |
|  | Weekday | 40 | 170 | 110 | 70 |  |  |  |  | 40 | 170 | 110 | 70 |  |  |  |  | 600 |


| 2015 |  | $\underset{\text { Background Traffic Volumes }}{\text { PM }}$ |  |  |  | Site Traffic Volumes |  |  |  | Total Traffic Volumes |  |  |  | Volume/Capacity |  |  |  | TotalCapacity |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | A |  | PM |  | A) |  | PM |  | AM |  | ${ }^{\text {P }}{ }^{\text {a }}$ |  |  |
|  |  | NB/EB | SB/WB | NB/EB | SBMB | NB/EB | SBNB | NB/EB | SBWB | NB/EB | SBMB | NB/EB | SB/WB | NB/EB | SB/WB | NB/EB | SBMB |  |
| Hwy 11 | Fri |  |  |  |  | 2,540 | 1,260 | 2,820 | 1,525 |  |  |  |  | 2,540 | 1,260 | 2,820 | 1,525 |  |  |  |  | 3,270 |
|  | Sat | 2,520 | 1,585 | 2,475 | 1,640 | 300 | 300 | 300 | 300 | 2,820 | 1,885 | 2,775 | 1,940 | 0.86 | 0.58 | 0.85 | 0.59 | 3,270 |
|  | Sun | 1,755 | 2,215 | 1,715 | 2,605 | 300 | 300 | 300 | 300 | 2,055 | 2,515 | 2,015 | 2,905 | 0.63 | 0.77 | 0.62 | 0.89 | 3,270 |
|  | Weekday | 1,520 | 1,225 | 2,005 | 1,420 |  |  |  |  | 1,520 | 1,225 | 2,005 | 1,420 |  |  |  |  | 3,270 |
| SR 20 <br> Line 3-7 | Fri | 35 | 55 | 55 | 45 |  |  |  |  | 35 | 55 | 55 | 45 |  |  |  |  | 800 |
|  | Sat | 40 | 65 | 70 | 55 | 30 | 30 | 30 | 30 | 70 | 95 | 100 | 85 | 0.09 | 0.12 | 0.13 | 0.11 | 800 |
|  | Sun | 35 | 55 | 60 | 50 | 30 | 30 | 30 | 30 | 65 | 85 | 90 | 80 | 0.08 | 0.11 | 0.11 | 0.10 | 800 |
|  | Weekday | 35 | 50 | 55 | 40 |  |  |  |  | 35 | 50 | 55 | 40 |  |  |  |  | 800 |
| SR 20 | Fri | 30 | 50 | 60 | 45 |  |  |  |  | 30 | 50 | 60 | 45 |  |  |  |  | 800 |
| Line7-11 | Sat | 30 | 60 | 65 | 60 | 30 | 30 | 30 | 30 | 60 | 90 | 95 | 90 | 0.08 | 0.11 | 0.12 | 0.11 | 800 |
|  | Sun | 30 | 50 | 60 | 55 | 30 | 30 | 30 | 30 | 60 | 80 | 90 | 85 | 0.08 | 0.10 | 0.11 | 0.11 | 800 |
|  | Weekday | 25 | 45 | 50 | 45 |  |  |  |  | 25 | 45 | 50 | 45 |  |  |  |  | 800 |
| Line 7 | Fri | 60 | 175 | 120 | 85 |  |  |  |  | 60 | 175 | 120 | 85 |  |  |  |  | 600 |
|  | Sat | 70 | 215 | 145 | 105 | 225 | 270 | 225 | 270 | 295 | 485 | 370 | 375 | 0.49 | 0.81 | 0.62 | 0.63 | 600 |
|  | Sun | 65 | 190 | 130 | 90 | 225 | 270 | 225 | 270 | 290 | 460 | 355 | 360 | 0.48 | 0.77 | 0.59 | 0.60 | 600 |
|  | Weekday | 55 | 165 | 110 | 80 |  |  |  |  | 55 | 165 | 110 | 80 |  |  |  |  | 600 |
| Line 8 | Fri | 10 | 5 | 10 | 15 |  |  |  |  | 10 | 5 | 10 | 15 |  |  |  |  | 600 |
|  | Sat | 10 | 10 | 10 | 15 | 225 | 270 | 225 | 270 | 235 | 280 | 235 | 285 | 0.39 | 0.47 | 0.39 | 0.48 | 600 |
|  | Sun | 10 | 5 | 10 | 15 | 225 | 270 | 225 | 270 | 235 | 275 | 235 | 285 | 0.39 | 0.46 | 0.39 | 0.48 | 600 |
|  | Weekday | 10 | 5 | 5 | 15 |  |  |  |  | 10 | 5 | 5 | 15 |  |  |  |  | 600 |
| Line 9 | Fri | 30 | 140 | 90 | 60 |  |  |  |  | 30 | 140 | 90 | 60 |  |  |  |  | 600 |
|  | Sat | 40 | 170 | 110 | 70 | 180 |  | 180 |  | 220 | 170 | 290 | 70 | 0.37 |  | 0.48 |  | 600 |
|  | Sun | 30 | 155 | 100 | 65 | 180 |  | 180 |  | 210 | 155 | 280 | 65 | 0.35 |  | 0.47 |  | 600 |
|  | Weekday | 30 | 135 | 85 | 55 |  |  |  |  | 30 | 135 | 85 | 55 |  |  |  |  | 600 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2020 |  | $\underset{\text { Background Traffic Volumes }}{\text { PM }}$ |  |  |  | Site Traffic Volumes |  |  |  | Total Traffic Volumes |  |  |  | Volume/Capacity |  |  |  | $\begin{aligned} & \text { Total } \\ & \text { Capacity } \end{aligned}$ |
|  |  | AM |  | P |  | AM |  | PM |  | AN |  | PM |  |  |
|  |  | NB/EB | SBMB | NB/EB | SBMB | NB/EB | SB/WB | NB/EB | SBNB | NB/EB | SBNB | NB/EB | SBMB | NB/EB | SBMB | NB/EB | SBMB |  |
| Hwy 11 | Fri |  |  |  |  | 2,685 | 1,335 | 2,980 | 1,610 |  |  |  |  | 2,685 | 1,335 | 2,980 | 1,610 |  |  |  |  | 3,270 |
|  | Sat | 2,660 | 1,675 | 2,615 | 1,730 | 300 | 300 | 300 | 300 | 2,960 | 1,975 | 2,915 | 2,030 | 0.91 | 0.60 | 0.89 | 0.62 | 3,270 |
|  | Sun | 1,855 | 2,340 | 1,815 | 2,745 | 300 | 300 | 300 | 300 | 2,155 | 2,640 | 2,115 | 3,045 | 0.66 | 0.81 | 0.65 | 0.93 | 3,270 |
|  | Weekday | 1,695 | 1,295 | 2,240 | 1,580 |  |  |  |  | 1,695 | 1,295 | 2,240 | 1,580 |  |  |  |  | 3,270 |
| SR 20 | Fri | 50 | 70 | 70 | 60 |  |  |  |  | 50 | 70 | 70 | 60 |  |  |  |  | 800 |
| Line 3-7 | Sat | 55 | 80 | 85 | 70 | 30 | 30 | 30 | 30 | 85 | 110 | 115 | 100 | 0.11 | 0.14 | 0.14 | 0.13 | 800 |
|  | Sun | 50 | 70 | 75 | 65 | 30 | 30 | 30 | 30 | 80 | 100 | 105 | 95 | 0.10 | 0.13 | 0.13 | 0.12 | 800 |
|  | Weekday | 50 | 65 | 70 | 55 |  |  |  |  | 50 | 65 | 70 | 55 |  |  |  |  | 800 |
| SR 20 Line7-11 | Fri | 45 | 65 | 75 | 60 |  |  |  |  | 45 | 65 | 75 | 60 |  |  |  |  | 800 |
|  | Sat | 45 | 75 | 80 | 75 | 30 | 30 | 30 | 30 | 75 | 105 | 110 | 105 | 0.09 | 0.13 | 0.14 | 0.13 | 800 |
|  | Sun | 45 | 65 | 75 | 70 | 30 | 30 | 30 | 30 | 75 | 95 | 105 | 100 | 0.09 | 0.12 | 0.13 | 0.13 | 800 |
|  | Weekday | 40 | 60 | 65 | 60 |  |  |  |  | 40 | 60 | 65 | 60 |  |  |  |  | 800 |
| Line 7 | Fri | 70 | 195 | 140 | 100 |  |  |  |  | 70 | 195 | 140 | 100 |  |  |  |  | 600 |
|  | Sat | 80 | 245 | 165 | 120 | 225 | 270 | 225 | 270 | 305 | 515 | 390 | 390 | 0.51 | 0.86 | 0.65 | 0.65 | 600 |
|  | Sun | 75 | 215 | 150 | 105 | 225 | 270 | 225 | 270 | 300 | 485 | 375 | 375 | 0.50 | 0.81 | 0.63 | 0.63 | 600 |
|  | Weekday | 65 | 185 | 125 | 95 |  |  |  |  | 65 | 185 | 125 | 95 |  |  |  |  | 600 |
| Line 8 | Fri | 15 | 10 | 15 | 20 |  |  |  |  | 15 | 10 | 15 | 20 |  |  |  |  | 600 |
|  | Sat | 15 | 15 | 15 | 20 | 225 | 270 | 225 | 270 | 240 | 285 | 240 | 290 | 0.40 | 0.48 | 0.40 | 0.48 | 600 |
|  | Sun | 15 | 10 | 15 | 20 | 225 | 270 | 225 | 270 | 240 | 280 | 240 | 290 | 0.40 | 0.47 | 0.40 | 0.48 | 600 |
|  | Weekday | 15 | 10 | 10 | 20 |  |  |  |  | 15 | 10 | 10 | 20 |  |  |  |  | 600 |
| Line 9 | Fri | 40 | 160 | 105 | 70 |  |  |  |  | 40 | 160 | 105 | 70 |  |  |  |  | 600 |
|  | Sat | 50 | 195 | 125 | 85 | 180 |  | 180 |  | 230 | 195 | 305 | 85 | 0.38 |  | 0.51 |  | 600 |
|  | Sun | 40 | 175 | 115 | 75 | 180 |  | 180 |  | 220 | 175 | 295 | 75 | 0.37 |  | 0.49 |  | 600 |
|  | Weekday | 40 | 150 | 100 | 65 |  |  |  |  | 40 | 150 | 100 | 65 |  |  |  |  | 600 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2025 |  | Background Traffic Volumes |  |  |  | Site Traffic Volumes |  |  |  | Total Traffic Volumes |  |  |  | Volume/Capacity |  |  |  | $\begin{aligned} & \text { Total } \\ & \text { Capacity } \end{aligned}$ |
|  |  | A |  | PI |  | AM |  | PM |  | AM |  | PN |  | AM |  | PM |  |  |
|  |  | NB/EB | SBMB | NB/EB | SBMB | NB/EB | SB/WB | NB/EB | SBNB | NB/EB | SBMB | NB/EB | SBMB | NB/EB | SBMB | NB/EB | SBMB |  |
| Hwy 11 | Fri | 2,820 | 1,410 | 3,145 | 1,700 |  |  |  |  | 2,820 | 1,410 | 3,145 | 1,700 |  |  |  |  | 3,270 |
|  | Sat | 2,805 | 1,770 | 2,760 | 1,825 | 300 | 300 | 300 | 300 | 3,105 | 2,070 | 3,060 | 2,125 | 0.95 | 0.63 | 0.94 | 0.65 | 3,270 |
|  | Sun | 1,955 | 2,465 | 1,920 | 2,890 | 300 | 300 | 300 | 300 | 2,255 | 2,765 | 2,220 | 3,190 | 0.69 | 0.85 | 0.68 | 0.98 | 3,270 |
|  | Weekday | 1,695 | 1,370 | 2,240 | 1,580 |  |  |  |  | 1,695 | 1,370 | 2,240 | 1,580 |  |  |  |  | 3,270 |
| SR 20 | Fri | 50 | 75 | 75 | 65 |  |  |  |  | 50 | 75 | 75 | 65 |  |  |  |  | 800 |
| Line 3-7 | Sat | 55 | 90 | 95 | 75 | 30 | 30 | 30 | 30 | 85 | 120 | 125 | 105 | 0.11 | 0.15 | 0.16 | 0.13 | 800 |
|  | Sun | 50 | 75 | 85 | 70 | 30 | 30 | 30 | 30 | 80 | 105 | 115 | 100 | 0.10 | 0.13 | 0.14 | 0.13 | 800 |
|  | Weekday | 50 | 65 | 75 | 60 |  |  |  |  | 50 | 65 | 75 | 60 |  |  |  |  | 800 |
| SR 20 | Fri | 45 | 70 | 80 | 65 |  |  |  |  | 45 | 70 | 80 | 65 |  |  |  |  | 800 |
| Line7-11 | Sat | 45 | 80 | 85 | 80 | 30 | 30 | 30 | 30 | 75 | 110 | 115 | 110 | 0.09 | 0.14 | 0.14 | 0.14 | 800 |
|  | Sun | 45 | 70 | 80 | 75 | 30 | 30 | 30 | 30 | 75 | 100 | 110 | 105 | 0.09 | 0.13 | 0.14 | 0.13 | 800 |
|  | Weekday | 40 | 60 | 70 | 65 |  |  |  |  | 40 | 60 | 70 | 65 |  |  |  |  | 800 |
| Line 7 | Fri | 75 | 215 | 155 | 110 |  |  |  |  | 75 | 215 | 155 | 110 |  |  |  |  | 600 |
|  | Sat | 90 | 270 | 180 | 130 | 225 | 270 | 225 | 270 | 315 | 540 | 405 | 400 | 0.53 | 0.90 | 0.68 | 0.67 | 600 |
|  | Sun | 85 | 235 | 165 | 115 | 225 | 270 | 225 | 270 | 310 | 505 | 390 | 385 | 0.52 | 0.84 | 0.65 | 0.64 | 600 |
|  | Weekday | 70 | 205 | 135 | 105 |  |  |  |  | 70 | 205 | 135 | 105 |  |  |  |  | 600 |
| Line 8 | Fri | 15 | 10 | 15 | 20 |  |  |  |  | 15 | 10 | 15 | 20 |  |  |  |  | 600 |
|  | Sat | 15 | 15 | 15 | 20 | 225 | 270 | 225 | 270 | 240 | 285 | 240 | 290 | 0.40 | 0.48 | 0.40 | 0.48 | 600 |
|  | Sun | 15 | 10 | 15 | 20 | 225 | 270 | 225 | 270 | 240 | 280 | 240 | 290 | 0.40 | 0.47 | 0.40 | 0.48 | 600 |
|  | Weekday | 15 | 10 | 10 | 20 |  |  |  |  | 15 | 10 | 10 | 20 |  |  |  |  | 600 |
| Line 9 | Fri | 40 | 175 | 115 | 75 |  |  |  |  | 40 | 175 | 115 | 75 |  |  |  |  | 600 |
|  | Sat | 50 | 215 | 135 | 90 | 180 |  | 180 |  | 230 | 215 | 315 | 90 | 0.38 |  | 0.53 |  | 600 |
|  | Sun | 40 | 190 | 125 | 80 | 180 |  | 180 |  | 220 | 190 | 305 | 80 | 0.37 |  | 0.51 |  | 600 |
|  | Weekday | 40 | 170 | 110 | 70 |  |  |  |  | 40 | 170 | 110 | 70 |  |  |  |  | 600 |

APPENDIX F: 2025 TRAFFIC OPERATIONS WITH BURL'S CREEK



|  | 4 |  |  | 7 | $\leftarrow$ | 4 | 4 | $\dagger$ | 7 | $\downarrow$ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | ¢ |  |  | ¢ |  |  | ¢ |  |
| Sign Control |  | Stop |  |  | Stop |  |  | Stop |  |  | Stop |  |
| Volume (vph) | 25 | 15 | 20 | 10 | 35 | 35 | 25 | 145 | 15 | 25 | 25 | 25 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 27 | 16 | 22 | 11 | 38 | 38 | 27 | 158 | 16 | 27 | 27 | 27 |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total (vph) | 65 | 87 | 201 | 82 |  |  |  |  |  |  |  |  |
| Volume Left (vph) | 27 | 11 | 27 | 27 |  |  |  |  |  |  |  |  |
| Volume Right (vph) | 22 | 38 | 16 | 27 |  |  |  |  |  |  |  |  |
| Hadj (s) | -0.08 | -0.20 | 0.01 | -0.10 |  |  |  |  |  |  |  |  |
| Departure Headway (s) | 4.6 | 4.4 | 4.4 | 4.4 |  |  |  |  |  |  |  |  |
| Degree Utilization, x | 0.08 | 0.11 | 0.24 | 0.10 |  |  |  |  |  |  |  |  |
| Capacity (veh/h) | 727 | 753 | 793 | 774 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 8.0 | 8.0 | 8.8 | 7.9 |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 8.0 | 8.0 | 8.8 | 7.9 |  |  |  |  |  |  |  |  |
| Approach LOS | A | A | A | A |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Delay |  |  | 8.3 |  |  |  |  |  |  |  |  |  |
| Level of Service |  |  | A |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 25.8\% |  | CU Level o | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |







|  | $\stackrel{ }{*}$ |  |  | 7 |  |  |  | $\dagger$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\uparrow$ |  |  | $\uparrow$ |  |  | \$ |  |  | \$ |  |
| Volume (veh/h) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 0 | 0 | 15 | 0 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 11 | 0 | 0 | 16 | 0 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal ( $m$ ) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| VC , conflicting volume | 27 | 27 | 16 | 27 | 27 | 11 | 16 |  |  | 11 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| $\mathrm{vCu}, \mathrm{unblocked} \mathrm{vol}$ | 27 | 27 | 16 | 27 | 27 | 11 | 16 |  |  | 11 |  |  |
| tC , single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| $\mathrm{tC}, 2$ stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 100 | 100 | 100 | 100 | 100 | 100 | 100 |  |  | 100 |  |  |
| cM capacity (veh/h) | 983 | 866 | 1063 | 983 | 866 | 1070 | 1601 |  |  | 1608 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 0 | 0 | 11 | 16 |  |  |  |  |  |  |  |  |
| Volume Left | 0 | 0 | 0 | 0 |  |  |  |  |  |  |  |  |
| Volume Right | 0 | 0 | 0 | 0 |  |  |  |  |  |  |  |  |
| cSH | 1700 | 1700 | 1601 | 1608 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.00 | 0.00 | 0.00 | 0.00 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.0 | 0.0 | 0.0 | 0.0 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 |  |  |  |  |  |  |  |  |
| Lane LOS | A | A |  |  |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 |  |  |  |  |  |  |  |  |
| Approach LOS | A | A |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 0.0 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 6.7\% |  | CU Level | f Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |





|  | $\rangle$ | $\rightarrow$ | 7 | 7 | $\leftarrow$ | 4 | 4 | $\uparrow$ | $p$ | $\checkmark$ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | ¢ |  |  | ¢ |  |  | $\uparrow$ |  |  | ¢ |  |
| Sign Control |  | Stop |  |  | Stop |  |  | Stop |  |  | Stop |  |
| Volume (vph) | 10 | 45 | 25 | 15 | 40 | 15 | 15 | 60 | 15 | 25 | 105 | 15 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 11 | 49 | 27 | 16 | 43 | 16 | 16 | 65 | 16 | 27 | 114 | 16 |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total (vph) | 87 | 76 | 98 | 158 |  |  |  |  |  |  |  |  |
| Volume Left (vph) | 11 | 16 | 16 | 27 |  |  |  |  |  |  |  |  |
| Volume Right (vph) | 27 | 16 | 16 | 16 |  |  |  |  |  |  |  |  |
| Hadj (s) | -0.13 | -0.05 | -0.03 | 0.01 |  |  |  |  |  |  |  |  |
| Departure Headway (s) | 4.5 | 4.5 | 4.4 | 4.4 |  |  |  |  |  |  |  |  |
| Degree Utilization, x | 0.11 | 0.10 | 0.12 | 0.19 |  |  |  |  |  |  |  |  |
| Capacity (veh/h) | 750 | 737 | 771 | 777 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 8.0 | 8.0 | 8.0 | 8.5 |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 8.0 | 8.0 | 8.0 | 8.5 |  |  |  |  |  |  |  |  |
| Approach LOS | A | A | A | A |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Delay |  |  | 8.2 |  |  |  |  |  |  |  |  |  |
| Level of Service |  |  | A |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 23.6\% |  | CU Level of | f Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |





|  | 4 | $\rightarrow$ | $\cdots$ | $\bigcirc$ |  | 4 | 4 | $\dagger$ | 7 |  | $\dagger$ | $\pm$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \& |  |  | * |  |  | \& |  |  | \& |  |
| Volume (veh/h) | 15 | 45 | 25 | 20 | 40 | 15 | 15 | 60 | 15 | 15 | 85 | 15 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 16 | 49 | 27 | 22 | 43 | 16 | 16 | 65 | 16 | 16 | 92 | 16 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  | None |  |  | None |  |  |  |  |  |  |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 60 |  |  | 76 |  |  | 253 | 198 | 62 | 239 | 204 | 52 |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu, unblocked vol | 60 |  |  | 76 |  |  | 253 | 198 | 62 | 239 | 204 | 52 |
| tC, single (s) | 4.1 |  |  | 4.1 |  |  | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 2.2 |  |  | 2.2 |  |  | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free \% | 99 |  |  | 99 |  |  | 97 | 90 | 98 | 97 | 86 | 98 |
| cM capacity (veh/h) | 1544 |  |  | 1523 |  |  | 606 | 680 | 1002 | 640 | 675 | 1016 |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 92 | 82 | 98 | 125 |  |  |  |  |  |  |  |  |
| Volume Left | 16 | 22 | 16 | 16 |  |  |  |  |  |  |  |  |
| Volume Right |  | 16 | 16 | 16 |  |  |  |  |  |  |  |  |
| cSH | 1544 | 1523 | 703 | 701 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.01 | 0.01 | 0.14 | 0.18 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.2 | 0.3 | 3.7 | 4.9 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 1.4 | 2.1 | 10.9 | 11.2 |  |  |  |  |  |  |  |  |
|  | A | A | B | B |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 1.4 | 2.1 | 10.9 | 11.2 |  |  |  |  |  |  |  |  |
| Approach LOS |  |  | B | B |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 7.0 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 21.0\% |  | CU Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |







|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |





|  | 4 |  |  | $\downarrow$ |  |  | 4 | $\uparrow$ | 1 |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 4 |  |  | \$ |  |  | \$ |  |  | ¢ |  |
| Volume (veh/h) | 10 | 40 | 20 | 15 | 50 | 10 | 10 | 50 | 10 | 10 | 80 | 10 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 11 | 43 | 22 | 16 | 54 | 11 | 11 | 54 | 11 | 11 | 87 | 11 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  | None |  |  | None |  |  |  |  |  |  |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| VC , conflicting volume | 65 |  |  | 65 |  |  | 223 | 174 | 54 | 207 | 179 | 60 |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC2, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu, unblocked vol | 65 |  |  | 65 |  |  | 223 | 174 | 54 | 207 | 179 | 60 |
| tC, single (s) | 4.1 |  |  | 4.1 |  |  | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| $\mathrm{tC}, 2$ stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 2.2 |  |  | 2.2 |  |  | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free \% | 99 |  |  | 99 |  |  | 98 | 92 | 99 | 98 | 88 | 99 |
| cM capacity (veh/h) | 1537 |  |  | 1537 |  |  | 648 | 707 | 1013 | 690 | 702 | 1006 |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB1 |  |  |  |  |  |  |  |  |
| Volume Total | 76 | 82 | 76 | 109 |  |  |  |  |  |  |  |  |
| Volume Left | 11 | 16 | 11 | 11 |  |  |  |  |  |  |  |  |
| Volume Right | 22 | 11 | 11 | 11 |  |  |  |  |  |  |  |  |
| CSH | 1537 | 1537 | 729 | 722 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.01 | 0.01 | 0.10 | 0.15 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.2 | 0.2 | 2.6 | 4.0 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 1.1 | 1.5 | 10.5 | 10.9 |  |  |  |  |  |  |  |  |
| Lane LOS | A | A | B | B |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 1.1 | 1.5 | 10.5 | 10.9 |  |  |  |  |  |  |  |  |
| Approach LOS |  |  | B | B |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 6.4 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 19.4\% |  | CU Level | f Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |



|  | 4 | $\rightarrow$ |  | 7 |  | 4 | 4 | $\dagger$ | \% |  | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | \$ |  |  | \$ |  |  | \$ |  |
| Volume (veh/h) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 20 | 0 | 0 | 10 | 0 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 22 | 0 | 0 | 11 | 0 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC, conflicting volume | 33 | 33 | 11 | 33 | 33 | 22 | 11 |  |  | 22 |  |  |
| vC 1 , stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu , unblocked vol | 33 | 33 | 11 | 33 | 33 | 22 | 11 |  |  | 22 |  |  |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 100 | 100 | 100 | 100 | 100 | 100 | 100 |  |  | 100 |  |  |
| cM capacity (veh/h) | 975 | 860 | 1070 | 975 | 860 | 1055 | 1608 |  |  | 1594 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 0 | 0 | 22 | 11 |  |  |  |  |  |  |  |  |
| Volume Left | 0 | 0 | 0 | 0 |  |  |  |  |  |  |  |  |
| Volume Right | 0 | 0 | 0 | 0 |  |  |  |  |  |  |  |  |
| cSH | 1700 | 1700 | 1608 | 1594 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.10 | 0.00 | 0.00 | 0.00 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.0 | 0.0 | 0.0 | 0.0 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 |  |  |  |  |  |  |  |  |
| Lane LOS | A | A |  |  |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 |  |  |  |  |  |  |  |  |
| Approach LOS | A | A |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 0.0 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 6.7\% |  | Leve | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |





|  | 4 |  |  | 7 | $\leftarrow$ | 4 | 4 | $\uparrow$ | $>$ | * | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | ¢ |  |  | ¢ |  |  | ¢ |  |  | ¢ |  |
| Sign Control |  | Stop |  |  | Stop |  |  | Stop |  |  | Stop |  |
| Volume (vph) | 15 | 40 | 25 | 15 | 35 | 20 | 10 | 60 | 10 | 45 | 100 | 40 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 16 | 43 | 27 | 16 | 38 | 22 | 11 | 65 | 11 | 49 | 109 | 43 |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total (vph) | 87 | 76 | 87 | 201 |  |  |  |  |  |  |  |  |
| Volume Left (vph) | 16 | 16 | 11 | 49 |  |  |  |  |  |  |  |  |
| Volume Right (vph) | 27 | 22 | 11 | 43 |  |  |  |  |  |  |  |  |
| Hadj (s) | -0.12 | -0.09 | -0.02 | -0.05 |  |  |  |  |  |  |  |  |
| Departure Headway (s) | 4.5 | 4.6 | 4.5 | 4.3 |  |  |  |  |  |  |  |  |
| Degree Utilization, x | 0.11 | 0.10 | 0.11 | 0.24 |  |  |  |  |  |  |  |  |
| Capacity (veh/h) | 733 | 727 | 758 | 790 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 8.1 | 8.1 | 8.0 | 8.7 |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 8.1 | 8.1 | 8.0 | 8.7 |  |  |  |  |  |  |  |  |
| Approach LOS | A | A | A | A |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Delay |  |  | 8.4 |  |  |  |  |  |  |  |  |  |
| Level of Service |  |  | A |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 29.2\% |  | CU Level of | f Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |





|  | $\stackrel{ }{*}$ |  |  | 1 | - |  | 4 | $\dagger$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\uparrow$ |  |  | $\uparrow$ |  |  | ${ }_{*}$ |  |  | \$ |  |
| Volume (veh/h) | 10 | 60 | 20 | 15 | 30 | 10 | 10 | 50 | 10 | 10 | 80 | 10 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 11 | 65 | 22 | 16 | 33 | 11 | 11 | 54 | 11 | 11 | 87 | 11 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  | None |  |  | None |  |  |  |  |  |  |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| VC , conflicting volume | 43 |  |  | 87 |  |  | 223 | 174 | 76 | 207 | 179 | 38 |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu, unblocked vol | 43 |  |  | 87 |  |  | 223 | 174 | 76 | 207 | 179 | 38 |
| tC , single (s) | 4.1 |  |  | 4.1 |  |  | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 2.2 |  |  | 2.2 |  |  | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free \% | 99 |  |  | 99 |  |  | 98 | 92 | 99 | 98 | 88 | 99 |
| cM capacity (veh/h) | 1565 |  |  | 1509 |  |  | 648 | 707 | 985 | 690 | 702 | 1034 |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 98 | 60 | 76 | 109 |  |  |  |  |  |  |  |  |
| Volume Left | 11 | 16 | 11 | 11 |  |  |  |  |  |  |  |  |
| Volume Right | 22 | 11 | 11 | 11 |  |  |  |  |  |  |  |  |
| cSH | 1565 | 1509 | 727 | 724 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.01 | 0.01 | 0.10 | 0.15 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.2 | 0.2 | 2.7 | 4.0 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 0.9 | 2.1 | 10.5 | 10.9 |  |  |  |  |  |  |  |  |
| Lane LOS | A | A | B | B |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 0.9 | 2.1 | 10.5 | 10.9 |  |  |  |  |  |  |  |  |
| Approach LOS |  |  | B | B |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 6.4 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 19.1\% | ICU Level of Service |  |  |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |



|  | $\stackrel{ }{ }$ |  |  | 7 |  |  | 4 | $\dagger$ | 7 | * | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\dagger$ |  |  | $\uparrow$ |  |  | ${ }_{\$}$ |  |  | ${ }_{\$}$ |  |
| Volume (veh/h) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 20 | 0 | 0 | 10 | 0 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 22 | 0 | 0 | 11 | 0 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| VC , conflicting volume | 33 | 33 | 11 | 33 | 33 | 22 | 11 |  |  | 22 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC2, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| $\mathrm{vCu}, \mathrm{unblocked} \mathrm{vol}$ | 33 | 33 | 11 | 33 | 33 | 22 | 11 |  |  | 22 |  |  |
| tC, single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| $\mathrm{tC}, 2$ stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 100 | 100 | 100 | 100 | 100 | 100 | 100 |  |  | 100 |  |  |
| cM capacity (veh/h) | 975 | 860 | 1070 | 975 | 860 | 1055 | 1608 |  |  | 1594 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 0 | 0 | 22 | 11 |  |  |  |  |  |  |  |  |
| Volume Left | 0 | 0 | 0 | 0 |  |  |  |  |  |  |  |  |
| Volume Right | 0 | 0 | 0 | 0 |  |  |  |  |  |  |  |  |
| cSH | 1700 | 1700 | 1608 | 1594 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.10 | 0.00 | 0.00 | 0.00 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.0 | 0.0 | 0.0 | 0.0 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 |  |  |  |  |  |  |  |  |
| Lane LOS | A | A |  |  |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 |  |  |  |  |  |  |  |  |
| Approach LOS | A | A |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 0.0 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 6.7\% |  | U Level | f Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |





|  | $\rangle$ | $\rightarrow$ |  | 7 | $\leftarrow$ | 4 | 4 | $\dagger$ | > | $\checkmark$ | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | \$ |  |  | \$ |  |  | ¢ |  |
| Sign Control |  | Stop |  |  | Stop |  |  | Stop |  |  | Stop |  |
| Volume (vph) | 15 | 15 | 25 | 10 | 35 | 30 | 30 | 160 | 15 | 15 | 30 | 15 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 16 | 16 | 27 | 11 | 38 | 33 | 33 | 174 | 16 | 16 | 33 | 16 |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB1 |  |  |  |  |  |  |  |  |
| Volume Total (vph) | 60 | 82 | 223 | 65 |  |  |  |  |  |  |  |  |
| Volume Left (vph) | 16 | 11 | 33 | 16 |  |  |  |  |  |  |  |  |
| Volume Right (vph) | 27 | 33 | 16 | 16 |  |  |  |  |  |  |  |  |
| Hadj (s) | -0.18 | -0.18 | 0.02 | -0.07 |  |  |  |  |  |  |  |  |
| Departure Headway (s) | 4.5 | 4.5 | 4.3 | 4.4 |  |  |  |  |  |  |  |  |
| Degree Utilization, x | 0.07 | 0.10 | 0.27 | 0.08 |  |  |  |  |  |  |  |  |
| Capacity (veh/h) | 741 | 749 | 803 | 770 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 7.8 | 7.9 | 8.9 | 7.8 |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 7.8 | 7.9 | 8.9 | 7.8 |  |  |  |  |  |  |  |  |
| Approach LOS | A | A | A | A |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Delay |  |  | 8.4 |  |  |  |  |  |  |  |  |  |
| Level of Service |  |  | A |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 25.1\% |  | ICU Level | f Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |


|  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
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|  | 4 |  |  | 7 | $\leftarrow$ | 4 | 4 | $\dagger$ | / | $\downarrow$ | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | $\uparrow$ |  |  | \$ |  |  | ${ }_{\text {¢ }}$ |  |
| Sign Control |  | Stop |  |  | Stop |  |  | Stop |  |  | Stop |  |
| Volume (vph) | 15 | 40 | 30 | 20 | 35 | 20 | 15 | 70 | 15 | 25 | 125 | 20 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 16 | 43 | 33 | 22 | 38 | 22 | 16 | 76 | 16 | 27 | 136 | 22 |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB1 |  |  |  |  |  |  |  |  |
| Volume Total (vph) | 92 | 82 | 109 | 185 |  |  |  |  |  |  |  |  |
| Volume Left (vph) | 16 | 22 | 16 | 27 |  |  |  |  |  |  |  |  |
| Volume Right (vph) | 33 | 22 | 16 | 22 |  |  |  |  |  |  |  |  |
| Hadj (s) | -0.14 | -0.07 | -0.03 | -0.01 |  |  |  |  |  |  |  |  |
| Departure Headway (s) | 4.5 | 4.6 | 4.5 | 4.4 |  |  |  |  |  |  |  |  |
| Degree Utilization, x | 0.12 | 0.10 | 0.14 | 0.23 |  |  |  |  |  |  |  |  |
| Capacity (veh/h) | 732 | 719 | 757 | 771 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 8.1 | 8.2 | 8.2 | 8.7 |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 8.1 | 8.2 | 8.2 | 8.7 |  |  |  |  |  |  |  |  |
| Approach LOS | A | A | A | A |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Delay |  |  | 8.4 |  |  |  |  |  |  |  |  |  |
| Level of Service |  |  | A |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 25.4\% |  | CU Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |





|  | 4 |  |  | 7 |  |  | 4 | $\uparrow$ | 7 |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 4 |  |  | \$ |  |  | \$ |  |  | ¢ |  |
| Volume (veh/h) | 15 | 40 | 25 | 20 | 35 | 15 | 15 | 65 | 15 | 15 | 100 | 15 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 16 | 43 | 27 | 22 | 38 | 16 | 16 | 71 | 16 | 16 | 109 | 16 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  | None |  |  | None |  |  |  |  |  |  |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| VC , conflicting volume | 54 |  |  | 71 |  |  | 250 | 188 | 57 | 231 | 193 | 46 |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC2, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu, unblocked vol | 54 |  |  | 71 |  |  | 250 | 188 | 57 | 231 | 193 | 46 |
| tC, single (s) | 4.1 |  |  | 4.1 |  |  | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| $\mathrm{tC}, 2$ stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 2.2 |  |  | 2.2 |  |  | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free \% | 99 |  |  | 99 |  |  | 97 | 90 | 98 | 97 | 84 | 98 |
| cM capacity (veh/h) | 1551 |  |  | 1530 |  |  | 597 | 690 | 1009 | 644 | 685 | 1023 |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 87 | 76 | 103 | 141 |  |  |  |  |  |  |  |  |
| Volume Left | 16 | 22 | 16 | 16 |  |  |  |  |  |  |  |  |
| Volume Right | 27 | 16 | 16 | 16 |  |  |  |  |  |  |  |  |
| cSH | 1551 | 1530 | 708 | 707 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.01 | 0.01 | 0.15 | 0.20 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.2 | 0.3 | 3.9 | 5.6 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 1.4 | 2.2 | 11.0 | 11.4 |  |  |  |  |  |  |  |  |
| Lane LOS | A | A | B | B |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 1.4 | 2.2 | 11.0 | 11.4 |  |  |  |  |  |  |  |  |
| Approach LOS |  |  | B | B |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 7.4 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 21.4\% |  | CU Level | f Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |



|  | 4 |  |  | 1 |  |  | 4 | $\dagger$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\uparrow$ |  |  | $\uparrow$ |  |  | ${ }_{\$}$ |  |  | \$ |  |
| Volume (veh/h) | 0 | 0 | 0 | 0 | 0 | 500 | 0 | 20 | 0 | 250 | 15 | 0 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 0 | 0 | 543 | 0 | 22 | 0 | 272 | 16 | 0 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| VC , conflicting volume | 1125 | 582 | 16 | 582 | 582 | 22 | 16 |  |  | 22 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu, unblocked vol | 1125 | 582 | 16 | 582 | 582 | 22 | 16 |  |  | 22 |  |  |
| tC , single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| $\mathrm{tC}, 2$ stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 100 | 100 | 100 | 100 | 100 | 49 | 100 |  |  | 83 |  |  |
| cM capacity (veh/h) | 77 | 352 | 1063 | 369 | 352 | 1055 | 1601 |  |  | 1594 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 0 | 543 | 22 | 288 |  |  |  |  |  |  |  |  |
| Volume Left | 0 | 0 | 0 | 272 |  |  |  |  |  |  |  |  |
| Volume Right | 0 | 543 | 0 | 0 |  |  |  |  |  |  |  |  |
| cSH | 1700 | 1055 | 1601 | 1594 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.00 | 0.51 | 0.00 | 0.17 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.0 | 23.1 | 0.0 | 4.7 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 0.0 | 12.0 | 0.0 | 7.4 |  |  |  |  |  |  |  |  |
| Lane LOS | A | B |  | A |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 0.0 | 12.0 | 0.0 | 7.4 |  |  |  |  |  |  |  |  |
| Approach LOS | A | B |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 10.1 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 58.9\% |  | U Level | f Service |  |  | B |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |





|  | $\stackrel{ }{*}$ |  |  | 7 | $\leftarrow$ | 4 | 4 | $\dagger$ | 7 | $\checkmark$ | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | ¢ |  |  | ¢ |  |  | $\uparrow$ |  |  | ¢ |  |
| Sign Control |  | Stop |  |  | Stop |  |  | Stop |  |  | Stop |  |
| Volume (vph) | 15 | 65 | 35 | 20 | 40 | 20 | 15 | 75 | 15 | 25 | 130 | 20 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 16 | 71 | 38 | 22 | 43 | 22 | 16 | 82 | 16 | 27 | 141 | 22 |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total (vph) | 125 | 87 | 114 | 190 |  |  |  |  |  |  |  |  |
| Volume Left (vph) | 16 | 22 | 16 | 27 |  |  |  |  |  |  |  |  |
| Volume Right (vph) | 38 | 22 | 16 | 22 |  |  |  |  |  |  |  |  |
| Hadj (s) | -0.12 | -0.07 | -0.02 | -0.01 |  |  |  |  |  |  |  |  |
| Departure Headway (s) | 4.6 | 4.7 | 4.6 | 4.5 |  |  |  |  |  |  |  |  |
| Degree Utilization, x | 0.16 | 0.11 | 0.15 | 0.24 |  |  |  |  |  |  |  |  |
| Capacity (veh/h) | 722 | 703 | 733 | 747 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 8.5 | 8.3 | 8.4 | 9.0 |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 8.5 | 8.3 | 8.4 | 9.0 |  |  |  |  |  |  |  |  |
| Approach LOS | A | A | A | A |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Delay |  |  | 8.6 |  |  |  |  |  |  |  |  |  |
| Level of Service |  |  | A |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 27.0\% |  | CU Level of | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |





|  | 4 |  |  | 1 |  |  | 4 | $\dagger$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\uparrow$ |  |  | $\uparrow$ |  |  | \$ |  |  | \$ |  |
| Volume (veh/h) | 15 | 45 | 25 | 20 | 60 | 15 | 15 | 70 | 15 | 15 | 100 | 15 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 16 | 49 | 27 | 22 | 65 | 16 | 16 | 76 | 16 | 16 | 109 | 16 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  | None |  |  | None |  |  |  |  |  |  |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| VC , conflicting volume | 82 |  |  | 76 |  |  | 283 | 220 | 62 | 266 | 226 | 73 |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu, unblocked vol | 82 |  |  | 76 |  |  | 283 | 220 | 62 | 266 | 226 | 73 |
| tC , single (s) | 4.1 |  |  | 4.1 |  |  | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 2.2 |  |  | 2.2 |  |  | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free \% | 99 |  |  | 99 |  |  | 97 | 88 | 98 | 97 | 83 | 98 |
| cM capacity (veh/h) | 1516 |  |  | 1523 |  |  | 564 | 661 | 1002 | 604 | 657 | 988 |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 92 | 103 | 109 | 141 |  |  |  |  |  |  |  |  |
| Volume Left | 16 | 22 | 16 | 16 |  |  |  |  |  |  |  |  |
| Volume Right | 27 | 16 | 16 | 16 |  |  |  |  |  |  |  |  |
| cSH | 1516 | 1523 | 679 | 676 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.01 | 0.01 | 0.16 | 0.21 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.2 | 0.3 | 4.3 | 6.0 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 1.4 | 1.6 | 11.3 | 11.7 |  |  |  |  |  |  |  |  |
| Lane LOS | A | A | B | B |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 1.4 | 1.6 | 11.3 | 11.7 |  |  |  |  |  |  |  |  |
| Approach LOS |  |  | B | B |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 7.1 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 22.8\% |  | CU Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |



|  | $\stackrel{ }{*}$ |  |  | 1 |  | 4 | 4 | $\dagger$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\uparrow$ |  |  | $\uparrow$ |  |  | ${ }_{\$}$ |  |  | \$ |  |
| Volume (veh/h) | 0 | 0 | 0 | 0 | 0 | 0 | 40 | 20 | 0 | 0 | 15 | 360 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 0 | 0 | 0 | 43 | 22 | 0 | 0 | 16 | 391 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| VC , conflicting volume | 321 | 321 | 212 | 321 | 516 | 22 | 408 |  |  | 22 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu, unblocked vol | 321 | 321 | 212 | 321 | 516 | 22 | 408 |  |  | 22 |  |  |
| tC , single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| $\mathrm{tC}, 2$ stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 100 | 100 | 100 | 100 | 100 | 100 | 96 |  |  | 100 |  |  |
| cM capacity (veh/h) | 614 | 574 | 828 | 614 | 445 | 1055 | 1151 |  |  | 1594 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 0 | 0 | 65 | 408 |  |  |  |  |  |  |  |  |
| Volume Left | 0 | 0 | 43 | 0 |  |  |  |  |  |  |  |  |
| Volume Right | 0 | 0 | 0 | 391 |  |  |  |  |  |  |  |  |
| cSH | 1700 | 1700 | 1151 | 1594 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.00 | 0.51 | 0.04 | 0.00 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.0 | 0.0 | 0.9 | 0.0 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 0.0 | 0.0 | 5.6 | 0.0 |  |  |  |  |  |  |  |  |
| Lane LOS | A | A | A |  |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 0.0 | 0.0 | 5.6 | 0.0 |  |  |  |  |  |  |  |  |
| Approach LOS | A | A |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 0.8 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 33.1\% |  | U Level | f Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |





|  | 4 | $\rightarrow$ |  | 7 | $\leftarrow$ | 4 | 4 | $\dagger$ | 1 | $\downarrow$ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 4 |  |  | 4 |  |  | ¢ |  |  | ¢ |  |
| Sign Control |  | Stop |  |  | Stop |  |  | Stop |  |  | Stop |  |
| Volume (vph) | 45 | 45 | 35 | 20 | 55 | 20 | 15 | 75 | 15 | 40 | 130 | 35 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 49 | 49 | 38 | 22 | 60 | 22 | 16 | 82 | 16 | 43 | 141 | 38 |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total (vph) | 136 | 103 | 114 | 223 |  |  |  |  |  |  |  |  |
| Volume Left (vph) | 49 | 22 | 16 | 43 |  |  |  |  |  |  |  |  |
| Volume Right (vph) | 38 | 22 | 16 | 38 |  |  |  |  |  |  |  |  |
| Hadj (s) | -0.06 | -0.05 | -0.02 | -0.03 |  |  |  |  |  |  |  |  |
| Departure Headway (s) | 4.8 | 4.8 | 4.8 | 4.6 |  |  |  |  |  |  |  |  |
| Degree Utilization, x | 0.18 | 0.14 | 0.15 | 0.29 |  |  |  |  |  |  |  |  |
| Capacity (veh/h) | 693 | 682 | 708 | 736 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 8.8 | 8.6 | 8.6 | 9.5 |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 8.8 | 8.6 | 8.6 | 9.5 |  |  |  |  |  |  |  |  |
| Approach LOS | A | A | A | A |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Delay |  |  | 9.0 |  |  |  |  |  |  |  |  |  |
| Level of Service |  |  | A |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 35.2\% |  | CU Level | f Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |





|  | 3 | $\rightarrow$ | $\cdots$ | $\checkmark$ |  | 4 | 4 | 9 | $p$ |  | $\frac{1}{\dagger}$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \& |  |  | ¢ |  |  | * |  |  | \& |  |
| Volume (veh/h) | 15 | 75 | 25 | 20 | 70 | 15 | 15 | 70 | 15 | 15 | 105 | 105 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 16 | 82 | 27 | 22 | 76 | 16 | 16 | 76 | 16 | 16 | 114 | 114 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  | None |  |  | None |  |  |  |  |  |  |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal (m) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| vC , conflicting volume | 92 |  |  | 109 |  |  | 427 | 264 | 95 | 310 | 269 | 84 |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vC 2 , stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu, unblocked vol | 92 |  |  | 109 |  |  | 427 | 264 | 95 | 310 | 269 | 84 |
| tC , single (s) | 4.1 |  |  | 4.1 |  |  | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| tC, 2 stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 2.2 |  |  | 2.2 |  |  | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 |
| p0 queue free \% | 99 |  |  | 99 |  |  | 96 | 88 | 98 | 97 | 82 | 88 |
| cM capacity (veh/h) | 1502 |  |  | 1482 |  |  | 400 | 625 | 962 | 562 | 621 | 975 |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 125 | 114 | 109 | 245 |  |  |  |  |  |  |  |  |
| Volume Left | 16 | 22 | 16 | 16 |  |  |  |  |  |  |  |  |
| Volume Right | 27 | 16 | 16 | 114 |  |  |  |  |  |  |  |  |
|  | 1502 | 1482 | 606 | 741 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.01 | 0.01 | 0.18 | 0.33 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.3 | 0.3 | 4.9 | 11.0 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 1.0 | 1.5 | 12.2 | 12.2 |  |  |  |  |  |  |  |  |
|  | A | A | B | B |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 1.0 | 1.5 | 12.2 | 12.2 |  |  |  |  |  |  |  |  |
| Approach LOS |  |  | B | B |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 7.8 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 29.8\% |  | Level | Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |



|  | $\stackrel{ }{*}$ |  |  | 1 |  |  | 4 | $\dagger$ | \% |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\uparrow$ |  |  | \$ |  |  | ${ }_{\$}$ |  |  | \$ |  |
| Volume (veh/h) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 20 | 120 | 225 | 15 | 0 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |
| Grade |  | 0\% |  |  | 0\% |  |  | 0\% |  |  | 0\% |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Hourly flow rate (vph) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 22 | 130 | 245 | 16 | 0 |
| Pedestrians |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Width ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| Walking Speed (m/s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Blockage |  |  |  |  |  |  |  |  |  |  |  |  |
| Right turn flare (veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Median type |  |  |  |  |  |  |  | None |  |  | None |  |
| Median storage veh) |  |  |  |  |  |  |  |  |  |  |  |  |
| Upstream signal ( m ) |  |  |  |  |  |  |  |  |  |  |  |  |
| pX, platoon unblocked |  |  |  |  |  |  |  |  |  |  |  |  |
| VC , conflicting volume | 592 | 658 | 16 | 592 | 592 | 87 | 16 |  |  | 152 |  |  |
| $\mathrm{vC1}$, stage 1 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| $\mathrm{vC2}$, stage 2 conf vol |  |  |  |  |  |  |  |  |  |  |  |  |
| vCu, unblocked vol | 592 | 658 | 16 | 592 | 592 | 87 | 16 |  |  | 152 |  |  |
| tC , single (s) | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 | 4.1 |  |  | 4.1 |  |  |
| $\mathrm{tC}, 2$ stage (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| tF (s) | 3.5 | 4.0 | 3.3 | 3.5 | 4.0 | 3.3 | 2.2 |  |  | 2.2 |  |  |
| p0 queue free \% | 100 | 100 | 100 | 100 | 100 | 100 | 100 |  |  | 83 |  |  |
| cM capacity (veh/h) | 363 | 319 | 1063 | 363 | 347 | 972 | 1601 |  |  | 1429 |  |  |
| Direction, Lane \# | EB 1 | WB 1 | NB 1 | SB 1 |  |  |  |  |  |  |  |  |
| Volume Total | 0 | 0 | 152 | 261 |  |  |  |  |  |  |  |  |
| Volume Left | 0 | 0 | 0 | 245 |  |  |  |  |  |  |  |  |
| Volume Right | 0 | 0 | 130 | 0 |  |  |  |  |  |  |  |  |
| cSH | 1700 | 1700 | 1601 | 1429 |  |  |  |  |  |  |  |  |
| Volume to Capacity | 0.00 | 0.51 | 0.00 | 0.17 |  |  |  |  |  |  |  |  |
| Queue Length 95th (m) | 0.0 | 0.0 | 0.0 | 4.7 |  |  |  |  |  |  |  |  |
| Control Delay (s) | 0.0 | 0.0 | 0.0 | 7.6 |  |  |  |  |  |  |  |  |
| Lane LOS | A | A |  | A |  |  |  |  |  |  |  |  |
| Approach Delay (s) | 0.0 | 0.0 | 0.0 | 7.6 |  |  |  |  |  |  |  |  |
| Approach LOS | A | A |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Average Delay |  |  | 4.8 |  |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization |  |  | 28.4\% |  | CU Level | f Service |  |  | A |  |  |  |
| Analysis Period (min) |  |  | 15 |  |  |  |  |  |  |  |  |  |




[^0]:    ${ }^{1}$ Highway Capacity Manual. Transportation Research Board, Washington DC, 2010.

[^1]:    Note: for site traffic, consider the peak directional volumes from both peak hours (ie. consider the maximum volumes)

